

COLUMBIA STREET ANNEX STORMWATER IMPACT STATEMENT

SPECIAL USE PERMIT SUBMITTAL

PIN 9788 20 4502 9788 21 5049

H CAROUS SEAL 23461

Calculations By: Tiffany R. McCormack, EI Checked By: Charles N. Hill, Jr, PE

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Project # 1306

April 25, 2018

Project Description and Summary

The Columbia Street Annex project is located at the northwest corner of the intersection of South Columbia Street and the on-ramp to NC 54 with the right-of-way for Monroe Street running through the site in Chapel Hill, North Carolina. The proposed project involves the conversion of a wooded lot into a seven story multi-use development with associated parking. The existing right-of-way will be closed and this area will become a part of the proposed development. Soils on the site and contributing drainage area are Wedowee, Appling and Urban (hydrologic soil groups B & D). The site is comprised of mostly B soils. The proposed site is located in the Cape Fear River Basin, the Jordan Lake protected watershed, and WS-IV NSW state designations.

Per the Town of Chapel Hill stormwater ordinance, the stormwater discharge rate leaving the site under post-development conditions may not exceed the stormwater discharge rate under predevelopment conditions for the 1-year, 2-year, and 25-year storms. The stormwater runoff volume post-development may not exceed the stormwater runoff volume pre-development for the 2-year storm event. Stormwater treatment must be used to achieve an 85% Total Suspended Solids (TSS) reduction from the post development water quality volume runoff.

Methodology

- HydroCAD software is used to calculate the time of concentration for each sub-basin.
 HydroCAD uses the SCS segmental approach to estimate the time of concentration.
- HydroCAD software is used to calculate the composite curve number for each sub-basin.
 HydroCAD uses the NRCS TR-55 method for calculation composite curve numbers.
- HydroCAD software is used to calculate pre- and post-development peak flow rates and volumes for each sub-basin. HydroCAD uses the SCS TR-20 method to develop hydrographs.
- The Orange County Soil Survey is used to identify the soil types located on the site.

Discussion of Results

Peak Flow Analysis

For peak flow analysis, the site was analyzed as two basins. Basin 1 drains south, to an unnamed tributary of Morgan Creek. Basin 2 also drains to the south, eventually discharging to the same unnamed tributary of Morgan Creek. The proposed development will increase the runoff for the design storms to the analysis point for Basin 1. An underground detention system is proposed to attenuate the discharge from Bain 1 for the design storms. There is no change to ground cover for Basin 2. Analysis of the 1-, 2- and 10-year storm events result in no increase to peak flow following development for Basin 2. (See spreadsheet *Peak Flow Analysis Summary*).

Stormwater Runoff Volume Analysis

For stormwater runoff analysis, only Basin 1 was analyzed. The underground detention system is sized to achieve no increase of the 2-year storm event to the runoff volume following development. (See spreadsheet *Runoff Volume Analysis Summary*).

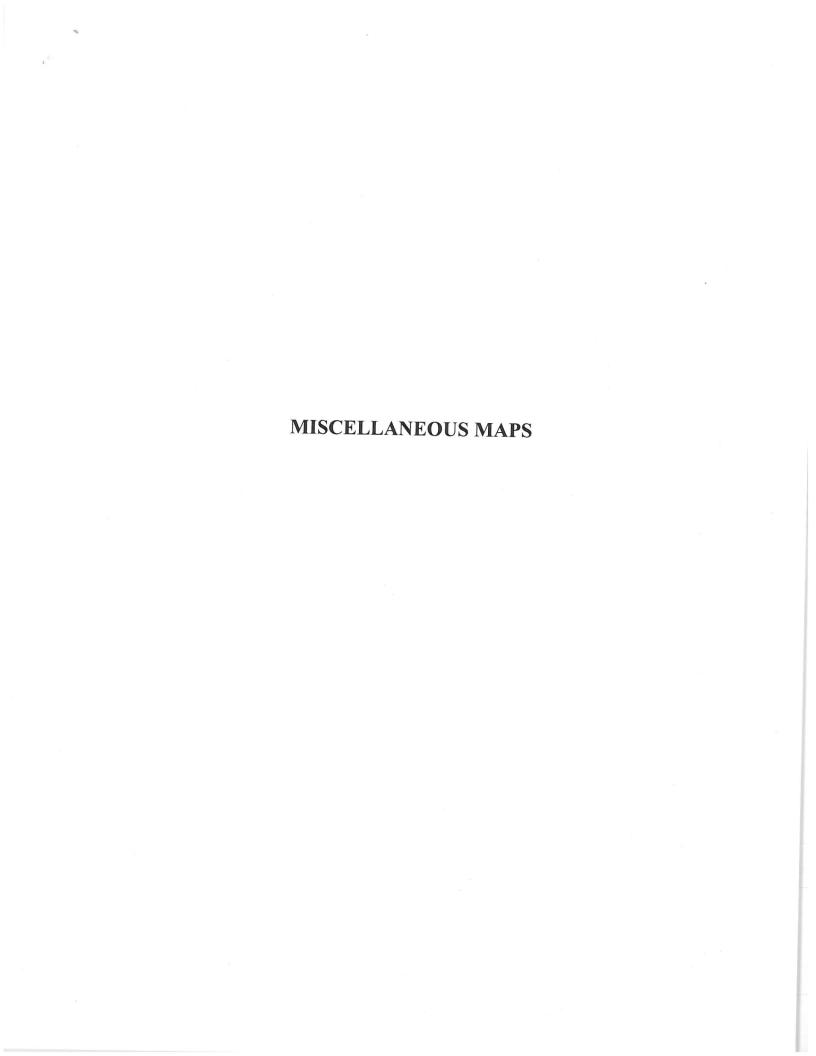
Stormwater Pollutant Analysis

TSS Reduction:

The proposed development of this site requires a TSS reduction for the increases in impervious areas per the Town of Chapel Hill Ordinance. A Storm Filter is proposed to meet the 85% TSS reduction for the new impervious surfaces. The drainage area to this stormwater facility is equals the required treatment area (See spreadsheet TSS Reduction Summary).

Results

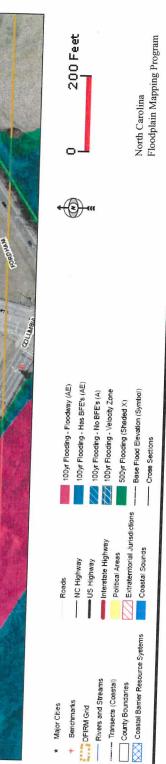
Based on the Town of Chapel Hill Land Use Management Ordinance and all applicable amendments to it, development of this site requires peak flow, volume, and TSS reductions. These requirements are met with the installation of an underground detention facility, and a StormFilter. All applicable calculations and maps follow in this report.



Columbia St. Annex



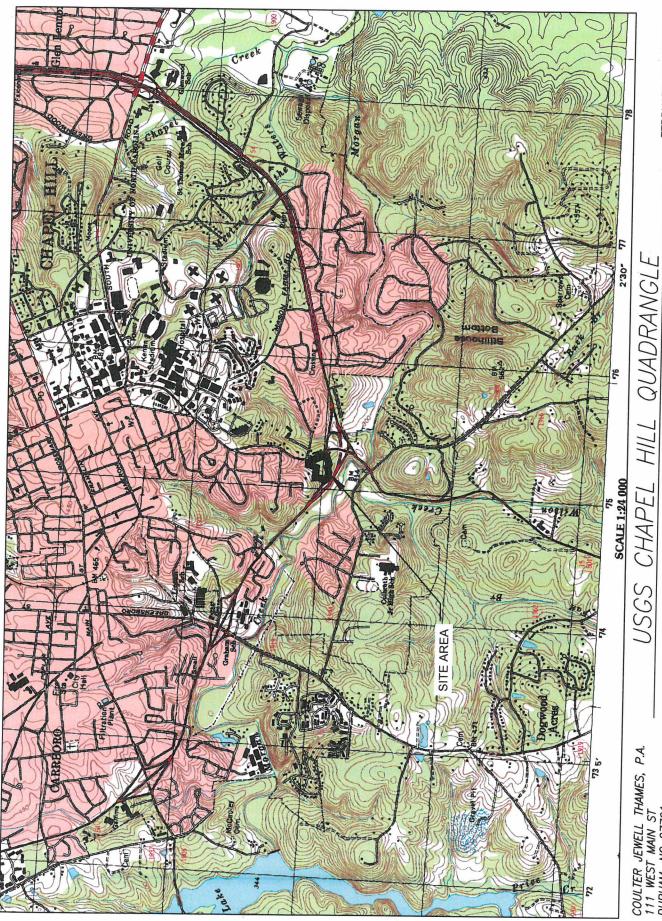




Major Cities

DFIRM Grid





FEBRUARY 20, 2013 SCALE: 1"=2000' DRAWN BY: TRM

COULTER JEWELL THAMES, P.A. 111 WEST MAIN ST DURHAM, NC 27701 (919) 682-0368



PEAK FLOW ANALYSIS



Project Name: Columbia Street Annex

Project Number: 1306

By:

TRM

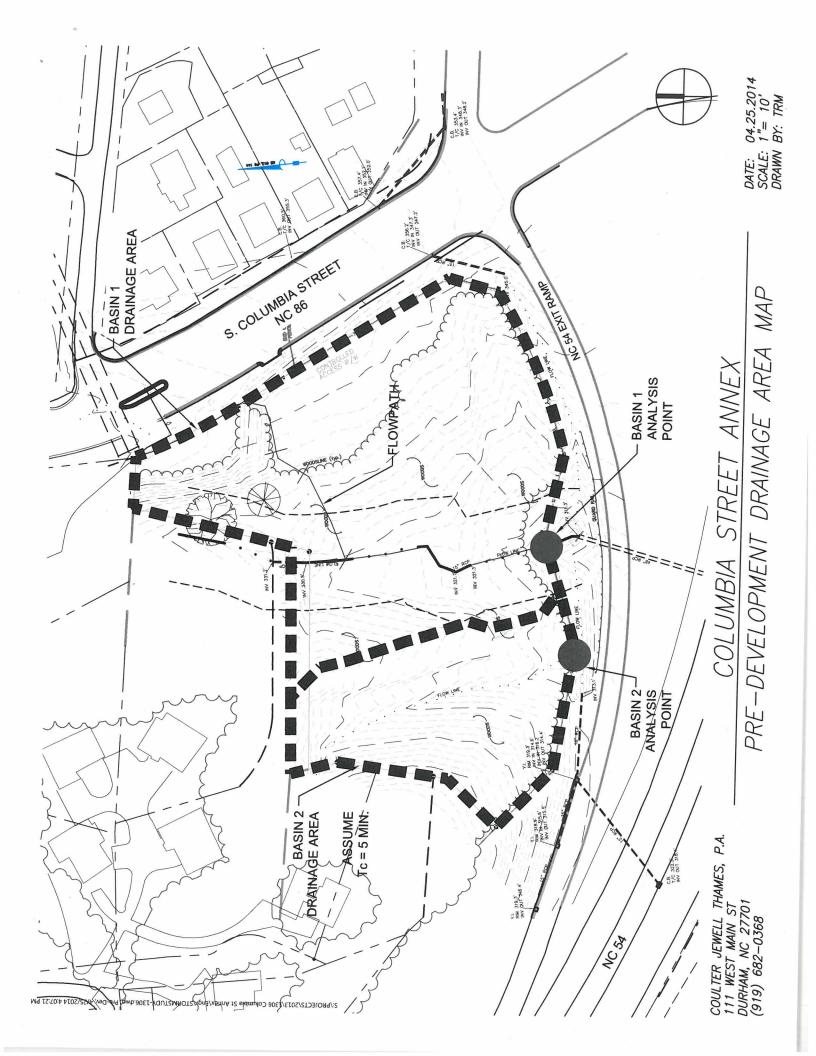
Revised: Date:

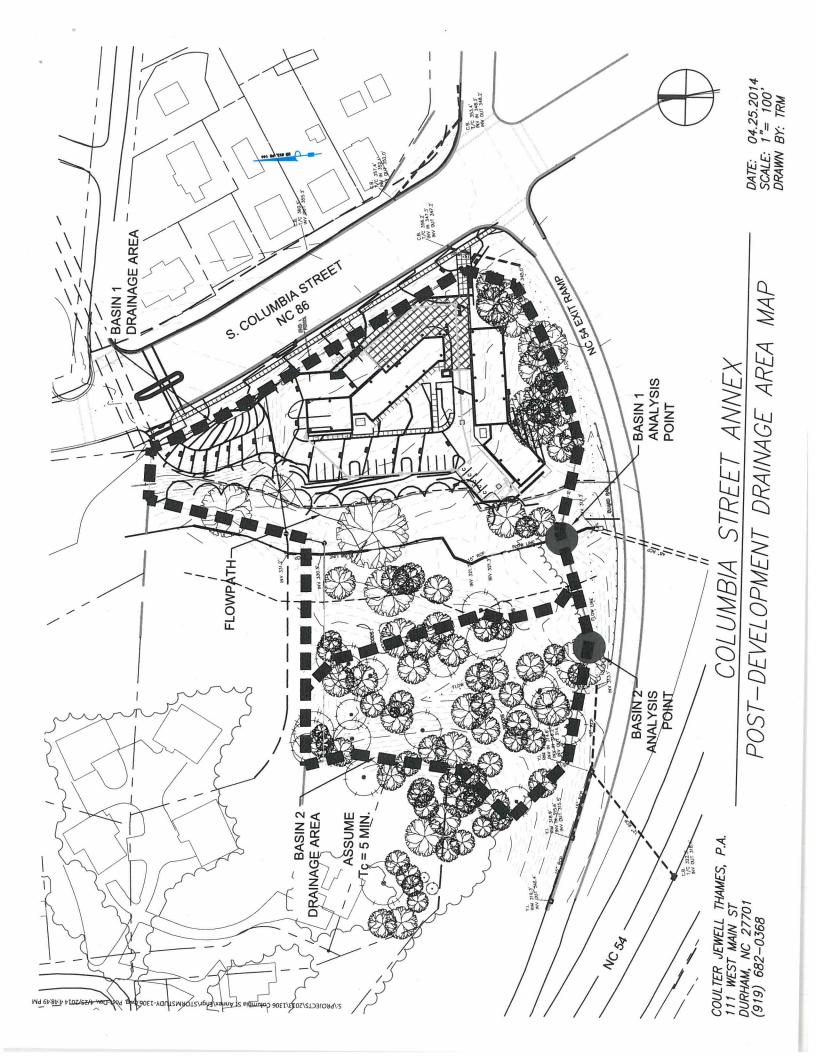
Date: 04.25.14

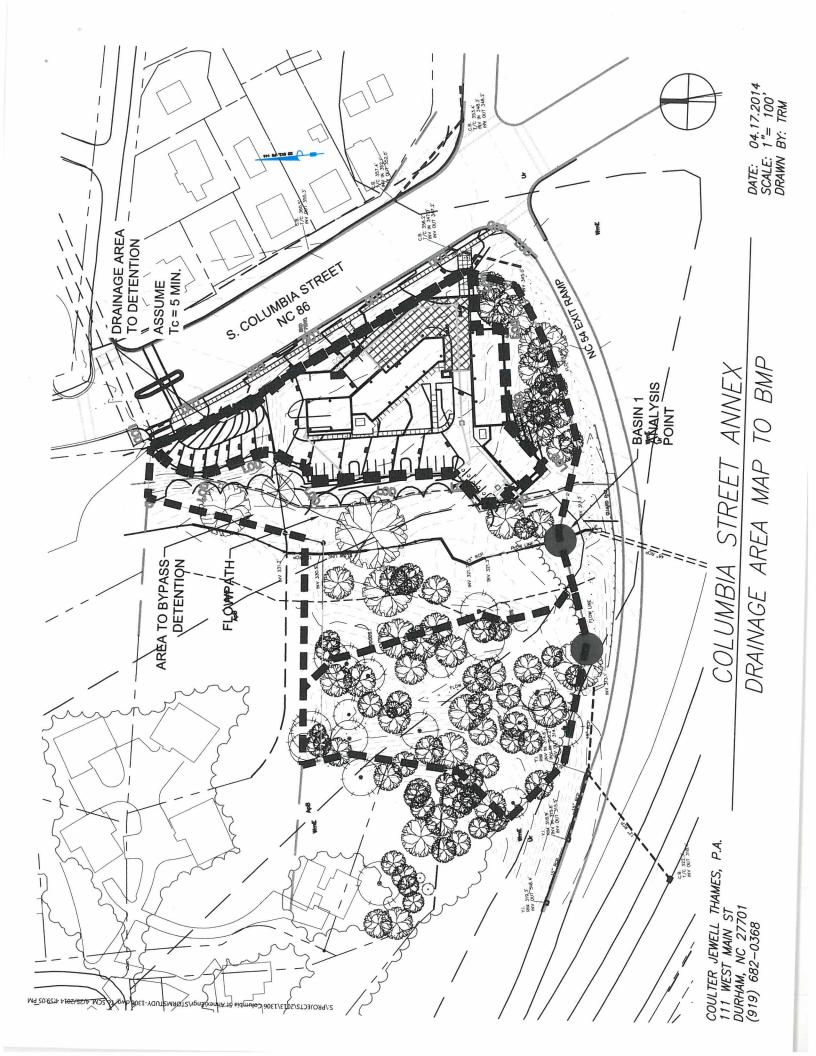
Peak Flow Analysis Summary

Drainage Basin 1	Tc (min)	Area (ac)	CN	Q1 (cfs)	Q2 (cfs)	Q25 (cfs)
Pre-Development	6.5	2.75	57	0.57	1.40	7.73
Post Development	5.8	2.75	73	4.04	5.64	14.72
Post-Development with Treatment Percent difference between Pre-				0.57	1.14	6.90
Development and Post Development with Treatment				0%	-19%	-11%

Drainage Basin 2	Tc (min)	Area (ac)	CN	Q1 (cfs)	Q2 (cfs)	Q25 (cfs)
Pre-Development	5	1.06	55	0.14	0.42	2.77
Post Development Percent difference between Pre-	5	1.06	55	0.14	0.42	2.77
Development and Post Development				0%	0%	0%









Basin 1 -Predevelopment



Basin 2 - Predevelopment



Basin 1 -Postdevelopment



Basin 2 - Postdevelopment









Page 2

Summary for Subcatchment 1S: Basin 1 - Predevelopment

Runoff

0.57 cfs @ 12.02 hrs, Volume=

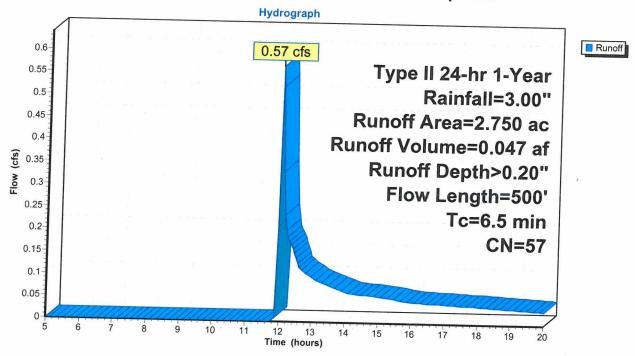
0.047 af, Depth> 0.20"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 1-Year Rainfall=3.00"

	Area	(ac)	CN Des	cription		x				
	0	.020	98 Unc	nconnected pavement, HSG B						
	0.720 61 >75% Grass cover, Good, HSG B									
_	2	.010	55 Woo	ods, Good,	HSG B					
				ghted Avei						
		.730	99.2	7% Pervio	us Area					
		020		% Impervi						
	0.	020	100.	00% Unco	nnected					
	То	Longth	Clana	M-14	0 "	D				
	Tc (min)	Length (feet)	Slope	Velocity	Capacity	Description				
			(ft/ft)	(ft/sec)	(cfs)					
	3.3	75	0.1467	0.38		Sheet Flow,				
	0.0	00	0.0005	4.75		Grass: Short n= 0.150 P2= 3.60"				
	0.9	96	0.0625	1.75		Shallow Concentrated Flow,				
	8.0	92	0.1415	1.00		Short Grass Pasture Kv= 7.0 fps				
	0.0	92	0.1415	1.88		Shallow Concentrated Flow,				
	1.5	237	0.0634	2.65	0.53	Woodland Kv= 5.0 fps				
	1.0	201	0.0004	2.00	0.55	Trap/Vee/Rect Channel Flow,				
						Bot.W=0.00' D=1.00' Z= 0.2 '/' Top.W=0.40' n= 0.030 Earth, grassed & winding				
	6.5	500	Total			11 0.000 Lattii, grasseu & wiriuing				

Page 3

Subcatchment 1S: Basin 1 - Predevelopment



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Page 4

Summary for Subcatchment 2S: Basin 2 - Predevelopment

Runoff

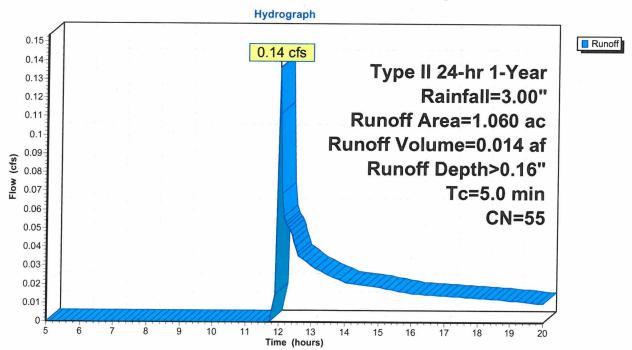
0.14 cfs @ 12.02 hrs, Volume=

0.014 af, Depth> 0.16"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 1-Year Rainfall=3.00"

	Area	(ac)	CN	Desc	cription						
	0.	.030	61	>759	>75% Grass cover, Good, HSG B						
	1.	.020	55	Woo	ds, Good,	HSG B					
_	0.	.010	0 80 >75% Grass cover, Good, HSG D								
	1.	.060	060 55 Weighted Average								
	1.	060		100.	00% Pervi	ous Area					
	-			. .							
	Tc	Lengt		Slope	Velocity	Capacity	Description				
_	(min)	(feet	t)	(ft/ft)	(ft/sec)	(cfs)					
	5.0						Direct Entry.				

Subcatchment 2S: Basin 2 - Predevelopment



Page 5

Summary for Subcatchment 3S: Basin 1 - Postdevelopment

Runoff =

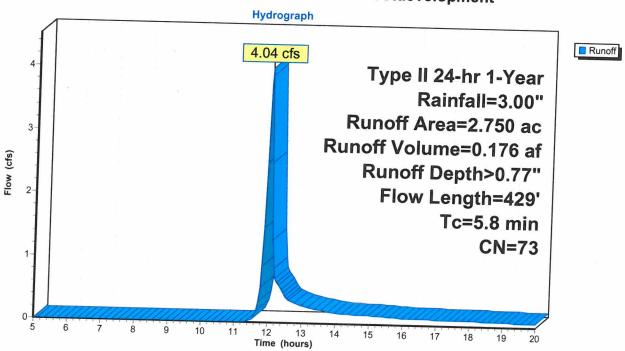
4.04 cfs @ 11.98 hrs, Volume=

0.176 af, Depth> 0.77"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 1-Year Rainfall=3.00"

Area	a (ac)	CN Des	cription		
•	1.070	98 Pav	ed parking	, HSG B	
	0.430	61 >75	% Grass c	over, Good	L HSG B
1	.250	55 Woo	ods, Good,	HSG B	,, 1188 B
2	2.750		ghted Avei		
	.680	61.0	9% Pervio	us Area	
1	.070			ious Area	
_					
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	1 2703-00 €
3.0	75	0.1867	0.42		Sheet Flow,
4.0	44-				Grass: Short n= 0.150 P2= 3.60"
1.3	117	0.0939	1.53		Shallow Concentrated Flow,
1.5	227	0.0004			Woodland Kv= 5.0 fps
1.5	237	0.0634	2.65	0.53	Trap/Vee/Rect Channel Flow
	400	-			Bot.W=0.00' D=1.00' Z= 0.2 '/' Top.W=0.40' n= 0.030
5.8	429	Total			1.0000

Subcatchment 3S: Basin 1 - Postdevelopment



Page 6

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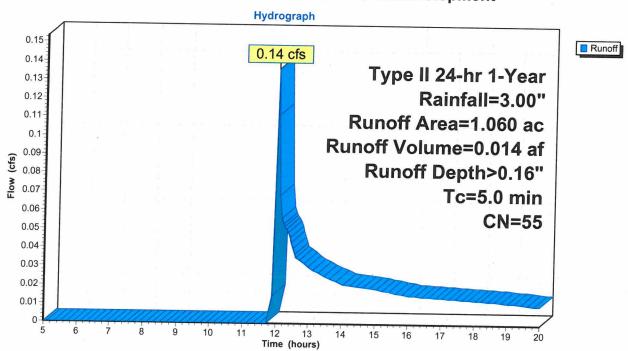
Summary for Subcatchment 4S: Basin 2 - Postdevelopment Runoff 0.14 cfs @ 12.02 hrs, Volume=

0.014 af, Depth> 0.16"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 1-Year Rainfall=3.00"

Area	(ac)	CN	Desc	cription					
0.	030	61	>75%	% Grass co	over, Good	. HSG B			
1.	020	55	Woo	ds, Good,	HSG B	,			
0.	010				over, Good	, HSG D			
	1.060 55 Weighted Average								
1.0	060		100.0	00% Pervi	ous Area				
(min)	Length (feet)		ope ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0						Direct Entry			

Subcatchment 4S: Basin 2 - Postdevelopment



STORM STUDY - 1306

Type II 24-hr 2-Year Rainfall=3.50" Printed 4/28/2014

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Page 4

Summary for Subcatchment 1S: Basin 1 - Predevelopment

Runoff

1.40 cfs @ 12.01 hrs, Volume=

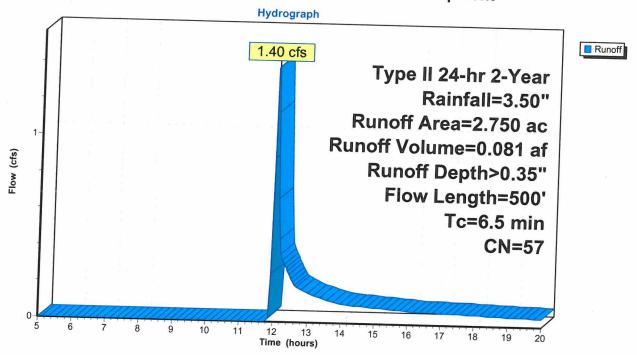
0.081 af, Depth> 0.35"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

Area	ı (ac)	CN Des	cription		
	0.020	98 Und	onnected	pavement,	HSG B
).720 :.010	61 >75 55 Woo	% Grass o ods, Good,	over, Good	d, HSG B
2	.750 .730 .020 .020	57 Wei 99.2 0.73	ghted Ave 27% Pervic 3% Impervi	rage ous Area ous Area	
Тс	Length	Slope	00% Unco	onnected Capacity	Description
<u>(min)</u>	(feet)		(ft/sec)	(cfs)	
3.3	75	0.1467	0.38		Sheet Flow,
0.9	96	0.0625	1.75		Grass: Short n= 0.150 P2= 3.60" Shallow Concentrated Flow,
0.8	92	0.1415	1.88		Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow.
1.5	237	0.0634	2.65	0.53	Woodland Kv= 5.0 fps Trap/Vee/Rect Channel Flow.
1					Bot.W=0.00' D=1.00' Z= 0.2 '/' Top.W=0.40'
6.5	500	Total			n= 0.030 Earth, grassed & winding

Page 5

Subcatchment 1S: Basin 1 - Predevelopment



Page 9

Summary for Subcatchment 2S: Basin 2 - Predevelopment

Runoff

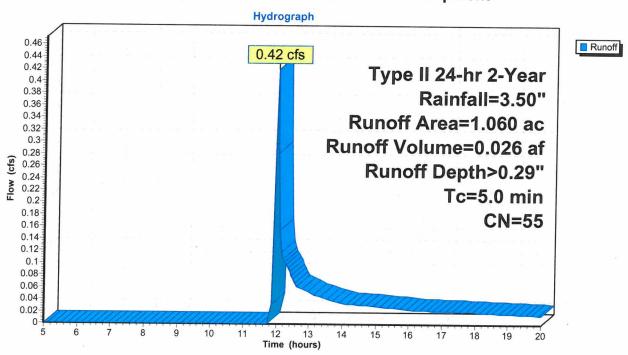
0.42 cfs @ 12.00 hrs, Volume=

0.026 af, Depth> 0.29"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

A	rea (ac)	CN	Des	cription							
	0.030	61	>759	75% Grass cover, Good, HSG B							
	1.020	55		/oods, Good, HSG B							
	0.010	80	>75	% Grass co	over, Good	, HSG D					
	1.060	55	Weig	ghted Aver	age						
	1.060		100.	00% Pervi	ous Area						
	Tc Leng		Slope	Velocity	Capacity	Description					
(m	in) (fee	et)	(ft/ft)	(ft/sec)	(cfs)		1				
5	5.0					Direct Entry					

Subcatchment 2S: Basin 2 - Predevelopment



Page 10

Summary for Subcatchment 3S: Basin 1 - Postdevelopment

Runoff

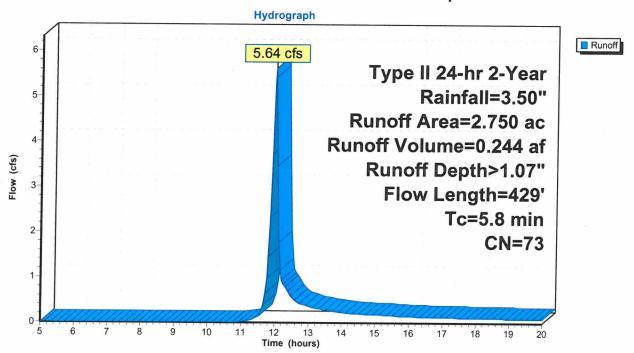
5.64 cfs @ 11.98 hrs, Volume=

0.244 af, Depth> 1.07"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

,	Area	(ac) (CN Des	cription		
	1	.070	98 Pav	ed parking	, HSG B	
	0.	.430	61 >75	% Ġrass c	over, Good	, HSG B
	1.	.250		ds, Good,		, and a second
	2.	.750	73 Wei	ghted Avei	rage	
	1.	.680		9% Pervio		
	1.	.070			ious Area	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	3.0	75	0.1867	0.42		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.60"
	1.3	117	0.0939	1.53		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	1.5	237	0.0634	2.65	0.53	Trap/Vee/Rect Channel Flow,
						Bot.W=0.00' D=1.00' Z= 0.2 '/' Top.W=0.40' n= 0.030
	5.8	429	Total		9	

Subcatchment 3S: Basin 1 - Postdevelopment



Page 11

Summary for Subcatchment 4S: Basin 2 - Postdevelopment

Runoff

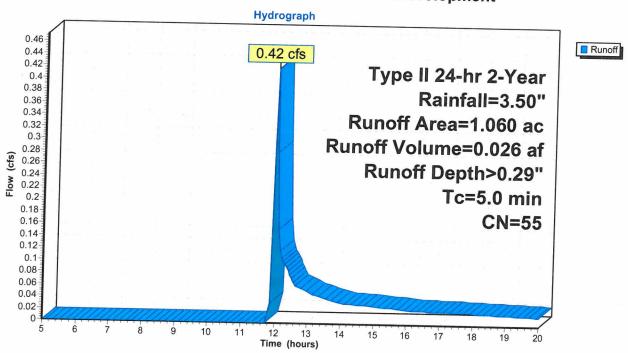
0.42 cfs @ 12.00 hrs, Volume=

0.026 af, Depth> 0.29"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

Area	(ac)	CN	Des	cription						
0	.030	61	>75	75% Grass cover, Good, HSG B						
	.020	55	Woo	ds, Good,	HSG B	, 1.00 B				
0	.010				over, Good	, HSG D				
1	.060			hted Aver						
1	.060			00% Pervi						
Tc (min)	Lengtl (feet		ope ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0						Direct Entry.				

Subcatchment 4S: Basin 2 - Postdevelopment



STORM STUDY - 1306

Type II 24-hr 25-Year Rainfall=6.00"

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Page 6

Summary for Subcatchment 1S: Basin 1 - Predevelopment

Runoff

7.71 cfs @ 11.99 hrs, Volume=

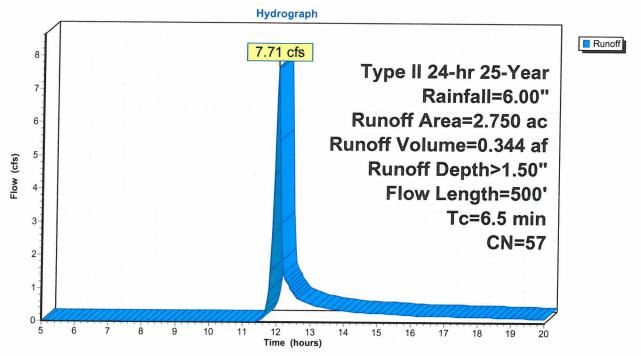
0.344 af, Depth> 1.50"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 25-Year Rainfall=6.00"

_	Area	(ac) (CN Des	cription					
	0	.020	98 Und	onnected	pavement,	HSG B			
	0.720 61 >75% Grass cover, Good, HSG B								
	2	.010		ods, Good,					
	2.	750	57 Wei	ghted Ave	rage				
	2.	730		7% Pervio					
	0.	020	0.73	% Impervi	ous Area				
	0.	020		00% Unco					
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.3	75	0.1467	0.38		Sheet Flow,			
						Grass: Short n= 0.150 P2= 3.60"			
	0.9	96	0.0625	1.75		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	8.0	92	0.1415	1.88		Shallow Concentrated Flow,			
			_			Woodland Kv= 5.0 fps			
	1.5	237	0.0634	2.65	0.53	Trap/Vee/Rect Channel Flow,			
						Bot.W=0.00' D=1.00' Z= 0.2 '/' Top.W=0.4	0'		
_						n= 0.030 Earth, grassed & winding			
	6.5	500	Total						

Page 7

Subcatchment 1S: Basin 1 - Predevelopment



Page 14

Summary for Subcatchment 2S: Basin 2 - Predevelopment

Runoff

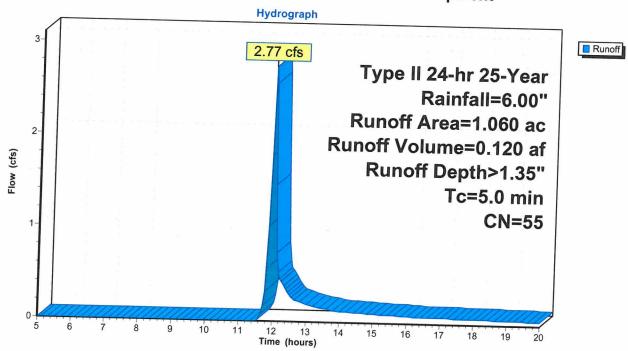
2.77 cfs @ 11.97 hrs, Volume=

0.120 af, Depth> 1.35"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 25-Year Rainfall=6.00"

_	Area	(ac)	CN	Des	cription			
		.030	61	>75	% Grass c	over, Good	. HSG B	
		.020	55	Woo	ds, Good,	HSG B	,	
_	0.	.010	80	>75	% Grass co	over, Good	HSG D	
	1.	060	55	Wei	ghted Aver	age	, 1.00 B	
	1.	060		100.	00% Pervi	ous Area		
	Tc (min)	Lengtl (feet	_	lope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	5.0						Direct Entry.	

Subcatchment 2S: Basin 2 - Predevelopment



Summary for Subcatchment 3S: Basin 1 - Postdevelopment

Runoff = 14.72 c

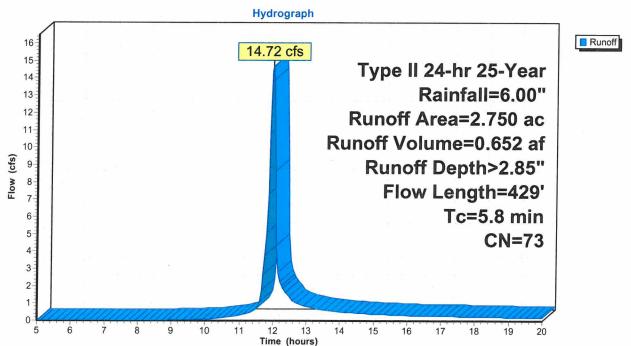
14.72 cfs @ 11.97 hrs, Volume=

0.652 af, Depth> 2.85"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 25-Year Rainfall=6.00"

	Area	(ac) (ON Des	cription					
	1.	.070	98 Pav	ed parking	. HSG B				
	0.430 61 >75% Grass cover, Good, HSG B								
1.250 55 Woods, Good, HSG B									
-		750		ghted Aver					
		680		9% Pervio					
		070		W 10 mm					
	1.	070	30.9	1% imperv	ious Area				
	т.	1	01	\	0 "	D			
	Tc	Length		Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.0	75	0.1867	0.42		Sheet Flow,			
						Grass: Short n= 0.150 P2= 3.60"			
	1.3	117	0.0939	1.53		Shallow Concentrated Flow,			
		151.75.5				Woodland Kv= 5.0 fps			
	1.5	237	0.0634	2.65	0.53	Trap/Vee/Rect Channel Flow,			
	1.0	201	0.0004	2.00	0.55	Bot.W=0.00' D=1.00' Z= 0.2 '/' Top.W=0.40' n= 0.030			
_		100				D01.7V - 0.00 $D - 1.00$ $Z - 0.27$ $10p.VV = 0.40$ $n = 0.030$			
	5.8	429	Total						

Subcatchment 3S: Basin 1 - Postdevelopment



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Page 16

Summary for Subcatchment 4S: Basin 2 - Postdevelopment

Runoff

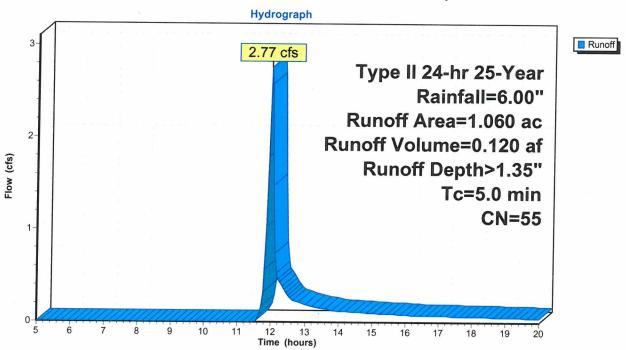
2.77 cfs @ 11.97 hrs, Volume=

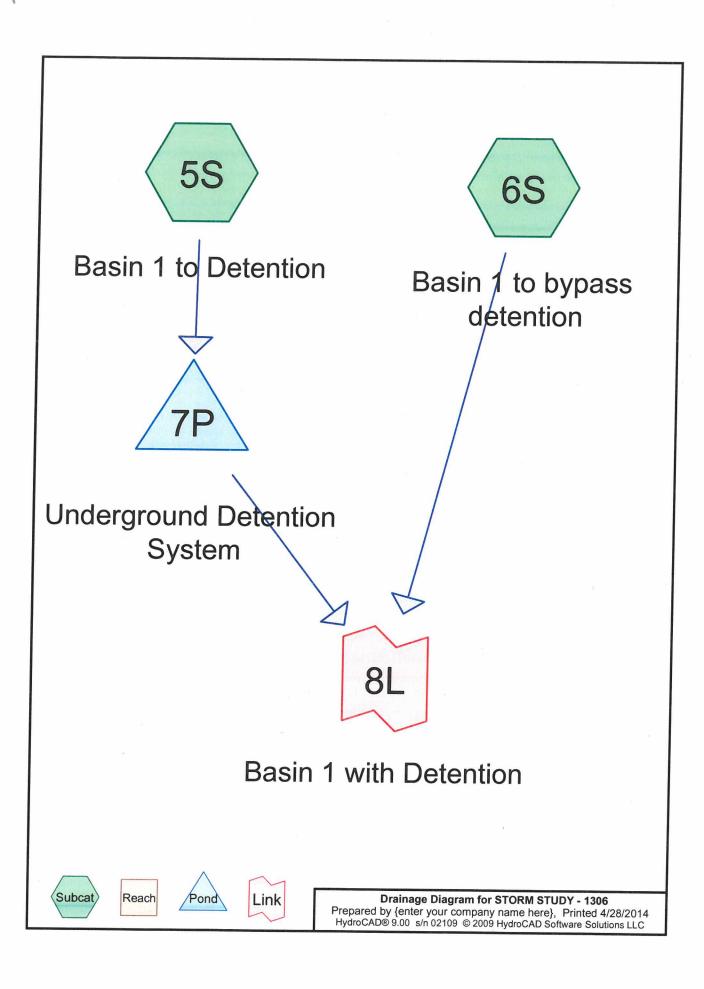
0.120 af, Depth> 1.35"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 25-Year Rainfall=6.00"

_	Area	(ac) CN Description								
0.030 61 >75% Grass cover, Good, HSG B										
1.020 55 Woods, Good, HSG B										
	0.	010	80	>759	% Grass co	over, Good	, HSG D			
1.060 55 Weighted Average										
	1.	060		100.	00% Pervi	ous Area				
	Tc (min)	Lengtl (feet		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0 Direct Entry										

Subcatchment 4S: Basin 2 - Postdevelopment





Page 2

Summary for Subcatchment 5S: Basin 1 to Detention

Runoff

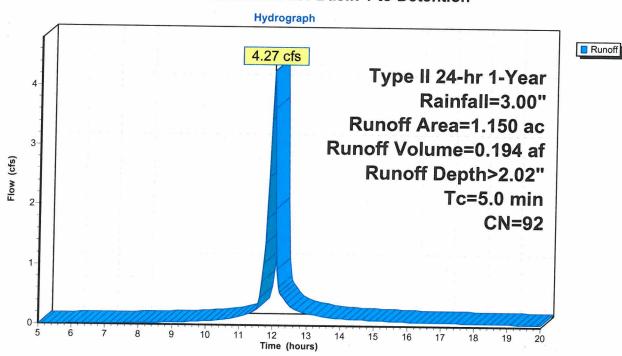
4.27 cfs @ 11.95 hrs, Volume=

0.194 af, Depth> 2.02"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 1-Year Rainfall=3.00"

_	Area	(ac)	CN	Desc	cription				
	0.	.960	98	Pave	ed parking	, HSG B			
0.190 61 >75% Grass cover, Good,							, HSG B		
	1.	150	92	Weig	hted Aver	age			
		190		16.52% Pervious Area					
	0.	960		83.48	3% Imperv	rious Area			
	Тс	Longth	. 61	one	Valasit.	0'	D		
	(min)	Length (feet		ope ft/ft)	Velocity (ft/sec)	Capacity	Description		
-		(ICCL		ivit)	(IDSEC)	(cfs)			
	5.0						Direct Entry		

Subcatchment 5S: Basin 1 to Detention



Summary for Subcatchment 6S: Basin 1 to bypass detention

Runoff

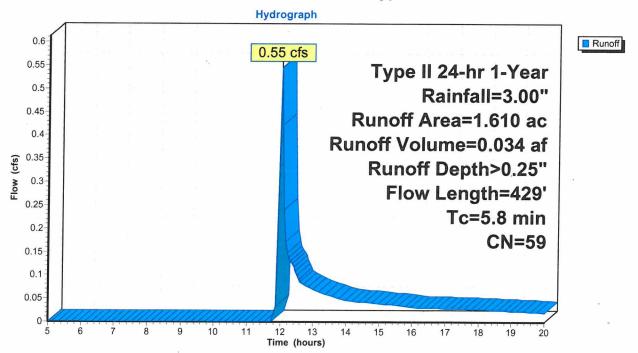
0.55 cfs @ 12.01 hrs, Volume=

0.034 af, Depth> 0.25"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 1-Year Rainfall=3.00"

_	Area	(ac) C	N Des	cription		
	0.	.120	98 Pave	ed parking	, HSG B	
	0.	.240	31 >75	% Ġrass c	over, Good	, HSG B
	1.	.250		ds, Good,		,
	1.	610	59 Wei	hted Aver	age	
	1.	490		5% Pervio	0	
	0.	120	7.45	% Impervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	a see of many
	3.0	75	0.1867	0.42		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.60"
	1.3	117	0.0939	1.53		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	1.5	237	0.0634	2.65	0.53	Trap/Vee/Rect Channel Flow,
						Bot.W=0.00' D=1.00' Z= 0.2'/' Top.W=0.40' n= 0.030
	5.8	429	Total			

Subcatchment 6S: Basin 1 to bypass detention



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Page 4

Summary for Pond 7P: Underground Detention System

Inflow Area =

1.150 ac, 83.48% Impervious, Inflow Depth > 2.02" for 1-Year event

Inflow

4.27 cfs @ 11.95 hrs, Volume=

0.194 af

Outflow

0.03 cfs @ 20.00 hrs, Volume=

0.021 af, Atten= 99%, Lag= 482.8 min

Primary

0.03 cfs @ 20.00 hrs, Volume=

0.021 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Peak Elev= 328.81' @ 20.00 hrs Surf.Area= 1,511 sf Storage= 7,522 cf

Plug-Flow detention time= 342.3 min calculated for 0.021 af (11% of inflow) Center-of-Mass det. time= 161.4 min (921.8 - 760.5)

Volume	e Inv	ert Avail.St	orage	Storage I	Description			
#1	323.	83' 12,8	352 cf			rismatic)Listed below (Recalc)		
Elevati (fe 323. 331.	et) 83	Surf.Area (sq-ft) 1,511 1,511	(cubic	Store c-feet)	Cum.Store (cubic-feet)	(Rodalo)		
332.		1,527		1,333 1,519	11,333 12,852			
Device	Routing	Invert		t Devices	12,002			
#1	Primary	323.50'	L= 20	Round (groove end w/	headwall, Ke= 0.200		
#2	Device 1	323.83'	0.5" \	/ert. Orific	ce/Grate X 2.0	500 '/' Cc= 0.900 n= 0.013		
#3	Device 1	330.33'	1.0' lo Head	ong x 0.5' (feet) 0.2	'	d-Crested Rectangular Weir		
#4 Device		331.33'	Coef. 8.0' Ic	Coef. (English) 2.80 2.92 3.08 3.30 3.32 8.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00				
			Coef.	(English)	2.80 2.92 3.0	8 3.30 3.32		

Primary OutFlow Max=0.03 cfs @ 20.00 hrs HW=328.81' (Free Discharge)

1=Culvert (Passes 0.03 cfs of 15.98 cfs potential flow)

⁻²⁼Orifice/Grate (Orifice Controls 0.03 cfs @ 10.72 fps)

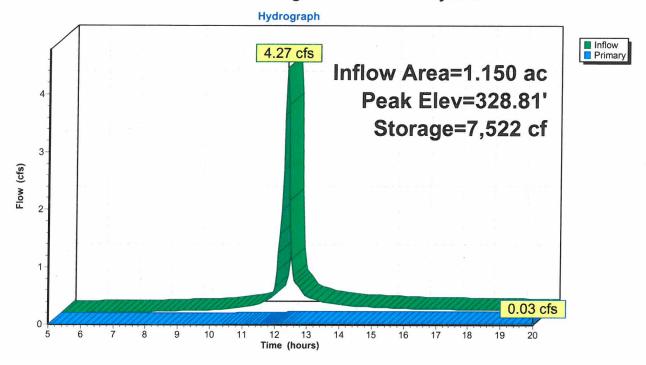
⁻³⁼Broad-Crested Rectangular Weir (Controls 0.00 cfs)

⁻⁴⁼Broad-Crested Rectangular Weir (Controls 0.00 cfs)

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Page 5

Pond 7P: Underground Detention System



Page 6

Summary for Link 8L: Basin 1 with Detention

Inflow Area =

2.760 ac, 39.13% Impervious, Inflow Depth > 0.24" for 1-Year event

Inflow

0.57 cfs @ 12.01 hrs, Volume=

0.055 af

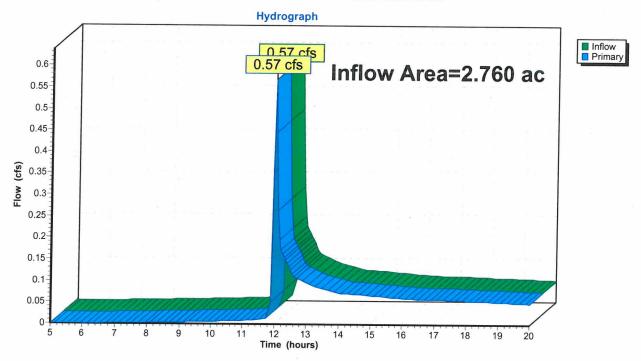
Primary

0.57 cfs @ 12.01 hrs, Volume=

0.055 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link 8L: Basin 1 with Detention



Page 7

Summary for Subcatchment 5S: Basin 1 to Detention

Runoff

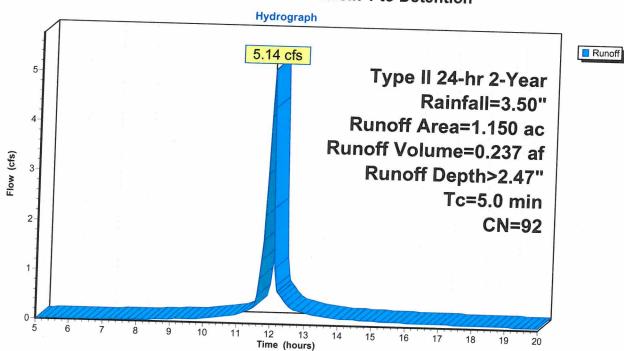
5.14 cfs @ 11.95 hrs, Volume=

0.237 af, Depth> 2.47"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

_	Area	(ac)	CN	Des	cription			
		.960	98	Pave	ed parking	. HSG B		
_	0	.190	61	>75	% Grass c	over, Good	HSG B	
	1	.150	92	Weig	ghted Aver	age	1100 B	_
		.190		16.5	2% Pervio	us Area		
	0.	.960				ious Area		
	То	Lawrett						
	Tc (min)	Length		lope	Velocity	Capacity	Description	
_	-	(feet	(ft/ft)	(ft/sec)	(cfs)	•	
	5.0						Direct Entry,	-

Subcatchment 5S: Basin 1 to Detention



Page 8

Summary for Subcatchment 6S: Basin 1 to bypass detention

Runoff =

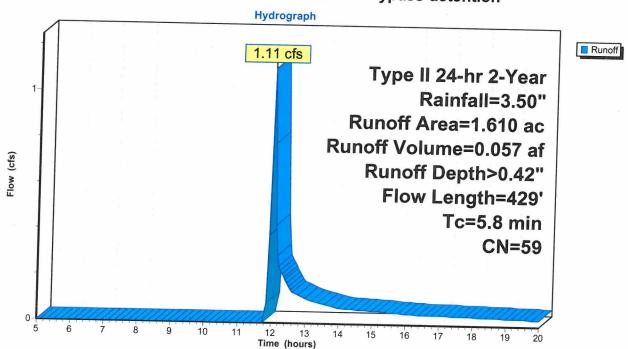
1.11 cfs @ 12.00 hrs, Volume=

0.057 af, Depth> 0.42"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

Are	ea (ac)	С	N Des	cription					
	0.120	9	8 Pav	ed parking	, HSG B				
	0.240	6	31 >75	% Grass c	over, Good	L HSG B			
1.250									
	1.610	5		ghted Avei					
	1.490		92.5	5% Pervio	us Area				
	0.120			% Impervi					
					04071104				
T	c Leng	th	Slope	Velocity	Capacity	Description			
(min) (fee	et)	(ft/ft)	(ft/sec)	(cfs)	Doddiption			
3.0) 7	' 5	0.1867	0.42	(3.5)	Sheet Flow,			
						Grass: Short n= 0.150 P2= 3.60"			
1.3	3 11	7	0.0939	1.53		Shallow Concentrated Flow,			
						Woodland Kv= 5.0 fps			
1.5	5 23	7	0.0634	2.65	0.53	Trap/Vee/Rect Channel Flow,			
7					0.00	Bot.W=0.00' D=1.00' Z= 0.2 '/' Top.W=0.40' n= 0.030			
5.8	42	9	Total			2-0.27 Top.vv=0.40' n= 0.030			

Subcatchment 6S: Basin 1 to bypass detention



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Page 9

Summary for Pond 7P: Underground Detention System

Inflow Area =

1.150 ac, 83.48% Impervious, Inflow Depth > 2.47" for 2-Year event

Inflow

5.14 cfs @ 11.95 hrs, Volume=

0.237 af

Outflow

0.03 cfs @ 20.00 hrs, Volume=

0.024 af, Atten= 99%, Lag= 482.9 min

Primary 0.03 cfs @ 20.00 hrs, Volume=

0.024 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Peak Elev= 329.97' @ 20.00 hrs Surf.Area= 1,511 sf Storage= 9,276 cf

Plug-Flow detention time= 362.2 min calculated for 0.024 af (10% of inflow)

Center-of-Mass det. time= 159.4 min (915.4 - 756.0)

Volume	Inv	ert Avail.Sto	orage	Storage D	escription	
#1	323.8	33' 12,8	52 cf	Custom S	Stage Data (Pr	ismatic)Listed below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Inc.	Store -feet)	Cum.Store (cubic-feet)	
323.8		1,511		0	0	
331.3		1,511	11	1,333	11,333	
332.3	33	1,527	1	1,519	12,852	
Device	Routing	Invert	Outle	Devices		
#1	Primary	323.50'	15.0"	Round C	ulvert	
			L= 20	.0' RCP,	groove end w/l	neadwall, Ke= 0.200
//0			Outlet	Invert= 32	22.50' S= 0.0	500 '/' Cc= 0.900 n= 0.013
#2	Device 1	323.83'			e/Grate X 2.00	
#3	Device 1	330.33'	1.0' lc	ng x 0.5'	breadth Broa	d-Crested Rectangular Weir
			Head	(feet) 0.20	0 0.40 0.60 0	.80 1.00
			Coef.	(English)	2.80 2.92 3.0	8 3.30 3.32
#4	Device 1	331.33'	8.0' lo	ng x 0.5'	breadth Broad	d-Crested Rectangular Weir
			Head	(feet) 0.20	0.40 0.60 0	.80 1.00
			Coef.	(English)	2.80 2.92 3.0	8 3.30 3.32

Primary OutFlow Max=0.03 cfs @ 20.00 hrs HW=329.97' (Free Discharge)

-1=Culvert (Passes 0.03 cfs of 17.86 cfs potential flow)

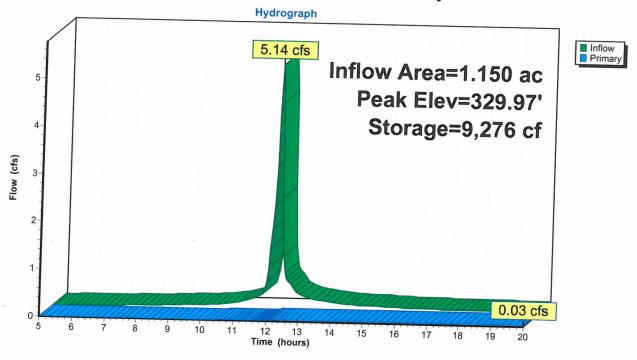
-2=Orifice/Grate (Orifice Controls 0.03 cfs @ 11.91 fps)

-3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 10

Pond 7P: Underground Detention System



Page 11

Summary for Link 8L: Basin 1 with Detention

Inflow Area =

2.760 ac, 39.13% Impervious, Inflow Depth > 0.35" for 2-Year event

Inflow

Primary

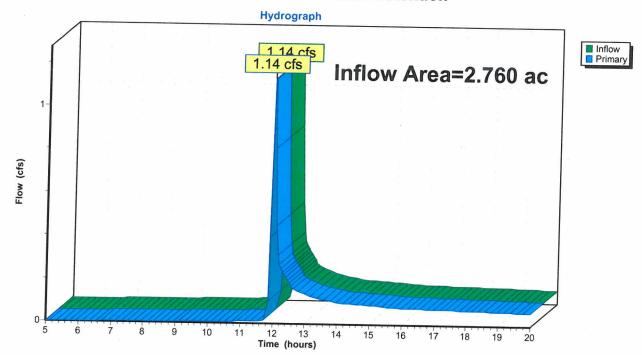
1.14 cfs @ 12.00 hrs, Volume= 1.14 cfs @ 12.00 hrs, Volume=

0.081 af

0.081 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link 8L: Basin 1 with Detention



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Runoff

Page 12

Summary for Subcatchment 5S: Basin 1 to Detention

Runoff

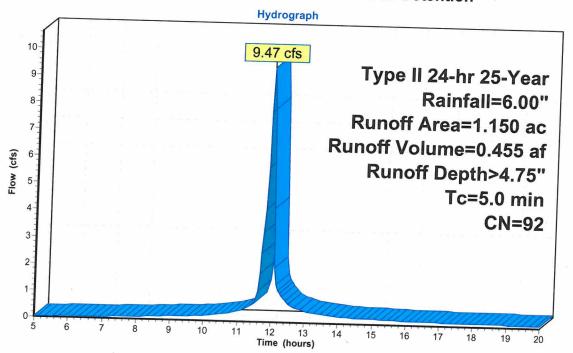
9.47 cfs @ 11.95 hrs, Volume=

0.455 af, Depth> 4.75"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 25-Year Rainfall=6.00"

Aı	rea (ac)	CN	Des	cription			
	0.960	98	Pav	ed parking	. HSG B		
- 100	0.190	61	>75	% Grass c	over, Good	d. HSG B	
	1.150 0.190 0.960		Weig 16.5	ghted Aver 2% Pervio	age		
(mir			lope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
5.	.0					Direct Entry,	

Subcatchment 5S: Basin 1 to Detention



Page 13

Summary for Subcatchment 6S: Basin 1 to bypass detention

Runoff =

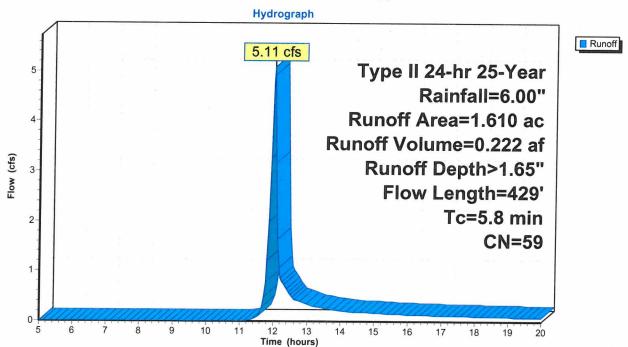
5.11 cfs @ 11.98 hrs, Volume=

0.222 af, Depth> 1.65"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 25-Year Rainfall=6.00"

Area	(ac) (CN Des	cription		
0	.120	98 Pav	ed parking	, HSG B	
0	.240			over, Good	. HSG B
1	.250		ds, Good,		
1.	.610		ghted Aver		
1.	.490		5% Pervio		
0.	.120	7.45	% Impervi	ous Area	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
3.0	75	0.1867	0.42		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.60"
1.3	117	0.0939	1.53		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
1.5	237	0.0634	2.65	0.53	Trap/Vee/Rect Channel Flow,
					Bot.W=0.00' D=1.00' Z= 0.2 '/' Top.W=0.40' n= 0.030
5.8	429	Total			*

Subcatchment 6S: Basin 1 to bypass detention



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Page 14

Summary for Pond 7P: Underground Detention System

Inflow Area =

1.150 ac, 83.48% Impervious, Inflow Depth > 4.75" for 25-Year event

Inflow =

9.47 cfs @ 11.95 hrs, Volume=

0.455 af

Outflow =

3.78 cfs @ 12.07 hrs, Volume=

0.227 af, Atten= 60%, Lag= 7.0 min

Primary

3.78 cfs @ 12.07 hrs, Volume=

0.227 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Peak Elev= 331.39' @ 12.07 hrs Surf.Area= 1,512 sf Storage= 11,429 cf

Plug-Flow detention time= 172.5 min calculated for 0.226 af (50% of inflow)

Center-of-Mass det. time= 89.2 min (832.8 - 743.6)

Volume	Inve	ert Avail.Sto	rage Storag	e Description	
#1	323.8	33' 12,8	52 cf Custo	m Stage Data (P	rismatic)Listed below (Recalc)
Elevation	- 10.1	Surf.Area	Inc.Store	Cum.Store	
(fee		(sq-ft)	(cubic-feet)	(cubic-feet)	
323.8	33	1,511	0	0	
331.3	33	1,511	11,333	11,333	
332.3	33	1,527	1,519	12,852	
Device	Routing	Invert	Outlet Devic	es	
#1	Primary	323.50'	15.0" Roun	d Culvert	
			L= 20.0' RC	CP, groove end w	/headwall, Ke= 0.200
			Outlet Invert	= 322.50' S= 0.0	500 '/' Cc= 0.900 n= 0.013
#2	Device 1	323.83'		rifice/Grate X 2.0	
#3	Device 1	330.33'			ad-Crested Rectangular Weir
			Head (feet)	0.20 0.40 0.60 (0.80 1.00
				sh) 2.80 2.92 3.0	
#4	Device 1	331.33'			d-Crested Rectangular Weir
			Head (feet)	0.20 0.40 0.60 (0.80 1.00
				h) 2.80 2.92 3.0	

Primary OutFlow Max=3.43 cfs @ 12.07 hrs HW=331.34' (Free Discharge)

-1=Culvert (Passes 3.43 cfs of 19.84 cfs potential flow)

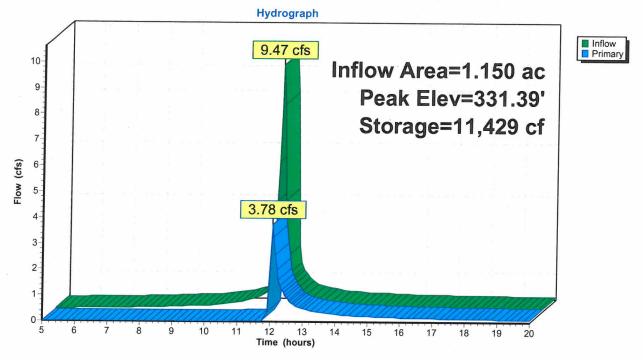
2=Orifice/Grate (Orifice Controls 0.04 cfs @ 13.18 fps)

—3=Broad-Crested Rectangular Weir (Weir Controls 3.37 cfs @ 3.34 fps)
—4=Broad-Crested Rectangular Weir (Weir Controls 0.02 cfs @ 0.28 fps)

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Page 15

Pond 7P: Underground Detention System



Page 16

Summary for Link 8L: Basin 1 with Detention

Inflow Area =

2.760 ac, 39.13% Impervious, Inflow Depth > 1.95" for 25-Year event

Inflow =

6.90 cfs @ 12.03 hrs, Volume=

0.449 af

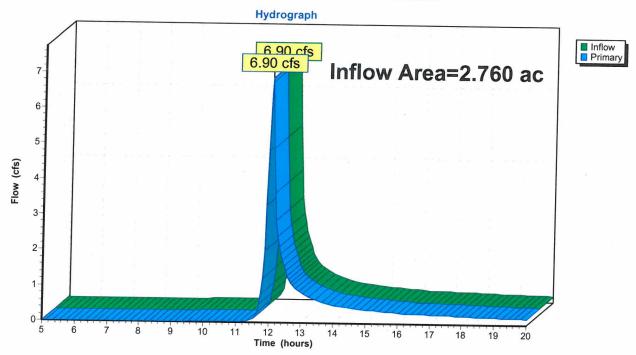
Primary =

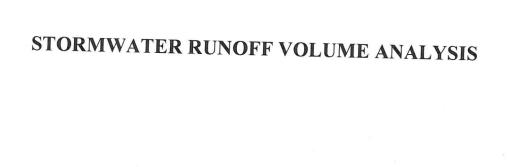
6.90 cfs @ 12.03 hrs, Volume=

0.449 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link 8L: Basin 1 with Detention







Project Name: Columbia Street Annex

Project Number: 1306

By: Date: TRM 04.25.14

Revised:

Date:

Runoff Volume Analysis Summary

Drainage Basin 1	Tc (min)	Area (ac)	CN	V2 (cf)
Pre-Development	6.5	2.75	57	3,528
Post Development	5.8	2.75	70	10,629
Post-Development with Treatment				3,528
Percent difference between Pre- Development and Post Development with Treatment				0%

Drainage Basin 2	Tc (min)	Area (ac)	CN	V2 (cf)
Pre-Development	5	1.06	55	1,133
Post Development	5	1.06	55	1,133
Percent difference between Pre- Development and Post Development	æ			0%



Basin 1 - Predevelopment



Basin 1 - Postdevelopment









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Type II 24-hr 2-Year Rainfall=3.50" Printed 4/28/2014

Prepared by {enter your company name here}
HydroCAD® 9.00 s/n 02109 © 2009 HydroCAD Software Solutions LLC

Page 1

Summary for Subcatchment 1S: Basin 1 - Predevelopment

Runoff =

1.40 cfs @ 12.01 hrs, Volume=

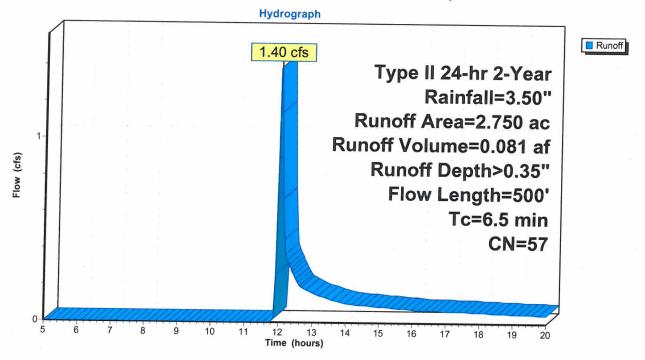
0.081 af, Depth> 0.35"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

_	Area	(ac)	CN Des	cription		
	0	.020	98 Unc	onnected	pavement,	HSG B
	0	.720	61 >75	% Grass c	over, Good	I, HSG B
	2	.010	55 Woo	ods, Good,	HSG B	
	2.	750	57 Wei	ghted Avei	rage	
	2.	730		7% Pervio		
	0.	.020	0.73	% Impervi	ous Area	
	0.	020		00% Unco		
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	3.3	75	0.1467	0.38		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.60"
	0.9	96	0.0625	1.75		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.8	92	0.1415	1.88		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	1.5	237	0.0634	2.65	0.53	Trap/Vee/Rect Channel Flow,
						Bot.W=0.00' D=1.00' Z= 0.2 '/' Top.W=0.40'
-						n= 0.030 Earth, grassed & winding
	6.5	500	Total			

Page 2

Subcatchment 1S: Basin 1 - Predevelopment



Page 4

Summary for Subcatchment 3S: Basin 1 - Postdevelopment

Runoff

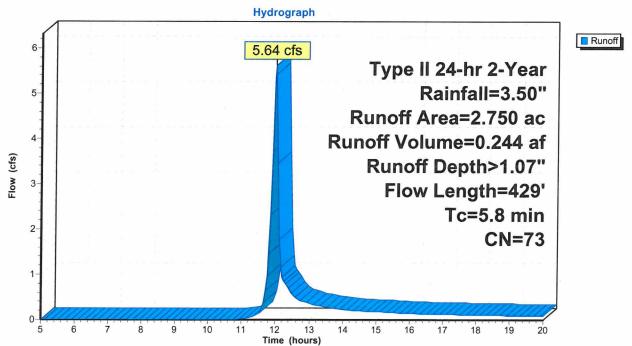
5.64 cfs @ 11.98 hrs, Volume=

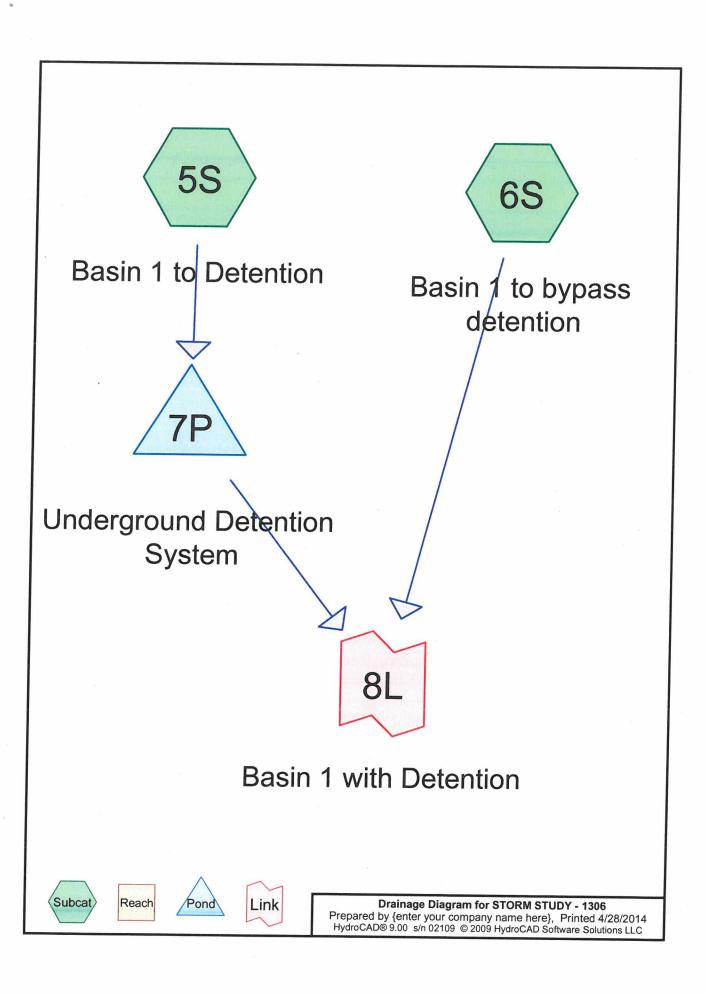
0.244 af, Depth> 1.07"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

	Area	(ac) (CN Des	cription		
1.070 98 Paved pa					, HSG B	
	0.	.430	61 >75	% Grass c	over, Good	, HSG B
	1.	.250	55 Woo	ds, Good,	HSG B	• • • • • • • • • • • • • • • • • • • •
	2.	750	73 Wei	ghted Aver	age	
	1.	680		9% Pervio		
	1.	070	38.9	1% Impen	vious Area	
						v ·
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	3.0	75	0.1867	0.42		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.60"
	1.3	117	0.0939	1.53		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	1.5	237	0.0634	2.65	0.53	Trap/Vee/Rect Channel Flow,
			-			Bot.W=0.00' D=1.00' Z= 0.2 '/' Top.W=0.40' n= 0.030
	5.8	429	Total			

Subcatchment 3S: Basin 1 - Postdevelopment





Page 2

Summary for Subcatchment 5S: Basin 1 to Detention

Runoff

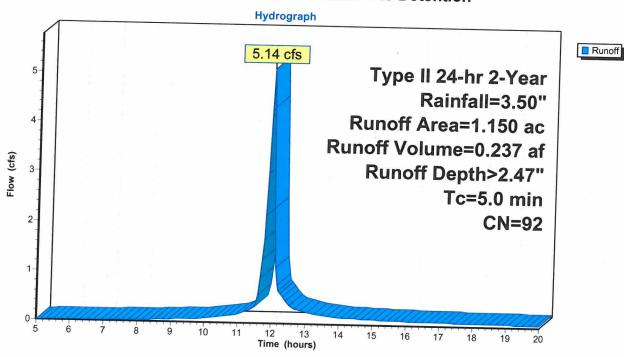
5.14 cfs @ 11.95 hrs, Volume=

0.237 af, Depth> 2.47"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

Area	a (ac)	CN	Des	cription		
	0.960	98	Pave	ed parking	. HSG B	
().190	61	>759	% Grass c	over, Good	d. HSG B
C	1.150 92 Weighted Average 0.190 16.52% Pervious Area 0.960 83.48% Impervious Area					
Tc (min)	Lengtl (feet		lope ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0						Direct Entry,

Subcatchment 5S: Basin 1 to Detention



Page 3

Summary for Subcatchment 6S: Basin 1 to bypass detention

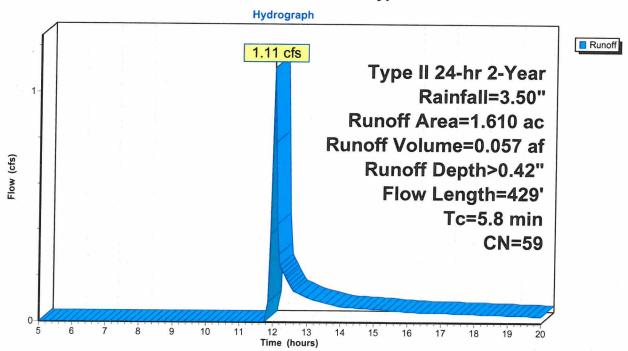
Runoff = 1.11 cfs @ 12.00 hrs, Volume=

0.057 af, Depth> 0.42"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

Area (ac) CN Description						
0.120 98 Paved parking, HSG B					. HSG B	
	0	240			over, Good	HSC B
	_					, 1100 D
-				ds, Good,	HSG B	
	1.	.610	59 Weig	ghted Aver	age	
	1.	490	92.5	5% Pervio	us Area	
	0	120		% Impervi		
	0.	120	7.40	70 IIIIpervi	ous Alea	
	т.	مالمت منام	01		0 "	B
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	3.0	75	0.1867	0.42		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.60"
	1.3	117	0.0939	1.53		TO STANDARD WAR AND A KIND DATE OF THE STANDARD WAS A STANDARD WAS
	1.0	117	0.0939	1.55		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	1.5	237	0.0634	2.65	0.53	Trap/Vee/Rect Channel Flow,
						Bot.W=0.00' D=1.00' Z= 0.2'/' Top.W=0.40' n= 0.030
	5.8	429	Total			
	0.0	723	iolai			

Subcatchment 6S: Basin 1 to bypass detention



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Page 4

Summary for Pond 7P: Underground Detention System

Inflow Area =

1.150 ac, 83.48% Impervious, Inflow Depth > 2.47" for 2-Year event

Inflow

5.14 cfs @ 11.95 hrs, Volume=

0.237 af

Outflow = 0.03 cfs @ 20.00 hrs, Volume=

0.024 af, Atten= 99%, Lag= 482.9 min

Primary

0.03 cfs @ 20.00 hrs, Volume=

0.024 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Peak Elev= 329.97' @ 20.00 hrs Surf.Area= 1,511 sf Storage= 9,276 cf

Plug-Flow detention time= 362.2 min calculated for 0.024 af (10% of inflow) Center-of-Mass det. time= 159.4 min (915.4 - 756.0)

Volume	Invert	Avail.Storage	Storage Description
#1	323.83'	12,852 cf	Custom Stage Data (Prismatic)Listed below (Recalc)
Elevation	Surf.Ar	rea Inc	c.Store Cum.Store

Elev	/ation	Surf.Area	Inc.Store	Cum.Store
	(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
3	23.83	1,511	0	0
33	31.33	1,511	11,333	11,333
33	32.33	1,527	1,519	12,852

Device	Routing	Invert	Outlet Devices
#1	Primary	323.50'	15.0" Round Culvert
			L= 20.0' RCP, groove end w/headwall, Ke= 0.200
			Outlet Invert= 322.50' S= 0.0500 '/' Cc= 0.900 n= 0.013
#2	Device 1	323.83'	0.5" Vert. Orifice/Grate X 2.00 C= 0.600
#3	Device 1	330.33'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#4	Device 1	331.33'	8.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=0.03 cfs @ 20.00 hrs HW=329.97' (Free Discharge)

-1=Culvert (Passes 0.03 cfs of 17.86 cfs potential flow)

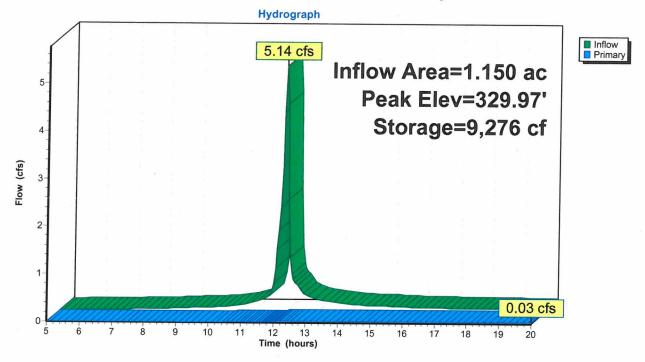
-2=Orifice/Grate (Orifice Controls 0.03 cfs @ 11.91 fps)

-3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 5

Pond 7P: Underground Detention System



Page 6

Summary for Link 8L: Basin 1 with Detention

Inflow Area =

2.760 ac, 39.13% Impervious, Inflow Depth > 0.35" for 2-Year event

Inflow

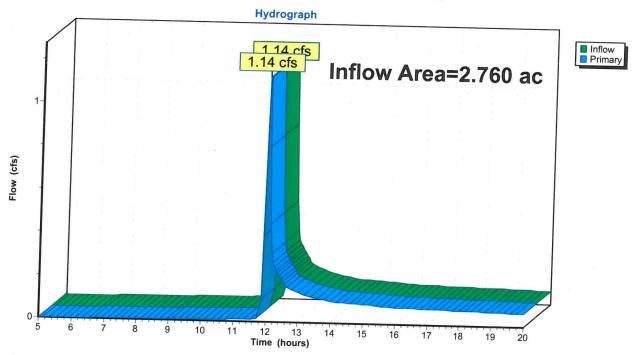
0.081 af

1.14 cfs @ 12.00 hrs, Volume= 1.14 cfs @ 12.00 hrs, Volume= Primary

0.081 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link 8L: Basin 1 with Detention





Basin 2 - Predevelopment



Basin 2 - Postdevelopment









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Page 2

Summary for Subcatchment 2S: Basin 2 - Predevelopment

Runoff

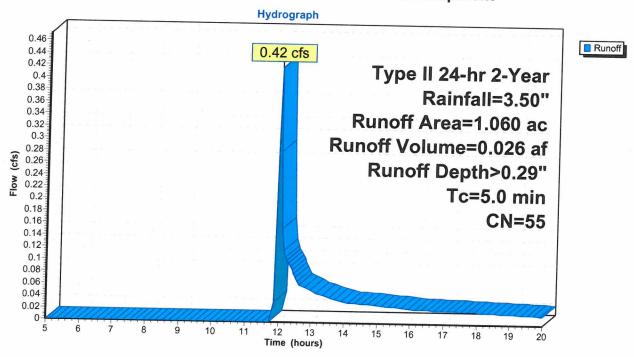
0.42 cfs @ 12.00 hrs, Volume=

0.026 af, Depth> 0.29"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

Ar	ea (ac)	CN	Des	cription					
	0.030 61 >75% Grass cover, Good, HSG B								
	1.020 55 Woods, Good, HSG B								
	0.010	_80			over, Good	. HSG D			
1.060 55 Weighted Average									
	1.060		100.	00% Pervi	ous Area				
T (min) (fee	-	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.	0					Direct Entry.			

Subcatchment 2S: Basin 2 - Predevelopment



Page 3

Summary for Subcatchment 4S: Basin 2 - Postdevelopment

Runoff

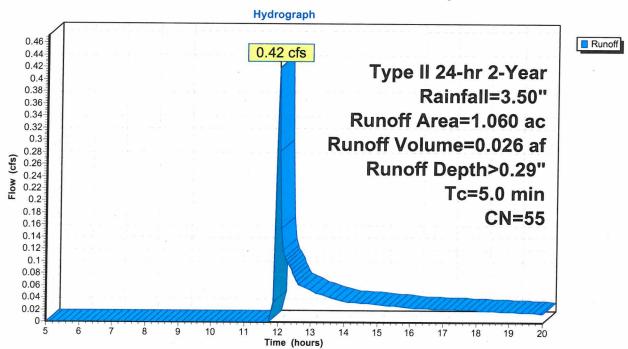
0.42 cfs @ 12.00 hrs, Volume=

0.026 af, Depth> 0.29"

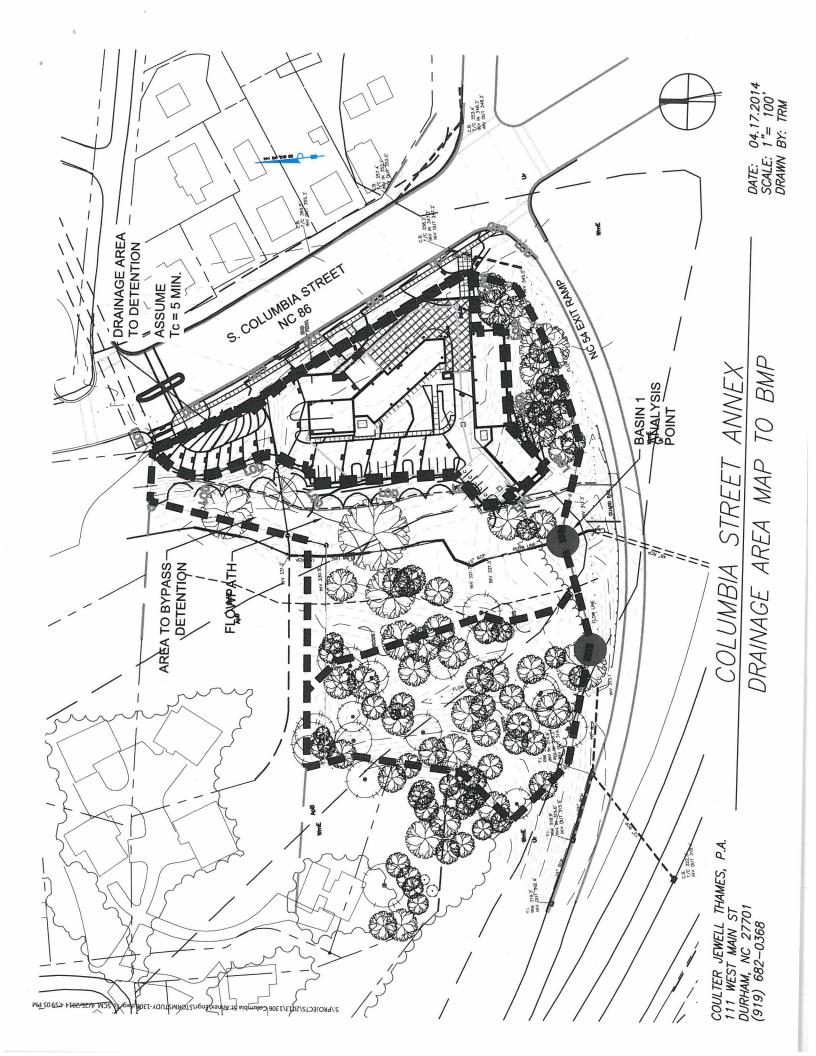
Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 2-Year Rainfall=3.50"

Area	ı (ac)	CN	Desc	cription					
(0.030	61	>75%	6 Grass co	over, Good	, HSG B			
1	.020	55	Woo	ds, Good,	HSG B				
	.010	80	>75%	6 Grass co	over, Good	, HSG D		,	
1	.060	55	Weig	hted Aver	age		0		
1	.060		100.0	00% Pervi	ous Area				
Tc	Lengt		Slope	Velocity	Capacity	Description			
(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)				
5.0						Direct Entry			

Subcatchment 4S: Basin 2 - Postdevelopment



		*	2		
		,			
UNDERG	ROUND D	ETENTI	ON FAC	CILITY D	ESIGN
		,			



Printed 4/28/2014

Page 4

Summary for Pond 7P: Underground Detention System

Inflow Area =

1.150 ac, 83.48% Impervious, Inflow Depth > 3.93" for 10-Year event

Inflow

7.92 cfs @ 11.95 hrs, Volume=

0.376 af

Outflow = 0.85 cfs @ 12.31 hrs, Volume=

0.149 af, Atten= 89%, Lag= 21.3 min

Primary

0.85 cfs @ 12.31 hrs, Volume=

0.149 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Peak Elev= 330.75' @ 12.31 hrs Surf.Area= 1,511 sf Storage= 10,461 cf

Plug-Flow detention time= 213.2 min calculated for 0.148 af (39% of inflow)

Center-of-Mass det. time= 117.0 min (863.9 - 746.9)

Volume	Inve	ert Avail.Sto	orage	Storage	e Description		
#1	323.8	3' 12,8	52 cf	Custon	n Stage Data (Pr	ismatic)Listed below (Recalc)	_
Elevation (fee		Surf.Area (sq-ft)	80 80 8	.Store c-feet)	Cum.Store (cubic-feet)		
323.	83	1,511		0	0		
331.3	33	1,511	. 1	1,333	11,333		
332.3	33	1,527		1,519	12,852		
Device	Routing	Invert	Outle	et Device	es		
#1	Primary	323.50'	15.0	' Round	d Culvert		
						headwall, Ke= 0.200	
110	D					500 '/' Cc= 0.900 n= 0.013	
#2	Device 1	323.83'			ifice/Grate X 2.00		
#3	Device 1	330.33'				d-Crested Rectangular Weir	
					0.20 0.40 0.60 0		
					n) 2.80 2.92 3.0		
#4	Device 1	331.33'				d-Crested Rectangular Weir	
			Head	(feet) 0	0.20 0.40 0.60 0	.80 1.00	

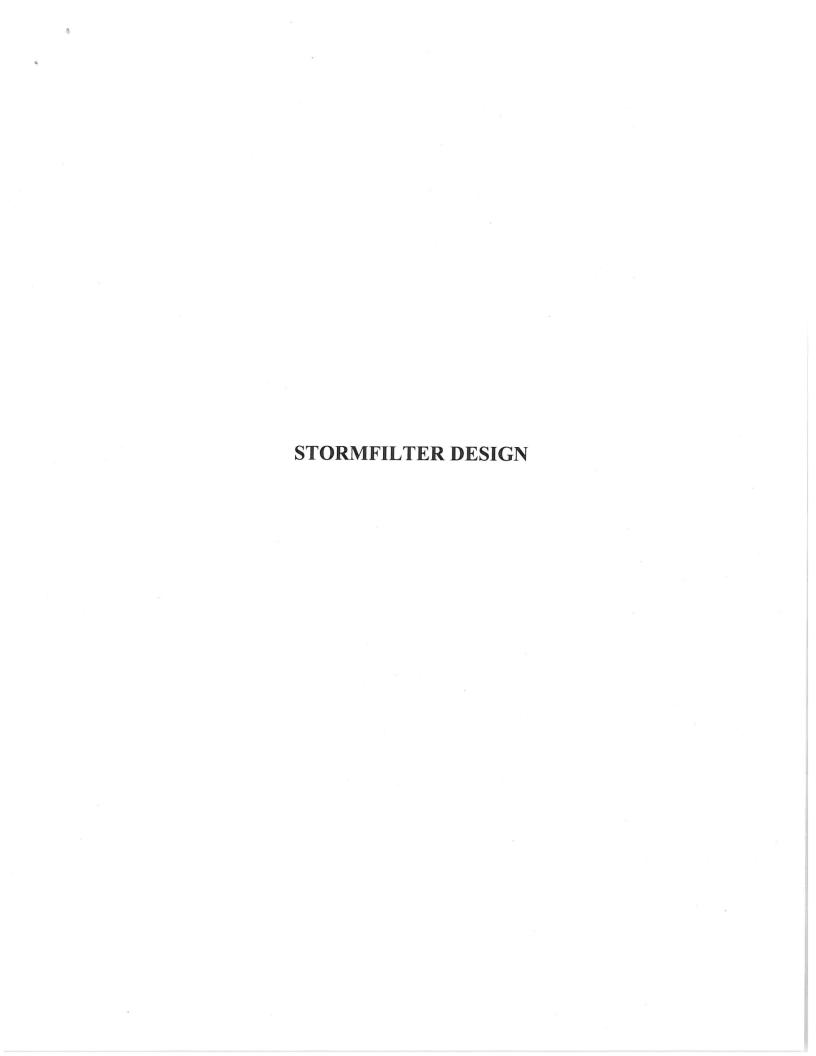
Coef. (English) 2.80 2.92 3.08 3.30 3.32

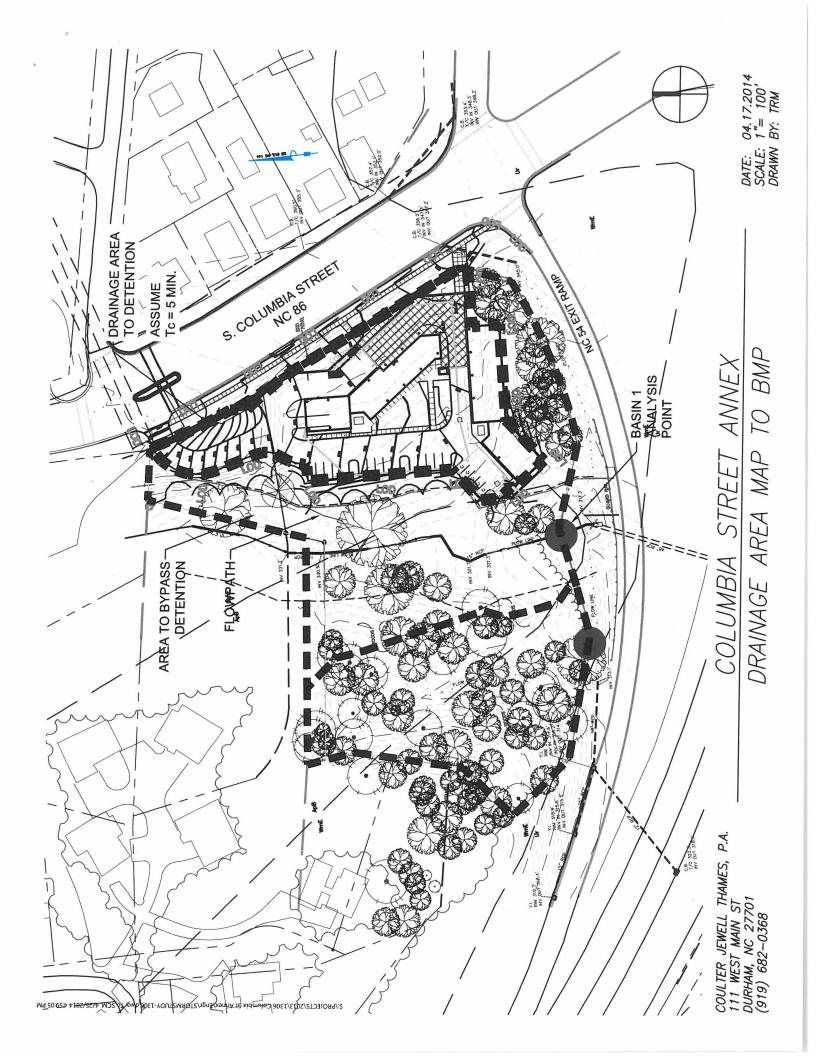
Primary OutFlow Max=0.84 cfs @ 12.31 hrs HW=330.75' (Free Discharge)

-1=Culvert (Passes 0.84 cfs of 19.02 cfs potential flow) -2=Orifice/Grate (Orifice Controls 0.03 cfs @ 12.65 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 0.81 cfs @ 1.91 fps)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)







Project Name: Columbia Street Annex

By:

TRM

Revised:

Project Number: 1306

Date: 04.25.14

Date:

TSS Reduction Summary

Threshold indicating TSS Reduc	tion is required
	Impervious Percentage
Jordan Basin	16%

Site Impervious Percentage							
Pre-Development	0%						
Post-Development	25%						

Additional Impervious to be treated =

0.96

acres

Imervious area Treated by SCMs for TSS =

0.96

acres



Project Name:

Columbia Street Annex

By:

Revised:

Project Number:

1306

Date: 04.25.14

TRM

Date:

StormFilter BMP Sizing

Drainage Area =

1.14

ac ac

Impervious Area =

0.96

Runoff Volume: Simple Method

V = 3630*Rd*Rv*A

V = Runoff Volume (cf)

Rd = Design storm rainfall depth (1.0 in)

Rv = Runoff coefficient (unitless)

A = Drainage area (ac)

Rv = 0.5 + 0.9*Ia

la = Impervious fraction

la = 0.842105263

Rv = 0.807894737

Required runoff Volume

V =

3,343 cf

min 3,630 for wetland