10-13-2021 Town Council Meeting Responses to Council Questions

ITEM #8: Close the Legislative Hearing and Consider a Conditional Zoning Application - Residence Inn and Summit Place Townhomes, 101-111 Erwin Road, Mixed Use-Village-Conditional Zoning District (MU-V-CZD)

Council Question:

Stormwater has been and continues to be a concern regarding this proposed project. Can we receive a presentation from the Town's stormwater staff laying out the concerns/issues and the proposed adequacy of the proposed solutions with particular reference to the impact on Summerfield Crossing?

Staff Response:

Stormwater staff will be participating in the staff presentation at Wednesday's meeting. A revised staff presentation with additional slides is attached.

Council Question:

Per the packet materials, can you share more as to why staff agreed to a 19.1' easement when they would like to see a 30' easement? What is the impact of that tradeoff?

Staff Response:

The applicant was hesitant to grant the full 30' easement so Town staff determined it was better to agree to the reduced easement to ensure that some measure is taken to facilitate construction of the multi-use path in the future, as designated in the Mobility and Connectivity Plan. While the path is expected to stay within the existing Dobbins Drive right of way, the potential impact of the reduced easement is that the Town may need to acquire additional easements in the future, which would add cost for any future project.

Council Question:

How wide is the existing sidewalk on Erwin?

Staff Response:

The existing sidewalk on Erwin Rd is 5' wide.

Council Question:

Will the applicant be required to provide a PIL for the future path on Dobbins?

Staff Response:

The applicant and Town staff have not discussed or agreed upon a payment-in-lieu for the future path.

10-13-2021 Town Council MeetingResponses to Council Questions

Council Question:

Can you ask the applicant to consider including secure e-bike storage in the design?

Applicant Response:

If the Council would like to include this as a stipulation, we will accept it.

- We can provide secure storage at both hotel and townhomes.
- Townhomes have space for added e-bike storage in garages or outside on "lot".
- Hotel can provide secure storage. We have looked at various storage racks [hanging, front wheel in racks, etc.]. If secure for hotel means inside, we will have some design work to do before we can ID how to put e-bike and regular bike storage in the same place. Some of the available common areas may need to be vetted with Fire Dept for corridor access, etc.

DRAFT STUDY AND ANALYSIS SWALES AT SUMMERFIELD BOUNDARY

RESIDENCE INN HOTEL – SUMMIT PLACE TOWNHOMES

Chapel Hill, North Carolina

September 16, 2021 Revised October 7, 2021



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INTRODUCTION

The Residence Inn-Summit Place townhome project is currently in the review process for a Mixed Use - Village Conditional Zoning District approval.

This study has been prepared at the request of the Town of Chapel Hill based on input from residents of the Summerfield Crossing neighborhood and analyzes stormwater runoff issues along a segment of the southwest boundary of the Residence Inn Parcel adjacent to the Summerfield Crossing neighborhood. This study only addresses potential solutions for issues at Summerfield Crossing that could be implemented on the Residence Inn parcel. Solutions outside the Residence Inn parcel that may alleviate other existing conditions are outside the scope of this study.

A draft stormwater impact statement for the proposed Residence Inn-Summit Place townhomes project has been prepared under separate cover. This analysis has been prepared as a separate, stand-alone analysis that independent of any development at the Residence Inn-Summit Place townhome sites.

BACKGROUND AND GENERAL INFORMATION

The existing townhome units at 116 and 118 Woodbridge Lane experience standing water at the front of the units during relatively small rainfall events. This study primarily analyzes potential solutions for the conditions at this location. However, locations south of this area may also receive benefits from proposed solutions.

Under current conditions, stormwater runoff from some areas of the existing Residence Inn hotel parcel flows across the hotel parcel boundary into the Summerfield Crossing neighborhood. Based on field topography in the area, there is a drainage divide between the unit at 120 Woodbridge Lane and 118 Woodbridge Lane. Generally, flow near the unit at 120 Woodbridge flows north and flow near the unit at 118 Woodbridge flows south. A topographic map based on field survey is include as an appendix to this report.

POTENTIAL SOLUTION

A swale flowing south to convey runoff parallel to the Residence Inn property boundary was studied to determine the effects on downstream and offsite properties. The swale is intended to intercept surface flow towards the southwest hotel boundary and discharge the flow towards the existing low area at the Resource Conservation Area (RCA) before flowing onto the Summerfield Crossing neighborhood property.

METHODOLOGY

Drainage Areas and SCS Curve Numbers

Drainage areas were delineated based on existing and proposed grading and calculated using CAD software to determine the area of each land use within the drainage area. Composite SCS Curve Numbers were calculated based on the values in Table 2.2 of the TOCHDM.

Drainage area maps are included in the appendix to this report.

Runoff Calculations

Runoff flowrates were calculated using Hydrology Studio software for several storm events. The software utilizes the SCS Methodology to determine peak flow rates.

Channels, Swales and Ditches

Channels were analyzed for the multiple storm events using Hydrology Studio/Channel Studio which calculates one-dimensional water surface profiles for steady, gradually varied flow in open channels for subcritical as well as supercritical flow regimes. The basic computational procedure is based on the solution of the one-dimensional energy equation. Energy losses are evaluated by friction using Manning's equation and using contraction/expansion coefficients multiplied by the change in velocity head. Flow velocities are limited to 4 feet per second in accordance with Section 2.6.2 of the TOCHDM.

Rainfall Depths

The following rainfall depths used in the calculations for this report were taken from the TOCHDM.

Rainfall event	Rainfall Depth (inches)
1-year, 24-hour	See note
2-year, 24-hour	3.60
10-year, 24-hour	5.38
25-year, 24-hour	6.41
100-year, 24-hour	8.00

Notes:

- 1. Only the 10-year and 100-year rainfall events were analyzed for runoff flowrates. Additional rainfall events are included for reference only.
- 2. The TOCHDM specifies the 1-year, 24-hour runoff to be 80% of the 2-year, 24-hour runoff and is not based on a rainfall depth.

RUNOFF ANALYSIS

Drainage area and runoff data for different conditions is summarized below. Refer to the appendix to this report for locations of each drainage area. Program output that calculates discharge is also included in the appendix to this report.

South Flowing Swale

Data for a swale to intercept surface flow that begins east of 118 Woodbridge Lane and flows south is summarized below.

Drainage Area 5 – South Flowing Swale			
	Existing conditions	To Swale	Swale Bypass
Drainage area	0.86 acres	0.65 acres	0.21 acres
SCS CN	79	80	77
Time of concentration	5.0 minutes	5.0 minutes	5.0 minutes
Q10	4.3 cfs	3.3 cfs	1.0 cfs
Q100	7.5 cfs	5.7 cfs	1.7 cfs

Flow towards 118 Woodbridge Lane

Data for the area that flows towards 118 Woodbridge Lane is summarized below. The data noted for the Swale Bypass assumes construction of the south flowing swale intercepts surface flow toward 118 Woodbridge Lane

Drainage Area 7 – Flow Towards 118 Woodbridge Lane			
	Existing Conditions	Swale Bypass	
Drainage area	0.12 acres	0.07 acres	
SCS CN	77	77	
Time of concentration	5.0 minutes	5.0 minutes	
Q10	0.6 cfs	0.4 cfs	
Q100	1.0 cfs	0.7 cfs	

SWALE ANALYSIS

Data from the Runoff Analysis was used for calculations related to the swale that may be used to intercept flow from crossing portions of the Residence Inn hotel boundary. Data the swale is summarized in the table below. Program output that calculates parameters in the swale is included in the appendix to this report.

South Flowing Swale

South Flowing Swale			
	Q10	Q100	
Discharge to swale	3.3 cfs	5.7 cfs	
Flow depth	0.4 feet	0.5 feet	
Flow velocity	2.9 ft/sec	3.4 ft/sec	
Swale bottom width	2.0 feet	·	
Swale depth	1.5 feet		
Longitudinal swale slope	2.0%		
Swale side slopes	3:1		
Manning's n	0.030		

CONCLUSION

Based on the results of this analysis, we believe that construction of a swale along the southwestern border between the Residence Inn hotel and Summerfield Crossing neighborhood will reduce the flow across the property boundary, including reducing flow towards the front of the townhome located at 118 Woodbridge Lane. Additional details for this analysis can be found in the Appendix.

APPENDICES

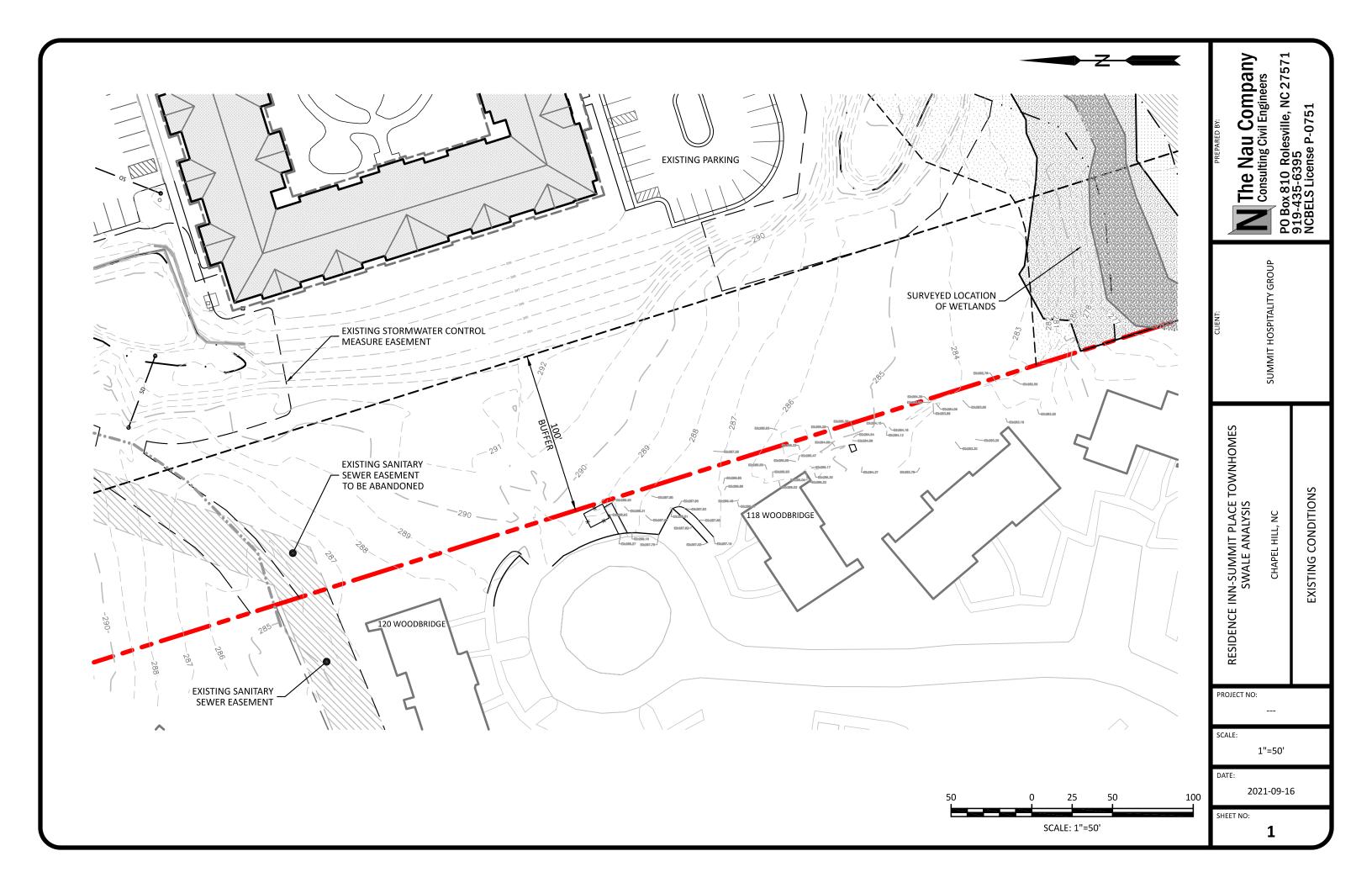
Appendix A – Existing Topography

Appendix B – Drainage Area Maps

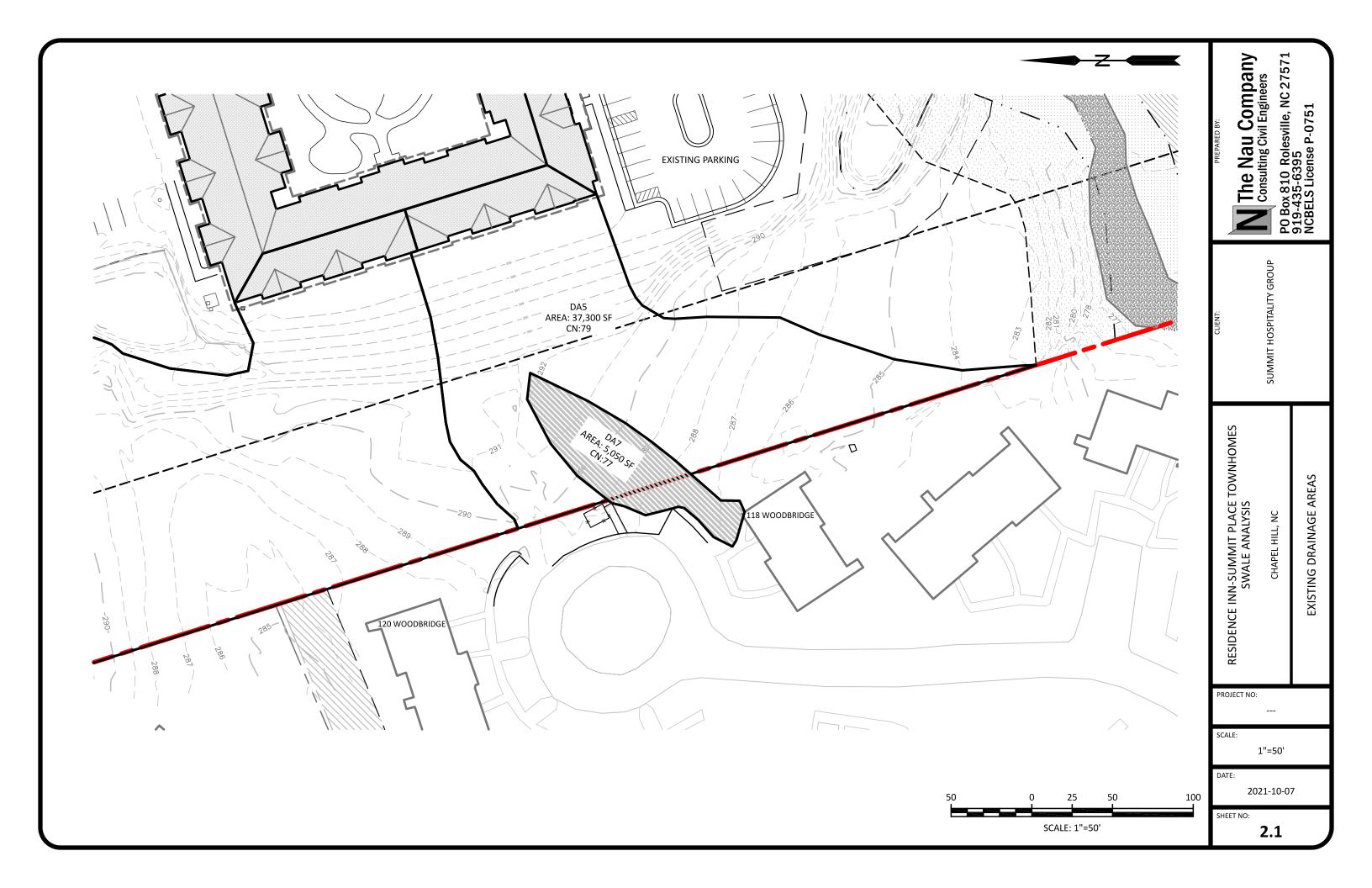
Appendix C – Runoff Calculations

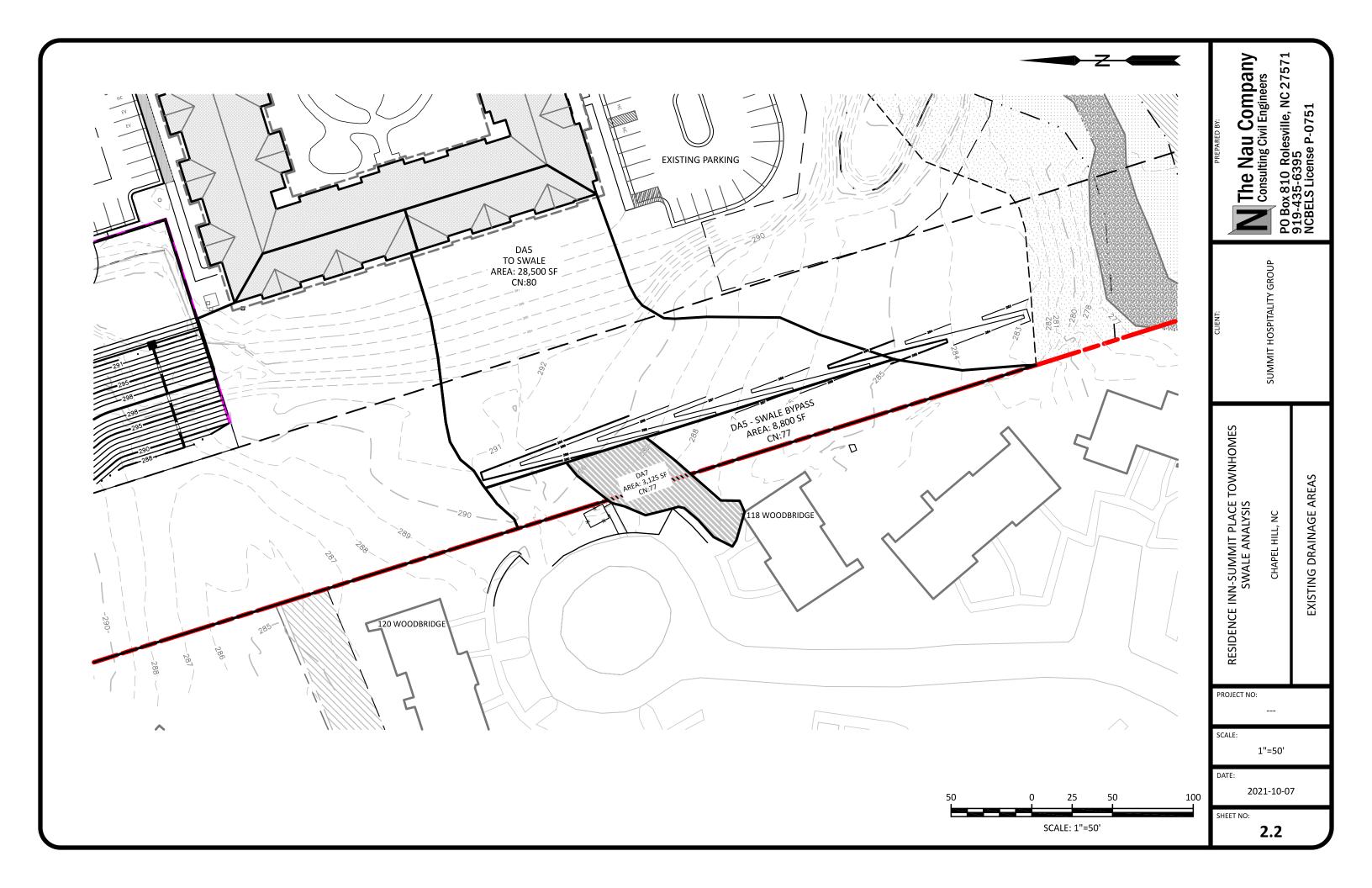
Appendix D – Swale Calculations

APPENDIX A EXISTING TOPOGRAPHY



APPENDIX B DRAINAGE AREA MAPS

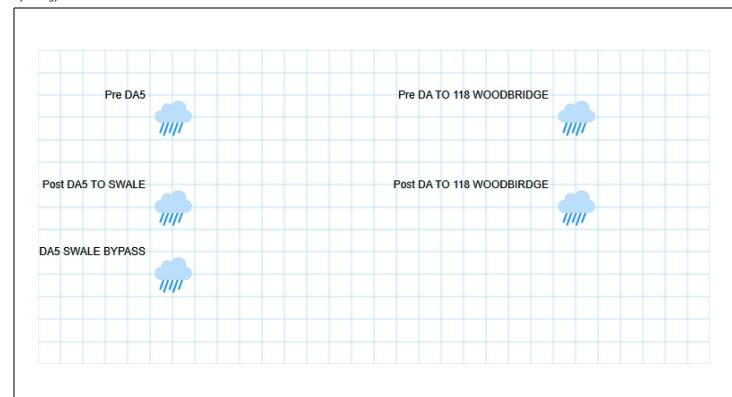




APPENDIX C RUNOFF CALCULATIONS

Basin Model

10-07-2021



Project Name:

Hydrograph by Return Period

10-07-2021 Hydrology Studio v 3.0.0.20 Peak Outflow (cfs) Hyd. Hydrograph Hydrograph No. Type Name 1-yr 2-yr 3-yr 5-yr 10-yr 25-yr 50-yr 100-yr NRCS Runoff Pre DA5 1.705 2.409 4.341 5.526 7.450 1 2 NRCS Runoff Post DA5 TO SWALE 1.362 1.903 3.377 4.281 5.738 0.371 0.537 1.749 3 NRCS Runoff Post DA5 SWALE BYPASS 0.998 1.284 NRCS Runoff Pre DA TO 118 WOODBRIDGE 0.215 0.310 0.738 1.004 0.575 4 5 NRCS Runoff Post DA TO 118 WOODBIRDGE 0.145 0.208 0.385 0.494 0.671

Project Name:

Hydrograph 1-yr Summary Hydrology Studio v 3.0.0.20

10-07-2021

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	Pre DA5	1.705	11.97	3,429			
2	NRCS Runoff	Post DA5 TO SWALE	1.362	11.97	2,735			
3	NRCS Runoff	Post DA5 SWALE BYPASS	0.371	11.97	751			
4	NRCS Runoff	Pre DA TO 118 WOODBRIDG	E 0.215	11.97	435			
			E 0.215					

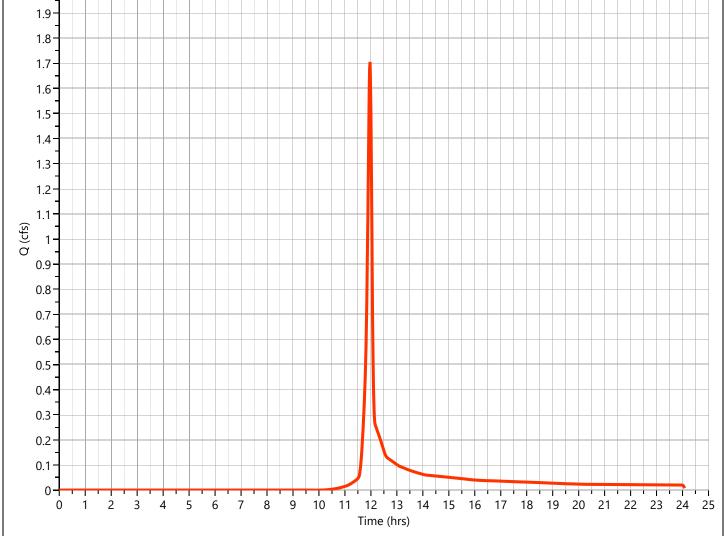
Pre DA5 Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.705 cfs
Storm Frequency	= 1-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 3,429 cuft
Drainage Area	= 0.86 ac	Curve Number	= 78.99*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 2.83 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

0.65 0.86	77 79	Woods Weighted CN Method Employed
0.15	80	Grass/open
0.06	98	Impervious
AREA (ac)	CN	DESCRIPTION





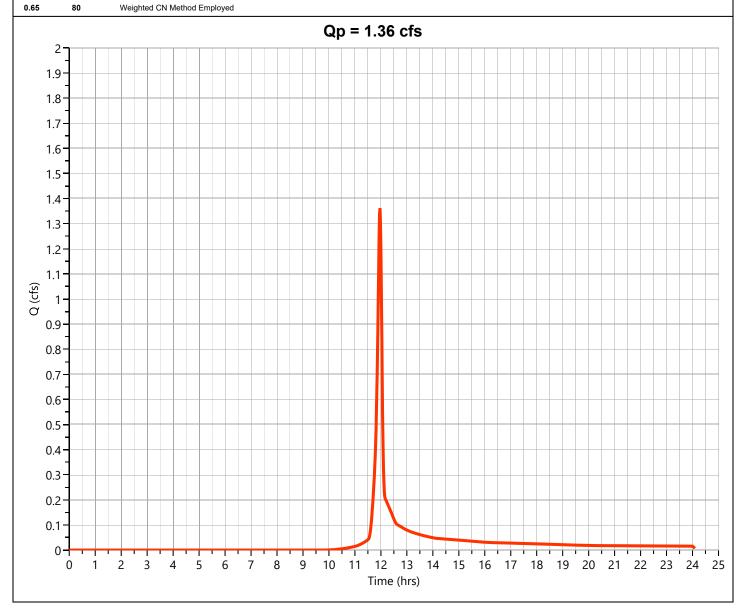
Post DA5 TO SWALE

Hyd. No. 2

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.362 cfs
Storm Frequency	= 1-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 2,735 cuft
Drainage Area	= 0.65 ac	Curve Number	= 80*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 2.83 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac)	CN	DESCRIPTION
0.06	98	Impervious
0.15	80	Grass/open
0.44	77	Woods
0.65	80	Weighted CN Met



Post DA5 SWALE BYPASS

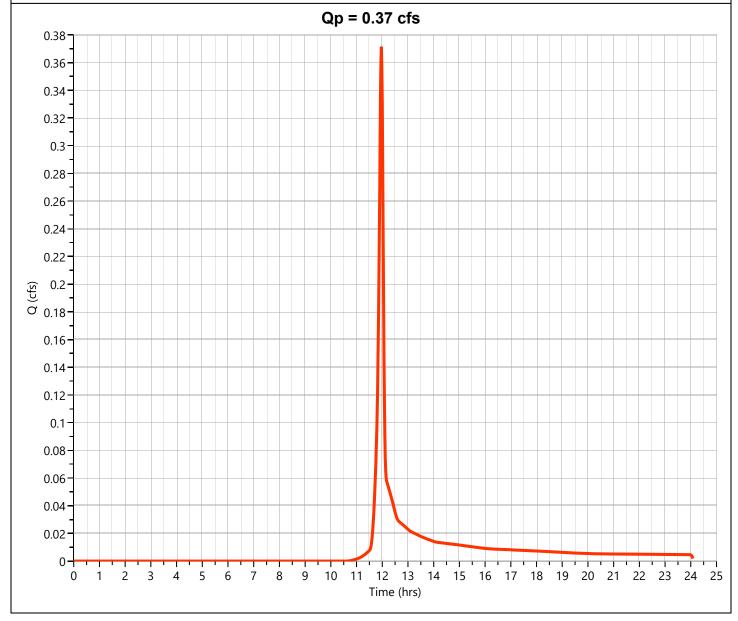
Hyd. No. 3

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.371 cfs
Storm Frequency	= 1-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 751 cuft
Drainage Area	= 0.21 ac	Curve Number	= 77*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 2.83 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac) CN DESCRIPTION 0.21 77 Woods

0.21 77 Weighted CN Method Employed



Pre DA TO 118 WOODBRIDGE

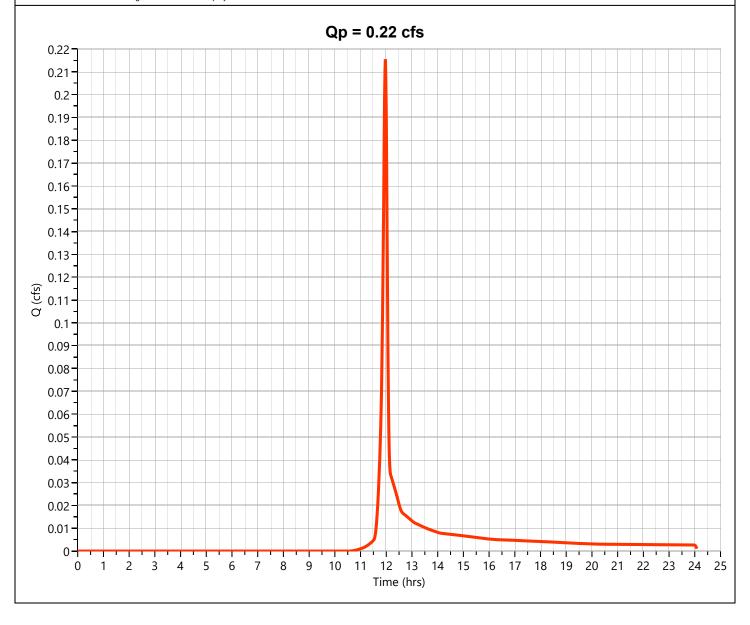
Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.215 cfs
Storm Frequency	= 1-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 435 cuft
Drainage Area	= 0.12 ac	Curve Number	= 77.25*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 2.83 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac) CN DESCRIPTION
0.11 77 Woods
0.01 80 Grass/open

0.12 77 Weighted CN Method Employed



Post DA TO 118 WOODBIRDGE

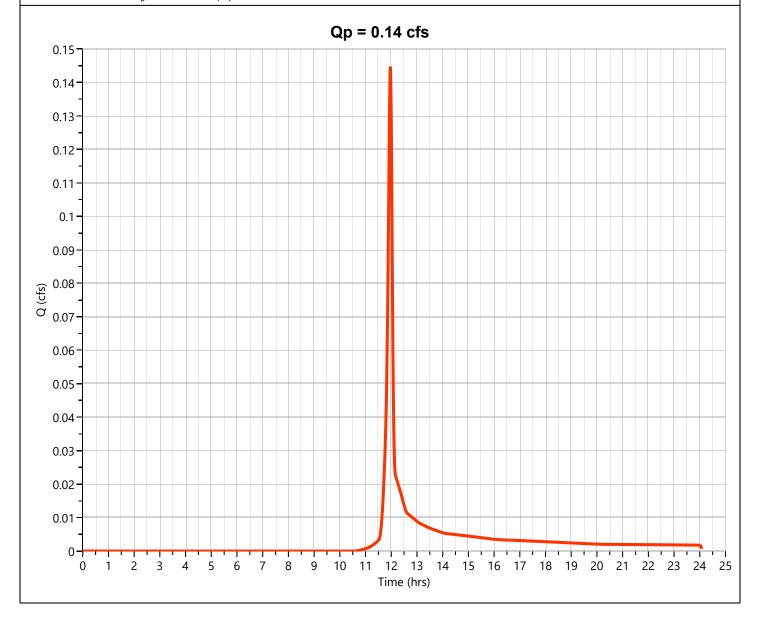
Hyd. No. 5

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.145 cfs
Storm Frequency	= 1-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 292 cuft
Drainage Area	= 0.08 ac	Curve Number	= 77.38*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 2.83 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac) CN DESCRIPTION 0.07 77 Woods 0.01 80 Grass/open

0.08 77 Weighted CN Method Employed



Project Name:

Hydrograph 2-yr Summary Hydrology Studio v 3.0.0.20

10-07-2021

yarology Sil	ıdio v 3.0.0.20							10-07-202
Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	Pre DA5	2.409	11.97	4,840			
2	NRCS Runoff	Post DA5 TO SWALE	1.903	11.97	3,828			
3	NRCS Runoff	Post DA5 SWALE BYPASS	0.537	11.97	1,078			
4	NRCS Runoff	Pre DA TO 118 WOODBRIDG	E 0.310	11.97	623			
			E 0.310					

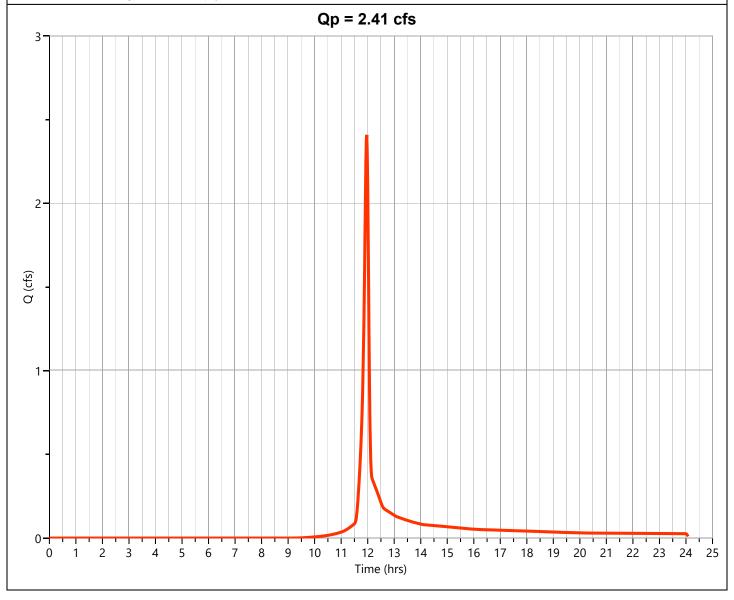
Pre DA5 Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 2.409 cfs
Storm Frequency	= 2-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 4,840 cuft
Drainage Area	= 0.86 ac	Curve Number	= 78.99*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 3.42 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac)	CN	DESCRIPTION
0.06	98	Impervious
0.15	80	Grass/open
0.65	77	Woods
0.00	70	Weighted CN Method

0.86 79 Weighted CN Method Employed



Post DA5 TO SWALE

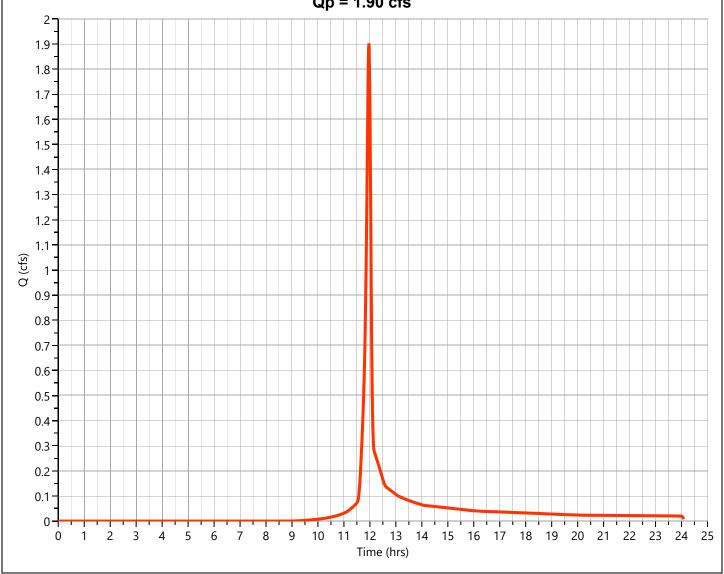
Hyd. No. 2

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.903 cfs
Storm Frequency	= 2-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 3,828 cuft
Drainage Area	= 0.65 ac	Curve Number	= 80*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 3.42 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

0.65	80	Weighted CN Method Employed
0.44	77	Woods
0.15	80	Grass/open
0.06	98	Impervious
AREA (ac)	CN	DESCRIPTION

Qp = 1.90 cfs



Post DA5 SWALE BYPASS

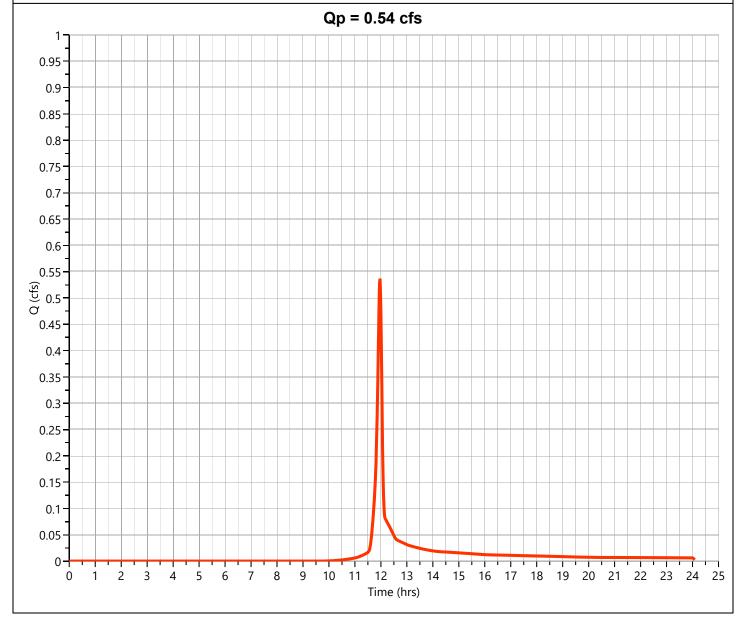
Hyd. No. 3

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.537 cfs
Storm Frequency	= 2-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 1,078 cuft
Drainage Area	= 0.21 ac	Curve Number	= 77*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 3.42 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac) CN DESCRIPTION 0.21 77 Woods

0.21 77 Weighted CN Method Employed



Pre DA TO 118 WOODBRIDGE

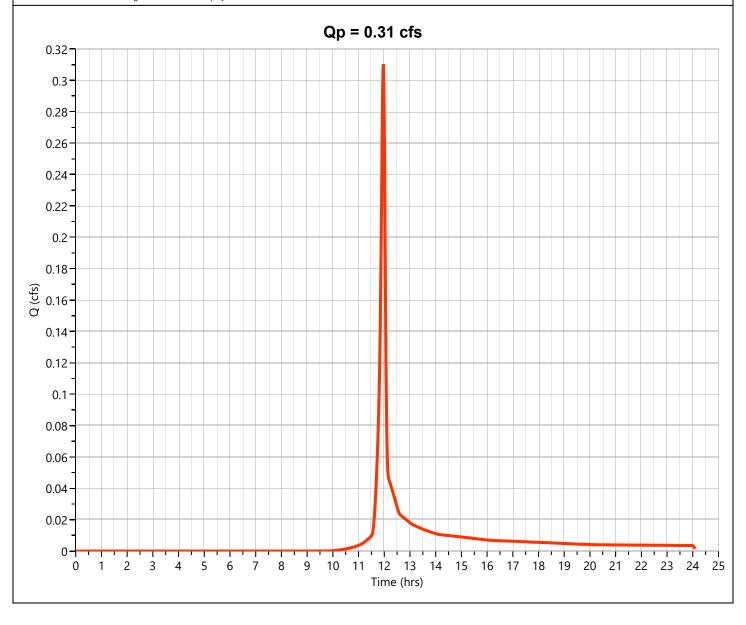
Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.310 cfs
Storm Frequency	= 2-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 623 cuft
Drainage Area	= 0.12 ac	Curve Number	= 77.25*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 3.42 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac)	CN	DESCRIPTION
0.11	77	Woods
0.01	80	Grass/open

0.12 77 Weighted CN Method Employed



Post DA TO 118 WOODBIRDGE

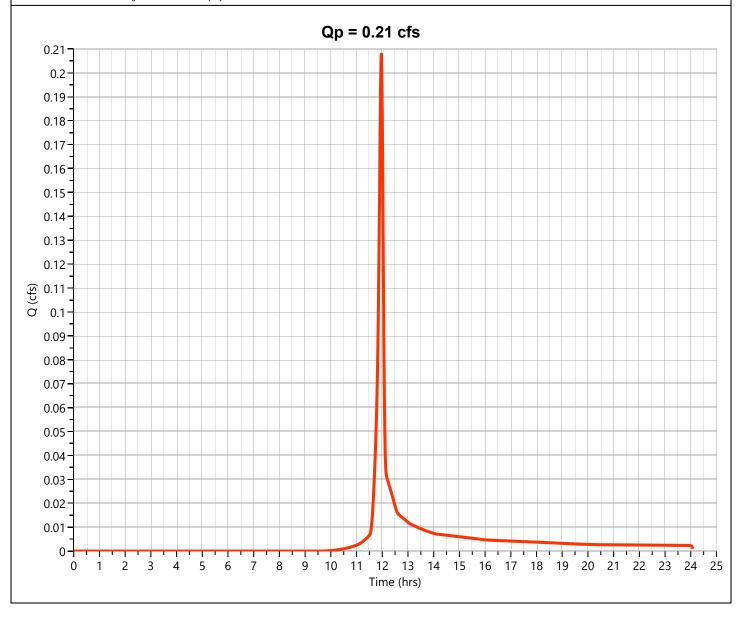
Hyd. No. 5

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.208 cfs
Storm Frequency	= 2-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 418 cuft
Drainage Area	= 0.08 ac	Curve Number	= 77.38*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 3.42 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac) CN DESCRIPTION 0.07 77 Woods 0.01 80 Grass/open

0.08 77 Weighted CN Method Employed



Project Name:

Hydrograph 10-yr Summary Hydrology Studio v 3.0.0.20

10-07-2021

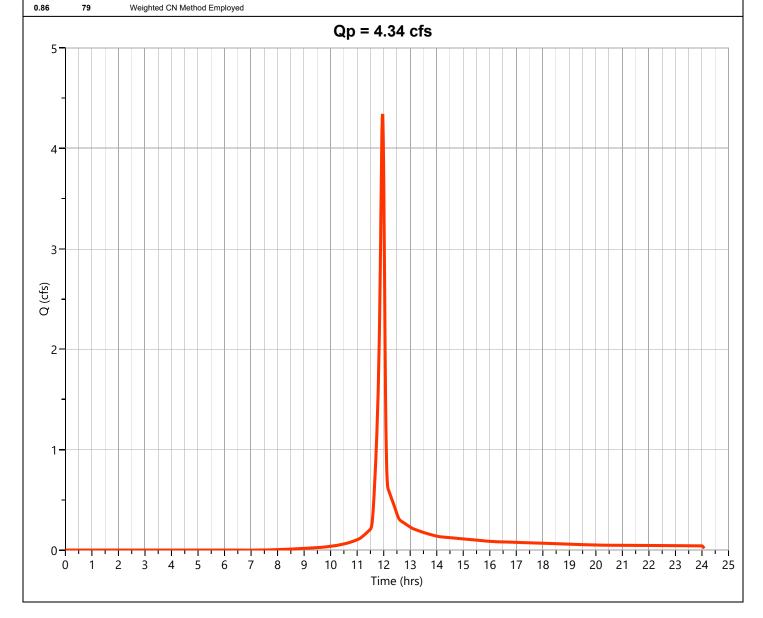
Hyd. No. 1 2 3 4 5	Hydrograph Type NRCS Runoff NRCS Runoff NRCS Runoff NRCS Runoff NRCS Runoff	Hydrograph Name Pre DA5 Post DA5 TO SWALE Post DA5 SWALE BYPASS Pre DA TO 118 WOODBRIDG Post DA TO 118 WOODBIRDO		Time to Peak (hrs) 11.97 11.97 11.97 11.97 11.97	Hydrograph Volume (cuft) 8,851 6,912 2,023 1,166 780	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
2 3 4	NRCS Runoff NRCS Runoff NRCS Runoff	Post DA5 TO SWALE Post DA5 SWALE BYPASS Pre DA TO 118 WOODBRIDG	3.377 0.998 ≡ 0.575	11.97 11.97 11.97	6,912 2,023 1,166			
3 4	NRCS Runoff NRCS Runoff	Post DA5 SWALE BYPASS Pre DA TO 118 WOODBRIDG	0.998 ≣ 0.575	11.97 11.97	2,023 1,166			
4	NRCS Runoff	Pre DA TO 118 WOODBRIDG	€ 0.575	11.97	1,166			
5	NRCS Runoff	Post DA TO 118 WOODBIRD C	6E0.385	11.97	780			

Pre DA5 Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 4.341 cfs
Storm Frequency	= 10-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 8,851 cuft
Drainage Area	= 0.86 ac	Curve Number	= 78.99*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 4.94 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac)	CN	DESCRIPTION
0.06	98	Impervious
0.15	80	Grass/open
0.65	77	Woods
		147 : 1 4 1 0 1 1 4 1



Post DA5 TO SWALE

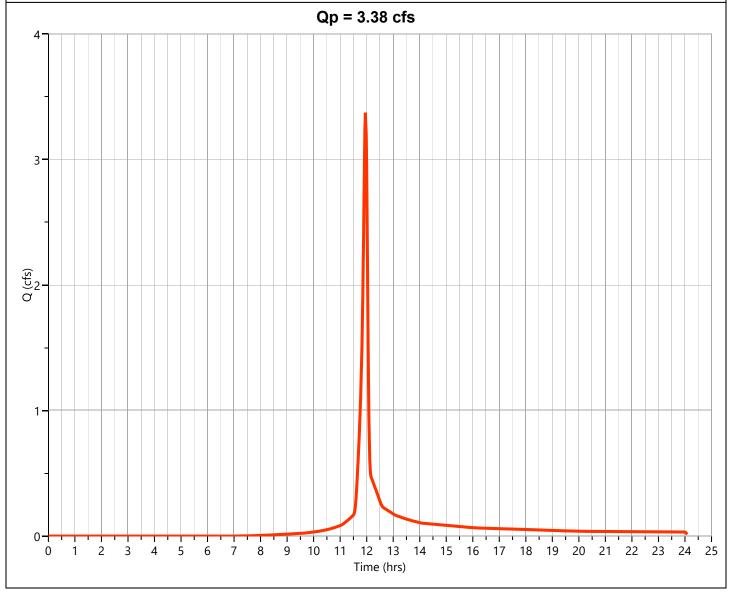
Hyd. No. 2

Hydrograph Type	ograph Type = NRCS Runoff		= 3.377 cfs
Storm Frequency	= 10-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 6,912 cuft
Drainage Area	= 0.65 ac	Curve Number	= 80*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 4.94 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac)	CN	DESCRIPTION
0.06	98	Impervious
0.15	80	Grass/open
0.44	77	Woods
0.65	80	Weighted CN Meth

0.65 80 Weighted CN Method Employed



Post DA5 SWALE BYPASS

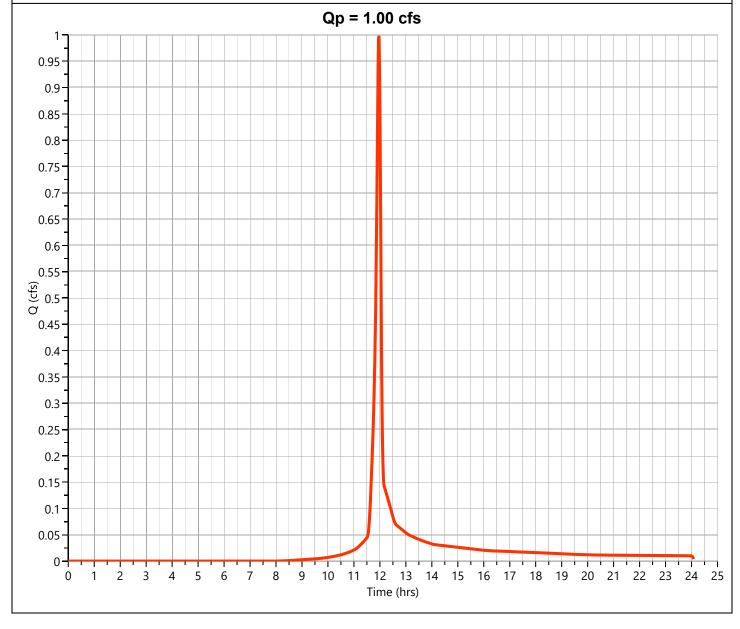
Hyd. No. 3

Hydrograph Type	graph Type = NRCS Runoff		= 0.998 cfs
Storm Frequency	= 10-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 2,023 cuft
Drainage Area	= 0.21 ac	Curve Number	= 77*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 4.94 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac) CN DESCRIPTION 0.21 77 Woods

0.21 77 Weighted CN Method Employed



Pre DA TO 118 WOODBRIDGE

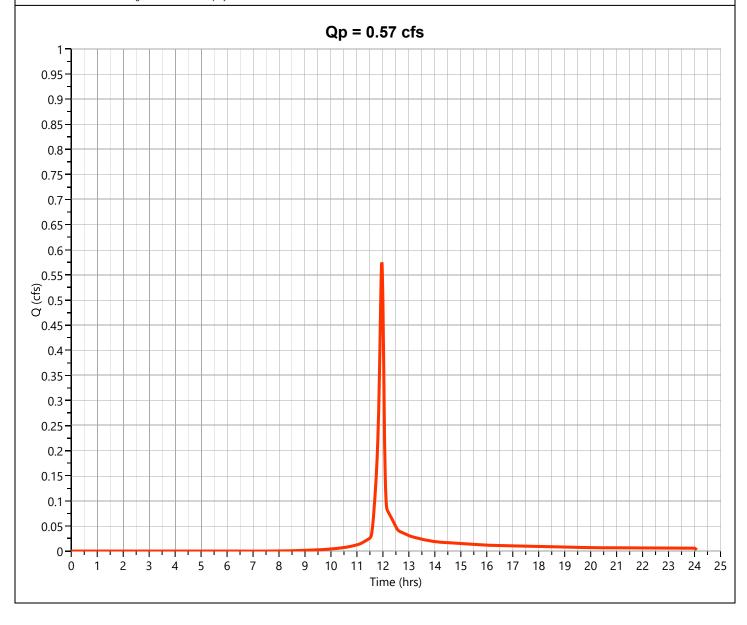
Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.575 cfs
Storm Frequency	= 10-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 1,166 cuft
Drainage Area	= 0.12 ac	Curve Number	= 77.25*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 4.94 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac) CN DESCRIPTION
0.11 77 Woods
0.01 80 Grass/open

0.12 77 Weighted CN Method Employed



Post DA TO 118 WOODBIRDGE

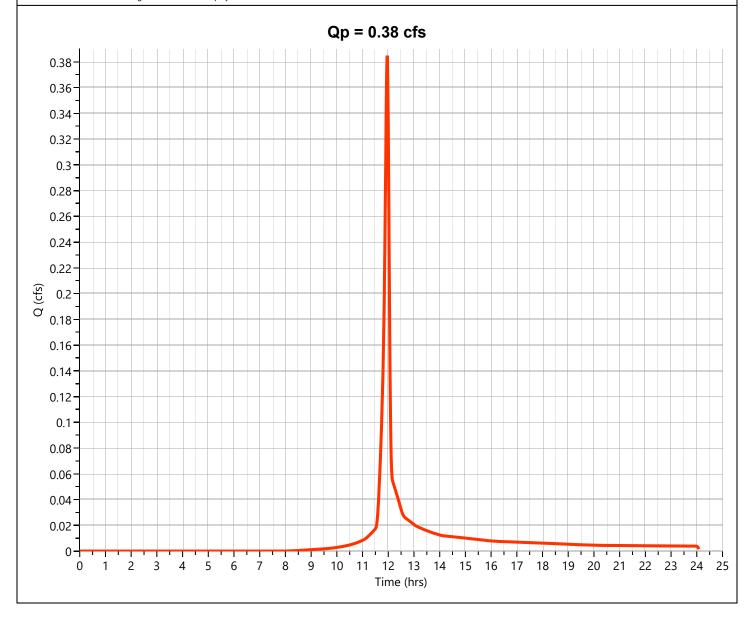
Hyd. No. 5

Hydrograph Type	Hydrograph Type = NRCS Runoff		= 0.385 cfs
Storm Frequency	= 10-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 780 cuft
Drainage Area	= 0.08 ac	Curve Number	= 77.38*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 4.94 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac) CN DESCRIPTION 0.07 77 Woods 0.01 80 Grass/open

0.08 77 Weighted CN Method Employed



Project Name:

Hydrograph 25-yr Summary

10-07-2021

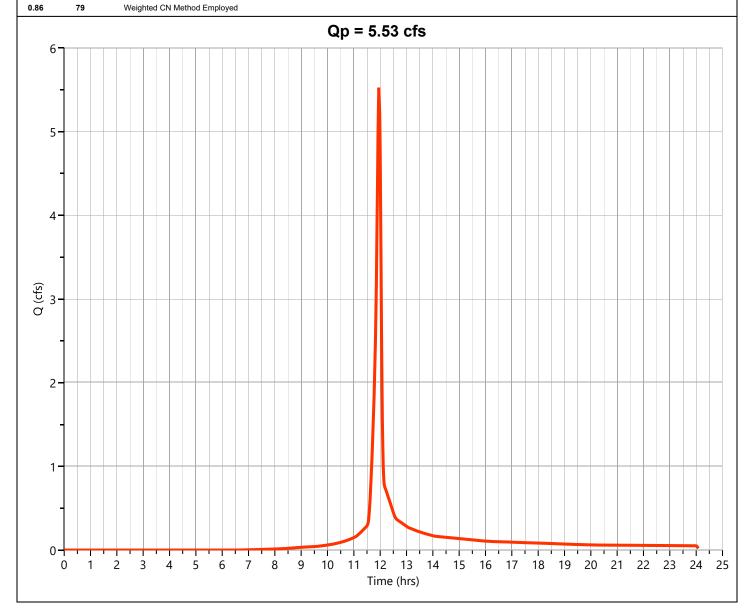
Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	Pre DA5	5.526	11.95	11,384			
2	NRCS Runoff	Post DA5 TO SWALE	4.281	11.95	8,850			
3	NRCS Runoff	Post DA5 SWALE BYPASS	1.284	11.97	2,625			
4	NRCS Runoff	Pre DA TO 118 WOODBRIDG	E 0.738	11.97	1,511			
4 5			E 0.738					

Pre DA5 Hyd. No. 1

Hydrograph Type = NRCS Runoff		Peak Flow	= 5.526 cfs
Storm Frequency	= 25-yr	Time to Peak	= 11.95 hrs
Time Interval	= 1 min	Runoff Volume	= 11,384 cuft
Drainage Area	= 0.86 ac	Curve Number	= 78.99*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 5.84 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac)	CN	DESCRIPTION
0.06	98	Impervious
0.15	80	Grass/open
0.65	77	Woods
0.86	70	Weighted CN Method



Post DA5 TO SWALE

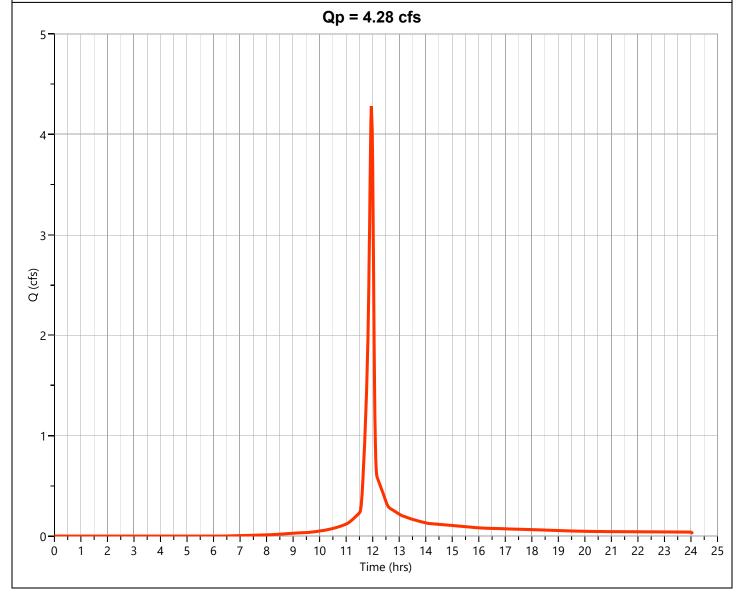
Hyd. No. 2

Hydrograph Type	= NRCS Runoff	Peak Flow	= 4.281 cfs
Storm Frequency	= 25-yr	Time to Peak	= 11.95 hrs
Time Interval	= 1 min	Runoff Volume	= 8,850 cuft
Drainage Area	= 0.65 ac	Curve Number	= 80*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 5.84 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac)	CN	DESCRIPTION
0.06	98	Impervious
0.15	80	Grass/open
0.44	77	Woods
		147 : 17 1 051 54 11

0.65 80 Weighted CN Method Employed



Post DA5 SWALE BYPASS

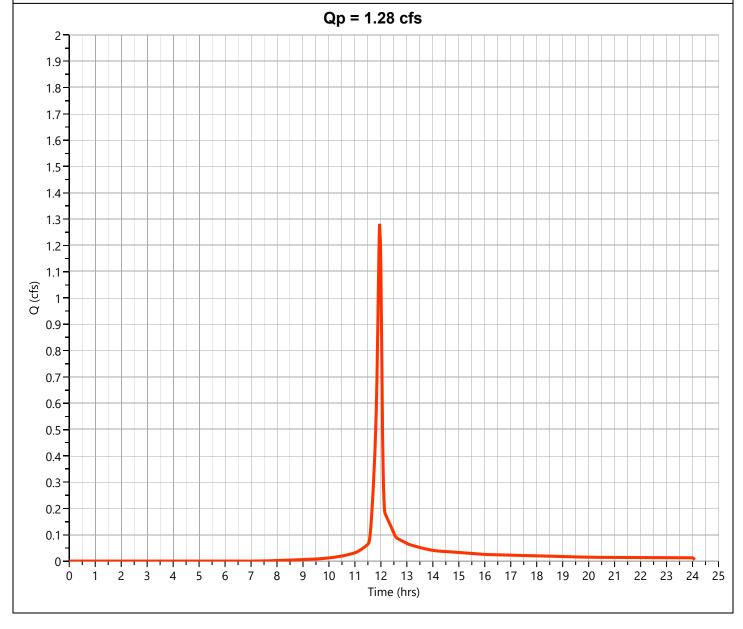
Hyd. No. 3

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.284 cfs
Storm Frequency	= 25-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 2,625 cuft
Drainage Area	= 0.21 ac	Curve Number	= 77*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 5.84 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac) CN DESCRIPTION 0.21 77 Woods

0.21 77 Weighted CN Method Employed



Pre DA TO 118 WOODBRIDGE

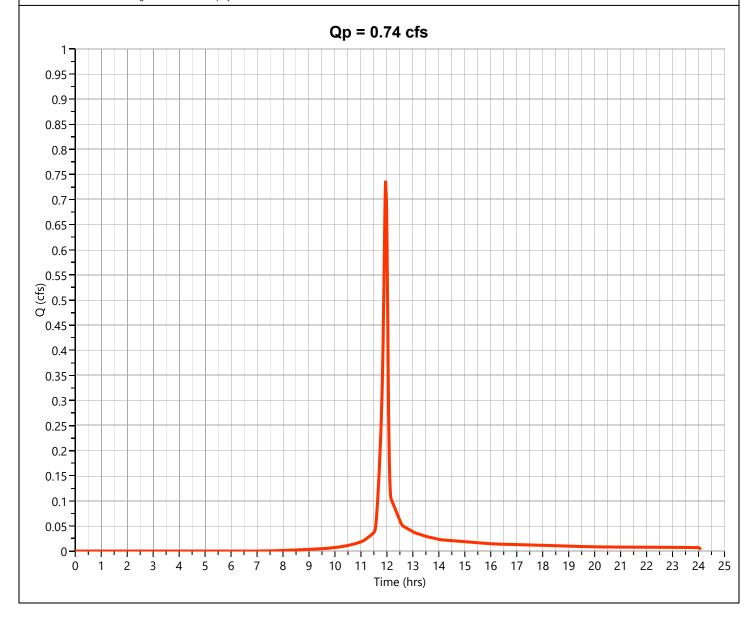
Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.738 cfs
Storm Frequency	= 25-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 1,511 cuft
Drainage Area	= 0.12 ac	Curve Number	= 77.25*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 5.84 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac) CN DESCRIPTION 0.11 77 Woods 0.01 80 Grass/open

0.12 77 Weighted CN Method Employed



Post DA TO 118 WOODBIRDGE

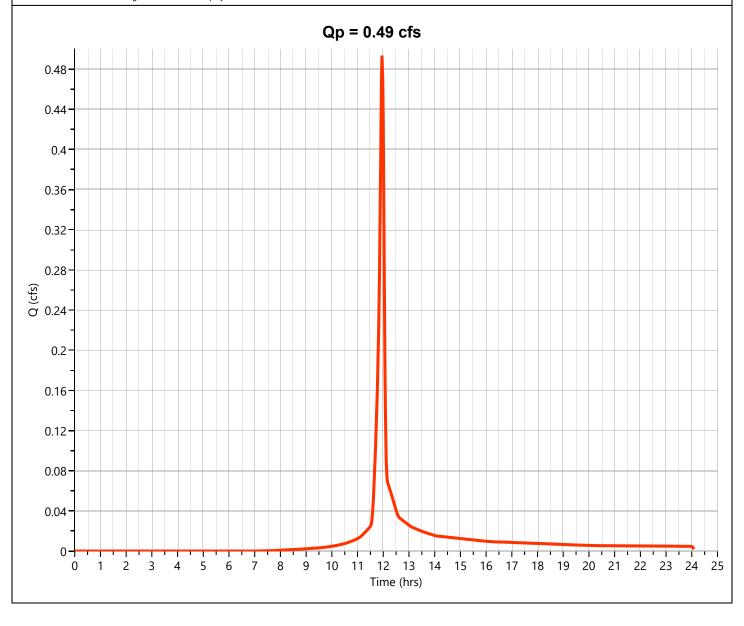
Hyd. No. 5

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.494 cfs
Storm Frequency	= 25-yr	Time to Peak	= 11.97 hrs
Time Interval	= 1 min	Runoff Volume	= 1,011 cuft
Drainage Area	= 0.08 ac	Curve Number	= 77.38*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 5.84 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac) CN DESCRIPTION 0.07 77 Woods 0.01 80 Grass/open

0.08 77 Weighted CN Method Employed



Project Name:

Hydrograph 100-yr Summary Hydrology Studio v 3.0.0.20

10-07-2021

ydrology St	udio v 3.0.0.20							10-07-202
Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	Pre DA5	7.450	11.95	15,553			
2	NRCS Runoff	Post DA5 TO SWALE	5.738	11.95	12,030			
3	NRCS Runoff	Post DA5 SWALE BYPASS	1.749	11.95	3,623			
4	NRCS Runoff	Pre DA TO 118 WOODBRIDG	E 1.004	11.95	2,083			
			E 1.004					

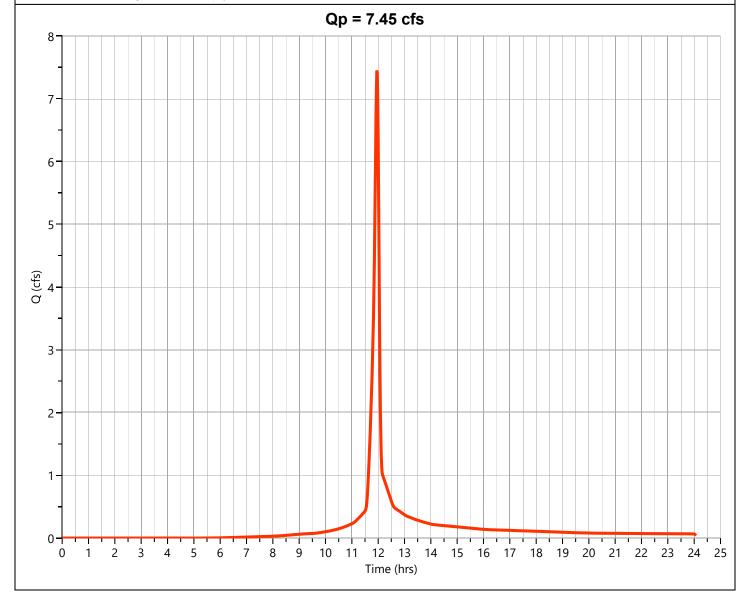
Pre DA5 Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 7.450 cfs
Storm Frequency	= 100-yr	Time to Peak	= 11.95 hrs
Time Interval	= 1 min	Runoff Volume	= 15,553 cuft
Drainage Area	= 0.86 ac	Curve Number	= 78.99*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 7.27 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac)	CN	DESCRIPTION
0.06	98	Impervious
0.15	80	Grass/open
0.65	77	Woods
0.86	70	Weighted CN Meth

0.86 79 Weighted CN Method Employed



Post DA5 TO SWALE

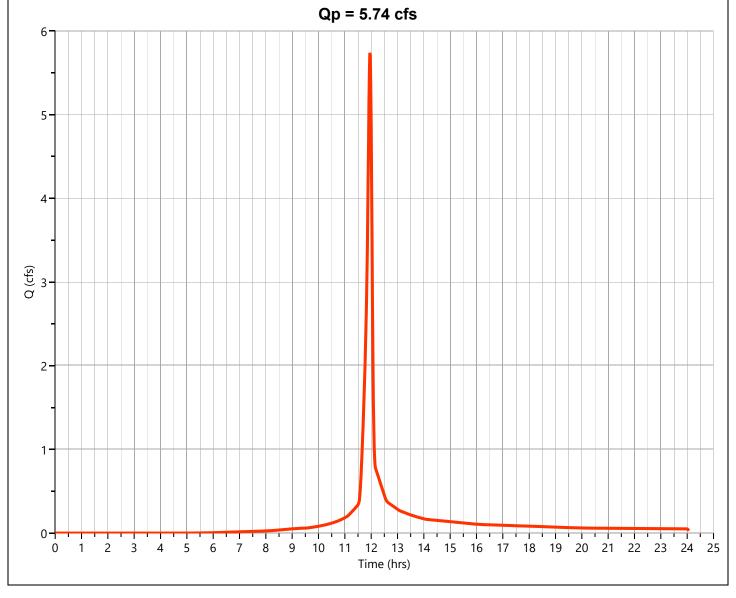
Hyd. No. 2

Hydrograph Type	= NRCS Runoff	Peak Flow	= 5.738 cfs
Storm Frequency	= 100-yr	Time to Peak	= 11.95 hrs
Time Interval	= 1 min	Runoff Volume	= 12,030 cuft
Drainage Area	= 0.65 ac	Curve Number	= 80*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 7.27 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac)	CN	DESCRIPTION
0.06	98	Impervious
0.15	80	Grass/open
0.44	77	Woods
0.05	00	Weighted CN Metho

0.65 80 Weighted CN Method Employed



Post DA5 SWALE BYPASS

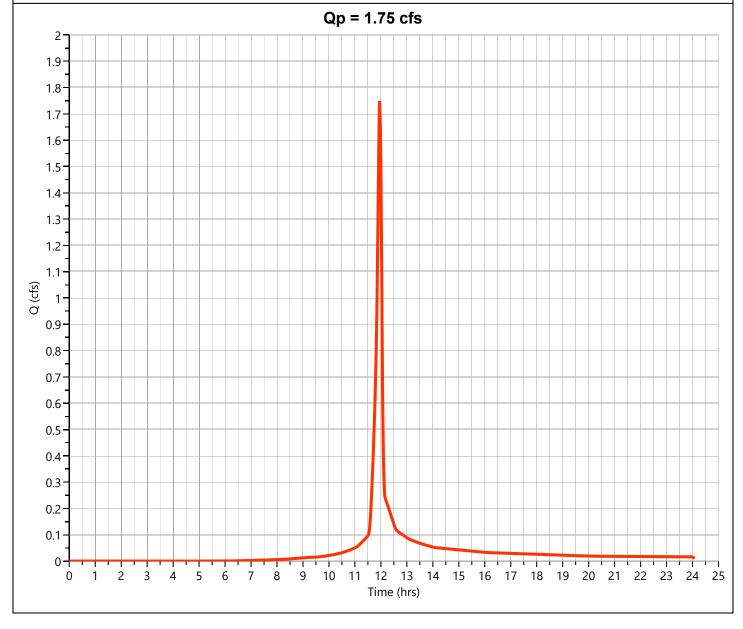
Hyd. No. 3

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.749 cfs
Storm Frequency	= 100-yr	Time to Peak	= 11.95 hrs
Time Interval	= 1 min	Runoff Volume	= 3,623 cuft
Drainage Area	= 0.21 ac	Curve Number	= 77*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 7.27 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac) CN DESCRIPTION 0.21 77 Woods

0.21 77 Weighted CN Method Employed



Pre DA TO 118 WOODBRIDGE

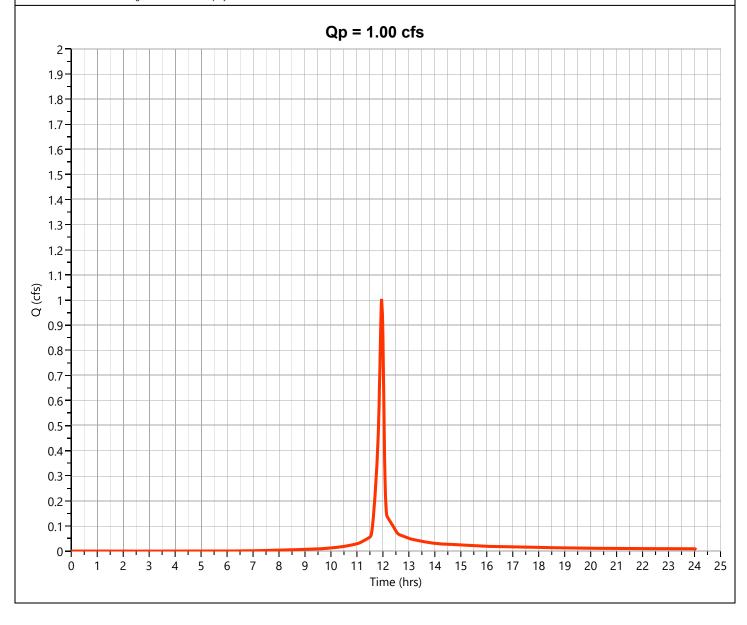
Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.004 cfs
Storm Frequency	= 100-yr	Time to Peak	= 11.95 hrs
Time Interval	= 1 min	Runoff Volume	= 2,083 cuft
Drainage Area	= 0.12 ac	Curve Number	= 77.25*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 7.27 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac) CN DESCRIPTION
0.11 77 Woods
0.01 80 Grass/open

0.12 77 Weighted CN Method Employed



Post DA TO 118 WOODBIRDGE

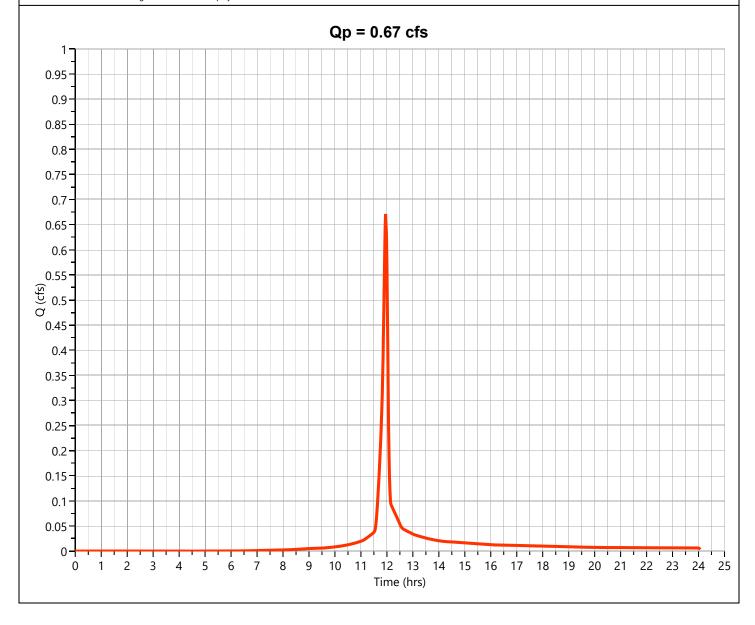
Hyd. No. 5

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.671 cfs
Storm Frequency	= 100-yr	Time to Peak	= 11.95 hrs
Time Interval	= 1 min	Runoff Volume	= 1,393 cuft
Drainage Area	= 0.08 ac	Curve Number	= 77.38*
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Total Rainfall	= 7.27 in	Design Storm	= Type II
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac) CN DESCRIPTION 0.07 77 Woods 0.01 80 Grass/open

0.08 77 Weighted CN Method Employed



APPENDIX D SWALE CALCULATIONS

Studio Express by Hydrology Studio v 1.0.0.9

09-14-2021

SOUTH SWALE - FROM DA5

Channel 1

TRAPEZOIDAL DISCHARGE

Bottom Width = 2.00 ft Method = User-defined

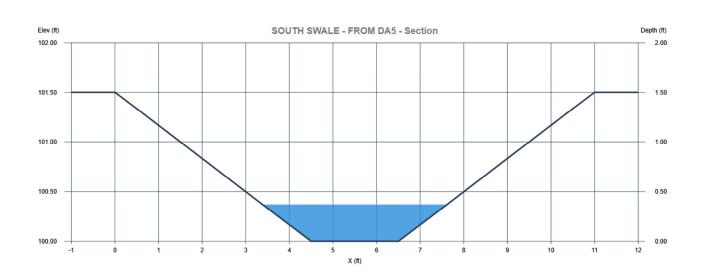
Side Slope Left, z:1 = 3.00Side Slope Right, z:1 = 3.00Total Depth = 1.50 ft Invert Elevation = 100.00 ft Channel Slope = 2.000 %

= 0.030

CALCULATION SAMPLE Q10

Manning's n

Flow	Depth	Area	Velocity	WP	n-value	Crit Depth	HGL	EGL	Max Shear	Top Width
(cfs)	(ft)	(sqft)	(ft/s)	(ft)		(ft)	(ft)	(ft)	(lb/sqft)	(ft)
3.30	0.37	1.15	2.87	4.34	0.030	0.37	100.37	100.50	0.46	4.22



Studio Express by Hydrology Studio v 1.0.0.9

09-14-2021

SOUTH SWALE - FROM DA5

Channel 1

TRAPEZOIDAL DISCHARGE

Bottom Width = 2.00 ft Method = User-defined

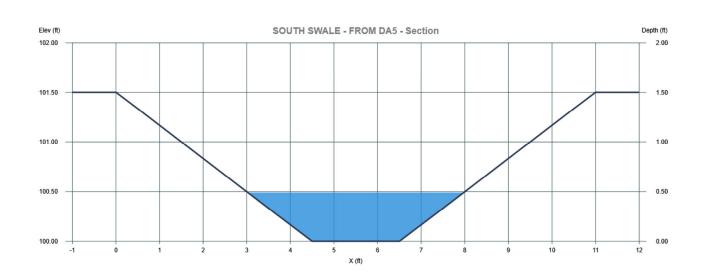
Side Slope Left, z:1 = 3.00Side Slope Right, z:1 = 3.00Total Depth = 1.50 ft Invert Elevation = 100.00 ft Channel Slope = 2.000 %

= 0.030

Manning's n

CALCULATION SAMPLE Q100

Flow	Depth	Area	Velocity	WP	n-value	Crit Depth	HGL	EGL	Max Shear	Top Width
(cfs)	(ft)	(sqft)	(ft/s)	(ft)		(ft)	(ft)	(ft)	(lb/sqft)	(ft)
5.70	0.49	1.70	3.35	5.10	0.030	0.50	100.49	100.66	0.61	4.94



PROPOSAL

Wayne L. Adams, Inc. P.O Box 995 Garner, NC 27529 (919)662-0095

PROPOSAL TO: RADWAY DESIGN	Phone	Date 10/11/21
Street	- CY	Job Name
City, State and Zip Code	SV	VALE INSTALLATION –RESIDENCE INN Job Location
City, State and Zip Code		Job Location
Date of Plans		Job Phone
ROUGH BUDGET COST AT THIS TIMEPROPOSED S	SWALE ESTIMATED LE	NGTH 350 LF
INSTALL EROSION CONTROL CONSTRUCTION ENTRA	ANCE & REMOVE LAT	ER LOCATED AT RESIDENCE INN PARKING LOT
INSTALL ESTIMATED 920 LF TREE FENCE BOTH SIDE	ES OF SWALE & 400 S	ILT FENCE ON DOWN HILL SIDE OF SWALE
HAUL IN MATERIAL & REMOVE LATER TO CREATE	ROADWAY FOR TRU	CKS TO ENTER SWALE SITE
CLEAR AND REMOVE TREES 20 -25 FOOT WIDE FOR TO GAIN ACCESS TO PROPOSED SWALE WORK ARE.		
TEMP BARRIADES INSTALLED IN RESIDENCE INN PA	RKING LOT	
CLEAR & REMOVE TREES IN SWALE AREA ESTIMATE	ED 420 LF X 25 FT WIE	DE
STRIP TROPSOIL FROM SWALE AREA AND HAUL OF	F SITE FOR DISPOSAL	
CUT & FILL PROPOSED SWALE TO GRADE +/.20 PER	PLAN – NO HAUL OF	F ANY EXCESS MATERIAL/DIRT FROM SWALE
REPAIR CURB IN RESIDENCE INN PARKING LOT UP	TO 50 ft ONLY	
DRESS UP AND SEED TEMP ACCESS ROAD W/ MATT	ΓING	
DRESS UP AND SEED SWALE AREA WITH SEED & MA	ATTING	
REMOVE ALL TREE FENCE & SILT FENCE		
· · · · · · · · · · · · · · · · · · ·	and labor – com	plete in accordance with above specifications, for
the sum of:	do	llows (\$ 169,000,00)
BILLED ON 25TH OF MONTH—PAY		llars (\$ 168,000.00) N 10 th OF FOLLOWING MONTH—
PRICES ARE GOOD FOR 15 DAYS (
		cations and conditions are satisfactory and are
ANY AND ALL LEGAL FEES PAID OUT BY W. 30 DAYS LATE FROM DATE OF INVOICE—W.	AYNE L. ADAMŜ IN ILL BE REPAID TO	cified. Payment will be made as outlined above. NC TO COLLECT UNPAID INVOICES, OVER WAYNE L. ADAMS INC., BY OWNER OF PROJECT DDED TO ALL INVOICES OVER 30 DAYS PAST DUE
		Signature
		Standard.
Date of Acceptance:		Signature

PROPOSAL

Wayne L. Adams, Inc. P.O Box 995 Garner, NC 27529 (919)662-0095

PROPOSAL TO: RADWAY DESIGN	Phone	Date 10/11/21				
Street	SV	Job Name VALE INSTALLATION –RESIDENCE INN				
City, State and Zip Code	Job Location					
Data of Blanca		T. I. Di				
Date of Plans		Job Phone				
PERMITS, FEES, BONDS, ENCROACHMENTS, D SOIL TESTING, LIQUIDATED DAMAGES, TREE MAINTAINED, REMOVAL OR DISPOSAL OF AN PART OF THE SWALE DURING CONSTRUCTION REPAIR OR REMOVE ANY EROSION CONTROL	OIRT MOISTURE (& PLANT INSTA NY HAZARDOUS N & AFTER CONS L DAMAGE OUTS	LLATION, NPDES RAINFALL RECORDS MATERIAL, SURVEY, REPAIR OR REPLACE ANY				
NOTES: WAYNE L. ADAMS INC WILL NOT BE RESPONI DAMAGED DURING CONSTRUCTION OR AFTE		GE OR REPLACEMENT/OR COST OF ANY TREES ON IS COMPLETED				
ALL SEEDING IS FOR ONE TIME INSTALLATIO	N—NO DETAIL I	PROVIDED				
ADDITIONAL COST MAY APPLY AFTER PLANS	S ARE APPROVEI) AND SENT OUT FOR REVIEW				
ANY ASPAHLT REPAIR WILL BE AT \$185.00 SY WITH UP TO 2.5" ASPHALT EACH PAVING JO		OVE NO MORE THAN 2.5" ASPHALT AND REPLACE 0.00 FOR MOBILIZATION				
We Propose hereby to furnish material a the sum of:	and labor – com	plete in accordance with above specifications, for				
		llars (\$)				
BILLED ON 25TH OF MONTH—PAYN PRICES ARE GOOD FOR 15 DAYS ON						
		cations and conditions are satisfactory and are				
ANY AND ALL LEGAL FEES PAID OUT BY WA' 30 DAYS LATE FROM DATE OF INVOICE—WIL	YNE L. ADAMŜ II L BE REPAID TO	cified. Payment will be made as outlined above. NC TO COLLECT UNPAID INVOICES, OVER WAYNE L. ADAMS INC., BY OWNER OF PROJECT DDED TO ALL INVOICES OVER 30 DAYS PAST DUE				
		Signature				
Date of Acceptance:		Signature				