

## TOWN OF CHAPEL HILL Planning Department

405 Martin Luther King Jr. Blvd. Chapel Hill, NC 27514-5705

phone (919) 968-2728 email planning@townofchapelhill.org www.townofchapelhill.org

## **Project Fact Sheet**

Project Information	Project Name	860 Weav	rer Dairy Road	Application Number [Staff to Complete]	CZD-24-8		
	Parcel Number(s)		9880564638				
Lot & Zoning	Property Address(es)		860 Weaver Dairy Road				
Information							
	Proposed Zoning District(s)		MU-V-CZD				

						Evaluation [Staff to Complete]
Uses (LUMO Sec. 3.7 and 3.10)	Existing Use(s)	Vacant land				
	Proposed Use(s)	Residential and non-	-residential space			
	Number of Dwelling Units	Existing	0	Proposed to be Removed	0	
		Proposed New, Minimum	630	Proposed New, Maximum	710	
		Market-rate units	TBD	Affordable units	TBD	
		For sale units	105-135	Rental units	525-575	

					Evaluation [Staff to Complete]
Land Area	Net Land Area (NLA) (sq. ft.)	1970128	Net Land Area (acres)	45.23	

	Gross Land Area (GLA) (sq. ft.)	2,167,141 sf	Gross Land Area (acres)	49.75	
					Evaluation [Staff to Complete]
	Proposed street setback (ft.)	48'	Required street setback (ft.)	0' – 22'	
Setbacks &	Proposed interior setback (ft.)	40'	Required interior setback (ft.)	0' – 8'	
Building Height (LUMO Sec. 3.8)	Proposed solar setback (ft.)	80'	Required solar setback (ft.)	8' – 20'	
	Proposed building height, setback (ft.)	70'	Allowed building height, setback (ft.)	70'	
	Proposed building height, core (ft.)	70'-80'	Allowed building height, core (ft.)	114'	
					Fredrick Oberfite Commisted
Land Disturbance	Total proposed land disturbance (sq. ft.)	1,278,040 sf	Total proposed land disturbance (acres)	29.34 ac	Evaluation [Staff to Complete]
					Evaluation [Staff to Complete]
Impervious	Existing ISA (sq. ft.)	0	Removed ISA (sq. ft.)	0	Evaluation [Stail to Complete]
Surface Area (ISA) (LUMO Sec. 3.8)	New ISA (sq. ft.)	780,000	Total ISA (sq. ft.)	780,000	
(2011/0 0001 010)	Proposed ISA ratio (% of GLA)	36.0%	Allowed ISA ratio (% of GLA)	70.00	
					F 1 1 1 1 1 1 1
	Existing floor area (sq. ft.)	0	Removed floor area (sq. ft.)	0	Evaluation [Staff to Complete]
Floor Area (LUMO Sec. 3.6,	New proposed floor area (sq. ft.)	742,039	Total proposed floor area (sq. ft.)	742,039	
3.8, 3.10)	Maximum allowed floor area* (sq. ft.)	2,232,085 sf			
	*Calculated according to th resource conservation distr				

								Evaluation [Staff to C	on
	Total land ar	ea in RCD			Sewered	$\boxtimes$			
	(sq. ft.)	ou 1105	414,4	437 sf	Unsewered				
		Stream	side Zone	Managed	use zone	Upland	d Zone		
	Land area (sq. ft.)	211	,379 sf	96,24	15 sf	106,8	313 sf		
	Proposed use(s)	Greenway trail, road and utility crossing, sanitary sewer installation		Greenway trail, road and utility crossing, stormwater mitigation outfall, and recreation amenities		Greenway trail, road and utility crossing, stormwater mitigation facility, and recreation amenities			
Resource Conservation District (RCD) LUMO 3.6)	3.6.3-2]	Proposed	Allowed	Proposed	Allowed	Proposed	Allowed		
	ISA (sq. ft.)	6,800 sf	12,682 sf	8,750 sf	11,549 sf	11,250 sf	12,818 sf		
	ISA ratio (%)	3.2%	6%	9.10%	12%	10.53%	12%		
	Disturbed area (sq. ft.)	17,600	42,276 sf	35,710	38,498 sf	55,050	42,725 sf		
	Disturbed area ratio (%)	8.28%	20%	37.10%	40%	51.54%	40%		
	Floor area (sq. ft.)	0	0	0	0	0	0		
	Floor area ratio (%)	0	1%	0	1.9%	0			

					Evaluation [Staff to Complete]
Steep Slopes (LUMO Sec. 5.3)	Total steep slopes area (sq. ft.)	40,426 sf	Proposed disturbed area (sq. ft.)	31,350 sf	
(LOMO 3ec. 3.3)	Proposed disturbed area (%)	77.55%	Maximum allowed disturbance (%)	25%	

					Evaluation [Staff to Complete]
Recreation Space	Proposed recreation space (sq. ft.)	85,860 sf	Required recreation space (sq. ft.)	85,489 sf	
(LUMO 5.5)	Proposed payment (\$)	0	Payment-in-lieu calculation	NA	

						Evaluation [Staff to Complete]
		Ту	pe	Wic	dth	
	<b>Direction</b> (North, South, East, West)	Proposed	Required	Proposed	Required	
	North	Е	Е	50'-100' w modif	100	
	West	В	В	10	10	
Landscape	SouthB	В	В	0-10 w modif	10	
Buffers (LUMO Sec. 5.6	Southwest	В	В	10	10	
and Design Manual)	South Weaver Dairy	D	D	30	30	
	East	В	В	10	10	

					Evaluation [Staff to Complete]
Tree Canopy Coverage (LUMO Sec. 5.7)	Proposed tree canopy coverage (% of NLA)	40	Required tree canopy coverage (% of NLA)	40	
					Evaluation [Staff to Complete]
	Existing vehicular parking spaces	0	Removed vehicular parking spaces	0	
	New vehicular parking spaces	780 + townhome garages	Total proposed vehicular parking spaces	780 + townhome garages	
Off-Street Vehicular Parking	Minimum required vehicular parking spaces	548	Maximum allowed vehicular parking spaces	801	
(LUMO Sec. 5.9)	Calculation for minimum requirement		+ 70 2-bd x 1.25+ 10 3-bd have their own garages	x 1.5 for multi-family	
	Calculation for maximum allowance				
					Evaluation [Staff to Complete]
	Existing loading spaces	0	Removed loading spaces	0	
Loading Spaces (LUMO Sec. 5.9)	New loading spaces	0	Total loading proposed spaces	0	
	Minimum required loading spaces	0	Calculation for minimum requirement		
					Evaluation [Staff to Complete]
	Existing bicycle spaces	0	Removed bicycle spaces	0	Evaluation [Stail to Complete]
Off-Street Bicycle Parking (LUMO Sec. 5.9)	New bicycle spaces	180	Total proposed bicycle spaces	180	
	Minimum required bicycle spaces	178			
	Calculation for minimum requirement	1 space per 4 resident	ial units = 710 units x .25 :		

Print Name	Wendi Ramsden	
Date	June 5, 2025	





#### TOWN OF CHAPEL HILL Sustainability and Resilience 405 Martin Luther King Jr. Blvd. Chapel Hill, NC 27514-5705

phone (919) 969-5075
email jrichardson@townofchapelhill.org
www.sustainchapelhill.org/

#### **Climate Action Plan Worksheet**

A Climate Action Plan is an integral component for Conditional Zoning Districts. The intent of this worksheet is to capture commitments made through the rezoning process to meet the Town's sustainability goals. **This is a** fillable form. Please select the box for each climate action design feature that will be provided as part of this project.

Project Name	860 Weaver Dairy Road	Application Number [Staff to Complete]	CZD-24-8			
Property Address(es)	860 Weaver Dairy Road					
	Established B. II I'm B.					
	Energy-Efficient Building Desi					
Commercial construction (including multifamily) will meet the New Buildings Institute's 40  Percent Stretch Energy Standard¹. Alternatively, commercial construction can be designed to be 40 percent better than the latest adopted version of ASHRAE 90.1 in the NC State Building Code.						
	re option, the applicant agrees to provide support apliance Permit can be issued.	ing evidence before the F	inal			
	struction (single-family and two-family) will me 8.12 certification requirements.	eet Energy Star Certified				
<ul> <li>Explain reason</li> </ul>	ons for not providing any energy-efficient buil	ding design features id	entified above.			
Applicant's response	The project will design to the National Green B	uilding Standard Silver ce	ertification.			
Evaluation [Staff to Complete]						
response  Evaluation [Staff to						

<sup>&</sup>lt;sup>1</sup> https://newbuildings.org/resource/40-stretch-energy-standard/

<sup>&</sup>lt;sup>2</sup> https://www.energystar.gov/ia/partners/downloads/ES Combined Path v3.1.pdf

Electric Vehicular Infrastructure	
Each <b>non-residential building</b> served by 10 or more on-site parking spaces will meet the following criteria:	
<ul> <li>5 percent (rounded up) of spaces must have Level 2 electric vehicle supply equipment (EVSE) installed</li> <li>25 percent (rounded up) of total parking spaces must be EVSE-ready</li> </ul>	
EVSE-ready means:	
<ul> <li>A parking space has one 40-amp, 208/240V branch circuit from the panel and is terminated at a receptacle or junction box.</li> <li>The panel should be labeled "EVSE-Ready" or "Future EVSE" and should support any future charging stations that provide at least 6.6 kW of power.</li> </ul>	
Each <i>unit in a multifamily building</i> (including mixed use) with on-site parking will have a minimum of 1 EVSE-capable parking space.	
EVSE-capable means	
<ul> <li>A designated parking space that is provided with continuous conduit/raceway from a panel that supports future charging stations, which provide at least 6.6 kW of power.</li> <li>These spaces do not require wiring or receptacles.</li> <li>For exterior surface lots, the conduit should be run underground to the parking location.</li> </ul>	
Each single-family attached or detached unit with a garage will have a minimum of 1 EVSE-ready parking space.	
<ul> <li>EVSE-ready means:</li> <li>A parking space has one 40-amp, 208/240V branch circuit from the panel and is terminated at a receptacle or junction box.</li> <li>The panel should be labeled "EVSE-Ready" or "Future EVSE" and should support any future charging stations that provide at least 6.6 kW of power.</li> </ul>	$\boxtimes$
Designs will adhere to the U.S. Access Board's most recently updated version of the <a href="Design Recommendations for Accessible Electric Vehicle Charging Stations">Design Recommendations for Accessible Electric Vehicle Charging Stations</a> . If the project calls for direct-current fast charging (DCFC) infrastructure, then appropriate proportional electrical and conduits should be included.	
Town staff review and approve all site designs for initial and future EV charging before a Zoning Compliance Permit is issued.	

Explain reasons for not providing any electric vehicular infrastructure not identified above. If alternative ratios are proposed, please provide justification. Please also describe how the proposed facilities will support current and future levels of electric vehicles used by residents and visitors (e.g., EV Charging Station Management Plan).

Applicant's response

Parking spaces for multi-family will not be assigned. At final buildout of the multi-family buildings, there will be multiple EV charging stations available in the parking structure totalling coverage for a parking space for 5% of the units. The parking spaces will be available for cars to park while charging, but would need to be moved when charging is complete. The the project will also provide infrastructure to accommodate expanded EV charging station offerings as user demand grows.

<sup>&</sup>lt;sup>3</sup> https://www.access-board.gov/tad/ev/

Evaluation [Staff to Complete]		
	Buildings and Energy Climate Actions	

Buildings and Energy Climate Actions	
Achieves all energy-efficient building design standards for applicable development type(s) [Energy Efficient Building Design expectations above]	
All-electric building and site design	$\boxtimes$
All-electric appliances (no fossil fuel appliances)	$\boxtimes$
Rooftop solar (50-100 percent of available roof area)	
Sustainable building materials and resource use equivalent to meeting LEED standard	
LED lighting only for interior spaces	$\boxtimes$
LED lighting only for exterior spaces, including street/parking lot lighting (3000 Kelvin or lower)	$\boxtimes$
Meets International Dark-Sky Association's principles for outdoor lighting	$\boxtimes$
WaterSense-rated appliances and equipment only	
Energy Star-rated appliances and equipment only	$\boxtimes$
Transportation 9 Land Has Climate Actions	
Transportation & Land Use Climate Actions  Achieves all EV charging station infrastructure standards for applicable development type(s) [See	
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<sup>&</sup>lt;sup>4</sup> https://darksky.org/resources/guides-and-how-tos/lighting-principles/

Native and droug	ht-tolerant landscape plantings only	
No irrigation with	potable water	
	Resiliency Climate Actions	
risk of nuisance f		
Exceeds Town st extreme heat imp	andards for green infrastructure elements (trees, bioretention) to reduce pacts	$\boxtimes$
	urface to reduce extreme heat impacts	
Concrete sidewal heat impacts	ks, natural surface trails, and high-albedo coated asphalt to reduce extreme	$\boxtimes$
	h battery storage to provide backup emergency power	
Shaded outdoor s	structures, walkways, and sitting areas to reduce extreme heat impacts	$\boxtimes$
<ul> <li>Explain reaso</li> </ul>	ons for not providing any climate action design elements not identified above	e.
Applicant's response	Level 2 EV charging stations will be provided to a quantity representing 1 parking 5% of the total multi-famliy units. Townhomes will have garages and will be EV-There will be disturbance of wetland and stream and stream buffer areas to instaroads, and greenway trails. There will not be any structures placed in these area majority of plantings will be native, and constructed landscape buffers will be all plantings. It is expected that there will be some irrigation on site in select areas. family portion of the project will offer e-bike capabilities with battery storage locked.	ready. Ill utilities, as. The native The multi-
Evaluation [Staff to Complete]		
	Select other incentives the project will pursue.	
Duke Energy's E	nergy Design Assistance Program <sup>5</sup>	
<u> </u>	lectric Vehicle Incentive Programs <sup>6</sup>	Ш
are part of your p		
	Credit <sup>8</sup> for New Residential Construction (starts at \$500/unit for multifamily and single-family home)	
<ul> <li>Describe other</li> </ul>	er incentives the project will pursue.	
Applicant's response		

<sup>&</sup>lt;sup>5</sup> https://www.duke-energy.com/business/products/design-assistance

<sup>6</sup> https://www.duke-energy.com/energy-education/electric-vehicles/ev-initiatives

<sup>7</sup> https://www.duke-energy.com/business/products/energy-advisor
8 https://www.energystar.gov/about/federal tax credits/federal tax credit archives/tax credits home builders

Evaluation
[Staff to
Complete]
Complete

#### Conditional Zoning Application –

#### Response to Concept Plan Council Comments

#### Concept Project Scope

The concept plan was presented to the Council in October 2024 and included two multi-family buildings, a central flex space option, and a community of 120 townhomes on the east end. The plan included a parking structure for the multi-family uses, garages for the townhomes, and no large surface parking lots. It also included a greenway trail running through the project and connecting to the existing greenway at the west end, ending at Weaver Dairy Road on the east end, and extending up to a point adjacent to the Carol Woods community in the northeast corner. The plan indicated intrusion into the I-40 landscape buffer on the north side, and a spine road running through the project which would cross streams and RCD zones in completing a vehicular connection from west to east.

#### **General comments:**

Focus on buffers against existing neighbors

- The required project buffers are 10 feet wide. The project was specifically laid out with proposed multi-family against existing multi-family, and proposed townhomes against existing townhome developments. The buffers will be planted with evergreen as well as deciduous screening and will be widened where possible. There are some locations along the southern property line where a wide electric easement precludes installation of trees and shrubs. Where possible plants will be installed between the proposed townhomes and the greenway trail/property line.

Make flex space open and inviting to everyone

 The non-residential space will be designed with pedestrian connections and with community green space.

#### Connectivity is important

- Specific design elements promoting connectivity:
  - Vehicular there will be three points of connection for vehicles Weaver Dairy Road at the east end, and connection to the end of Old University Station Road, and a new connection to the end of Adair Drive through the existing right of way adjacent to the project's western property line.
  - Pedestrian Sidewalks will be provided throughout the project and will connect to adjacent sidewalks on Weaver Dairy Road and the sidewalk adjacent to Adair Drive. There will also be nature trails connecting to adjacent properties.
  - Bike/Greenway The greenway trail will connect to the small trail parallel to
    Adair Drive and will extend east to both Carol Woods property at the northeast
    corner, and Weaver Dairy Road sidewalk at the southeast corner of the project.

#### Can retail be included

- The project is within walking distance of retail facilities. There is a possibility that retail could be included in the non-residential future phase of the project and also that retail could be incorporated into the ground floor of the multi-family residential buildings.

#### Intrusion into the I-40 landscape buffer would be acceptable

- The project will intrude into that buffer in small sections. Plantings will be installed to provide equivalent screening. Where intrusions into the I-40 landscape buffer occur, there will be a concentration of evergreen replacement plantings to provide year-round buffering and noise abatement.

#### Elements the Council members supported:

- Density
- Flex space opportunity in the central portion of the project
- Greenway location in the buffers
- Addition of the non-residential uses, pop-up / ghost kitchen space.
- Intrusion into the landscape buffer against I-40
- General design direction.

#### Elements of concern from the Council members:

- Stormwater treatment don't exacerbate existing problems
- Would like to see more variety of housing types
- Flex space opportunity in the central portion of the project
- Green space needs to be accessible and available for recreation

Initial review comments from Brian Peterson

#### Comments on 07-02-24 concept plan

- Provide a continuous pedestrian connection across the length of the site in the form of a greenway or other trails. Tie in with the existing greenway trail that leads to Weaver Dairy Road which currently terminates at Old University Station Road.
- Work with neighboring property owners to connect sidewalks/trails to existing developments, where possible.
- Provide trails/pathways within preservation areas to allow these places to become amenities for the community.
- 4. Orient buildings to engage the street, provide stoops, porches and other architectural features that create an attractive and interesting pedestrian experience.
- The main east/west street has been configured with shifts of alignment which enhances traffic calming and offers opportunities to create changing terminal views and other placemaking features.
- Design the park at the western edge of the development to offer activities or features that create
  gathering opportunities for not only the residents of the 860 Weaver Diary Road development but
  for residents of neighboring residential areas as well.
- 7. Provide for a diversity of housing types, including the "Missing Middle", as opportunities allow.
- In considering uses, look for ways to incorporate mixed-uses as much as possible, including small scale retail or other "retail-like" functions, to provide places of interest for those living and working in the neighboring areas.

A greenway trail is being provided as shown in the plans connecting to the existing trail west of the project and extending through the project to Carol Woods and also south to Weaver Dairy Road at the east end. Nature trails will be incorporated into the preservation areas, but paved trails will not be included.

Buildings are oriented along the street and perpendicular to the street with green spaces to provide interesting pedestrian experiences. The buildings have been pulled away from the sidewalk where possible to allow for streetside plantings between the internal sidewalks and the townhomes.

The alignment shifts in the main street have been preserved to provide better pockets of developable space and community gathering space, to minimize intrusions into environmentally sensitive areas, and also to provide traffic calming.

The park at the western entry to the project will be available to all residents.

There will be various sizes of rental and for-sale units from studio up to 4-bedroom size. In this way the project will attract a diversity of residents.

The non-residential uses have not been determined but it is possible that there will be some retail uses. It is also possible that some main floor retail space will be provided in the multi-family buildings.



## STORMWATER IMPACT ANALYSIS CONDITIONAL ZONING SUBMITTAL

# 860 WEAVER DAIRY ROAD CHAPEL HILL, NC



PREPARED BY:

**ALBERTO SARRAFF LOPEZ** 

**REVISED BY:** 

ANDREW M. OAKLEY, PE

THOMAS & HUTTON
2510 MERIDIAN PARKWAY, SUITE 100
DURHAM, NC 27713
NC BOARD OF ENGINEERS & SURVEYORS LICENSE NO.
F-0871

JOB NUMBER: 32044.0000

03.05.2025

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## 3. PROJECT NARRATIVE

#### A. PROJECT DESCRIPTION AND SUMMARY

This residential / mixed use project will encompass 45.23 acres of multi-family and town home housing located north of Weaver Dairy Road and south of I-40 and between Chapel Hill North and Carol Woods. The project involves 425-450 apartment rental units in two 6-story apartment buildings with 700 parking spaces, all located in a parking structure behind the apartments. The project also includes 120-140 townhomes with garages to reduce surface parking.

Calculations for peak discharge, runoff volume (2-yr only), and water quality treatment (85% TSS) for all of the proposed improvements are provided. Soils on the site are mainly hydrologic soil group B with some D. The proposed site is located in the Cape Fear River Basin in the Jordan Lake Watershed.

Per the Town of Chapel Hill stormwater ordinance, the stormwater runoff rate leaving the site under post-development conditions may not exceed the stormwater runoff rate under predevelopment conditions for the 1-year, 2-year, 25-year, and 100-year storms. The additional runoff volume from the pre-development to post-development conditions for the 2-year storm must also be captured on-site. In addition, added impervious surfaces must be treated for 85% total suspended solids (TSS) removal.

#### **B. METHODOLOGY**

- The Durham County Soil Survey is used to identify the soil types located on the site.
- HydroCAD software is used to calculate pre- and post-development peak flow rates and volumes for each sub-basin. HydroCAD uses the SCS TR-20 method to develop hydrographs.
- HydroCAD software is used to calculate the composite curve number for each sub-basin. HydroCAD
  uses the NRCS TR-55 method for calculation composite curve numbers.

Pre- development drainage patterns are based on existing topographic information available with Survey and GIS data. Post-development drainage patterns are based on the proposed grading and development. Summary tables are included that detail pre-development and post-development peak flow rates from the proposed work, along with the percentage increase or decrease.

#### C. DISCUSSION OF RESULTS

#### **Peak Flow Analysis:**

For the peak discharge calculations, Q1/Q2/Q25/Q100, the site was broken into 8 drainage basins with 6 points of analysis. The post development configuration of the site redirects drainage from 5 of the basins (Basin 2A, Basin 2C, Basin 3, Basin 4, and Basin 8) into the other drainage areas, therefore decreasing the peak flow to these areas without detention.

Basin 1 receives flow from the proposed apartment building and parking deck with a discharge point at the ROW of I-40 and underground detention will be required to provide attenuation. Basin 2B receives flow from offsite which bypasses detention and receives flow from a future mid-rise structure. Basin 2B discharge at the ROW of I-40 and will require a pond to attenuate flow. Basin 5 receives flow from townhomes, associated parking and walks. Basin 5 discharges at a point on the southern property line and will require a pond to attenuate flow. Basin 6 receives flow from townhomes, associated parking and walks. Basin 6 will require a pond to attenuate flow. Basin 7 receives flow from townhomes, associated parking and walks. Basin 7 discharges into an existing storm drain in Weaver Dairy Road and will require a pond to attenuate flow.

#### **Runoff Volume Analysis:**

Per the Town of Chapel Hill Design Manual, "the post-development stormwater runoff rate leaving the site shall not exceed the pre-development (existing conditions) stormwater runoff rate leaving the site for the local 1-year, 2-year, 25-year, and 100-year storm events." In addition, "the post-development stormwater runoff volume leaving the site shall not exceed the pre-development (existing conditions) stormwater runoff volume leaving the site for the local 2-year frequency, 24-hour duration storm event." The underground detention and the wet ponds are sized to capture the increased volume to meet this requirement.

#### **Pollutant Analysis:**

The Town of Chapel Hill Design Manual states that BMP's shall be designed to remove 85% average total suspended solids from the post-development stormwater runoff. The proposed project results in an increase in impervious surface. As a result, a BMP must be installed to treat for 85% TSS removal for the additional impervious surface added.

In order to meet this requirement, a sandfilter addition is proposed to the underground detention and the ponds will be wet ponds.

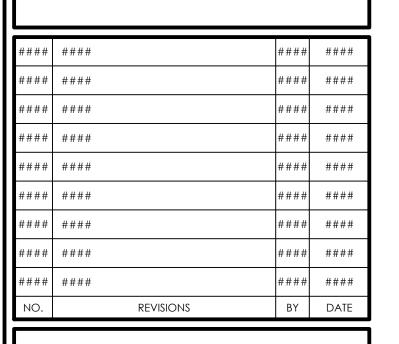
#### D. CONCLUSIONS

Because the increase in impervious surfaces as a result of this project resulted in an increase in peak flow for the 1-year, 2-year, 25-year, and 100-year storms, and in runoff volume for the 2-year storm, detention and treatment are proposed in the form of underground detention and wet ponds to meet the requirements of the Town's stormwater ordinance. In addition, a sandfilter and wet ponds will provide an 85% TSS reduction for all of the proposed impervious areas that can be captured. Some required trail and walk areas are unable to be directed to the SCM's.

## 4. REFERENCE MATERIAL



	PARKING, DRIVES, WALKS	94,275	SF	
	ROOF	55,905	SF	
* * * * * * * * * * * * * * * * * * *	GRASS	143,869	SF	
	WOODS	476,725	SF	
	POND	0	SF	
	TOTAL	770,774	SF	
				•





2510 Meridian Parkway • Suite 100 Durham, NC 27713 • 919.682.0368 www.thomasandhutton.com

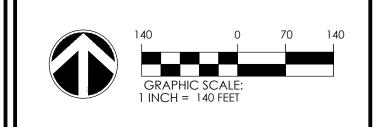
POST DEV STUDY

860 WEAVER DAIRY ROAD

PROJECT LOCATION: ####

CLIENT/OWNER:

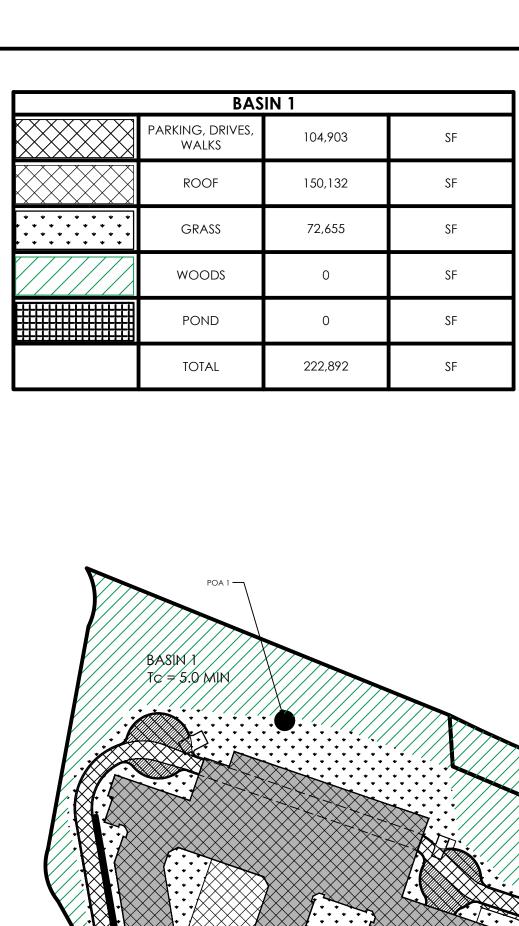
LAND PLANNING & ENTITLEMENTS 157 EAST FRANKLIN STREET CHAPEL HILL, NC 27514

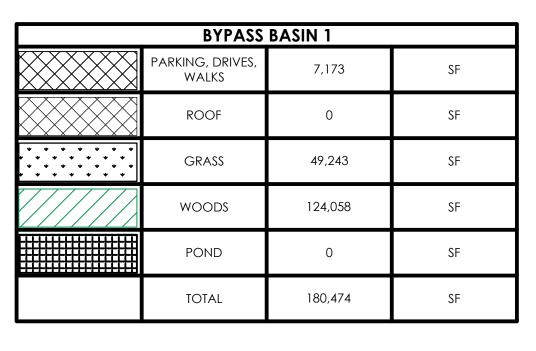


DATUM: HORIZ.: NAV 83

VERT.: NAVD 88

JOB NO:	32044.0000
DATE:	03/05/25
DRAWN:	ASL
DESIGNED:	####
REVIEWED:	AO
APPROVED:	####
SCALE:	1" = 140'

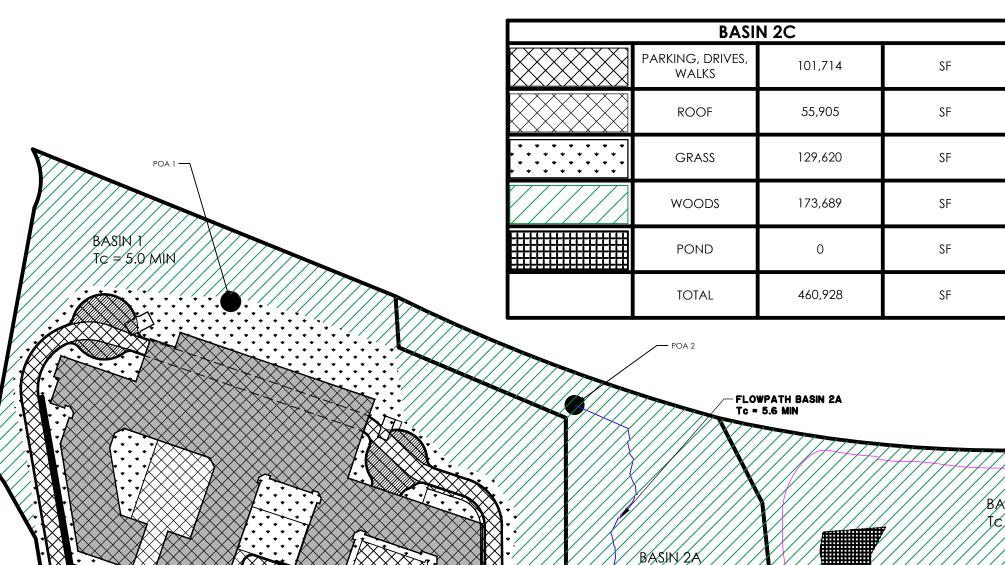




BYPASS BASIN 2A				
	Parking, Drives, Walks	0	SF	
	ROOF	0	SF	
* * * * * * * * * * * * * * * * * * * *	GRASS	0	SF	
	WOODS	120,581	SF	
	POND	0	SF	
	TOTAL	120,581	SF	

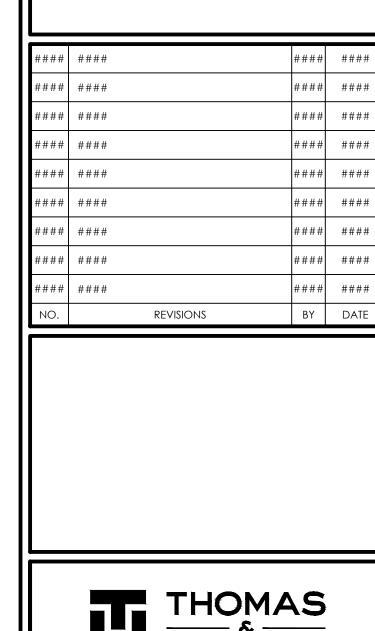
BASIN 2B				
	Parking, Drives, Walks	22,914	SF	
	ROOF	95,673	SF	
* * * * * * * * * * * * * * * * * * * *	GRASS	47,465	SF	
	WOODS	0	SF	
	POND	0	SF	
	TOTAL	166,052	SF	

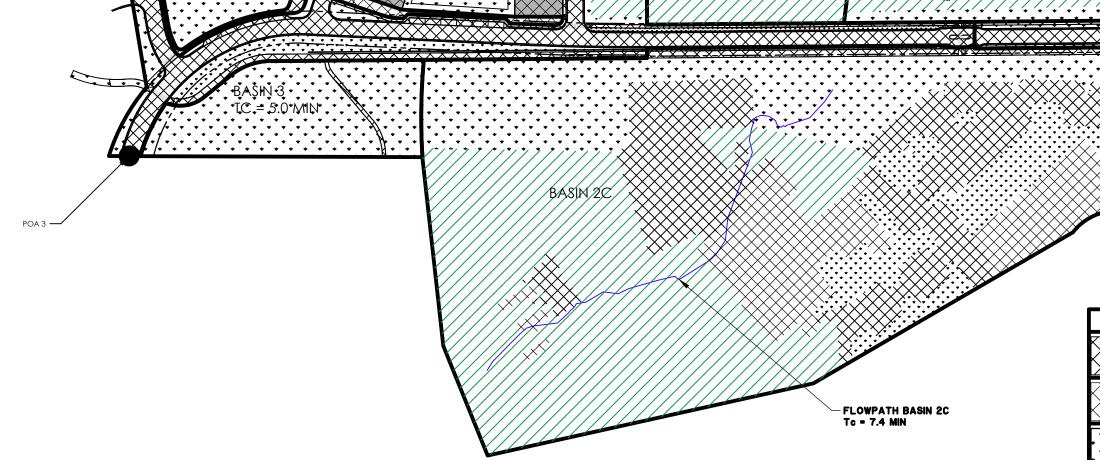
	BYPASS BASIN 2B				
	Parking, Drives, Walks	0	SF		
	ROOF	0	SF		
* * * * * * * * * * * * * * * * * * *	GRASS	4,450	SF		
	WOODS	54,125	SF		
	POND	0	SF		
	TOTAL	58,575	SF		



BASIN 3					
	Parking, Drives, Walks	0	SF		
	ROOF	0	SF		
* * * * * * * * * * * * * * * * * * * *	GRASS	48,549	SF		
	WOODS	0	SF		
	POND	0	SF		
	TOTAL	48,549	SF		

	BAS	IN 4	
	Parking, Drives, Walks	5,028	SF
	ROOF	0	SF
* * * * * * * * * * * * * * * * * * * *	GRASS	20,201	SF
	WOODS	6,966	SF
	POND	0	SF
	TOTAL	32,195	SF





	BAS	IN 5	
	PARKING, DRIVES, WALKS	44,766	SF
	ROOF	40,704	SF
* * * * * * * * * * * * * * * * * * *	GRASS	22,215	SF
	WOODS	0	SF
	POND	4639	SF
	TOTAL	112,324	SF

BYPASS BASIN 5					
	Parking, Drives, Walks	12,408	SF		
	ROOF	0	SF		
* * * * * * * * * * * * * * * * * * *	GRASS	51,602	SF		
	WOODS	110,888	SF		
	POND	0	SF		
	TOTAL	174,898	SF		

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POST DEV. STUDY

860 WEAVER DAIRY ROAD

PROJECT LOCATION:

####

####

CLIENT/OWNER: LAND PLANNING & ENTITLEMENTS

157 EAST FRANKILING STREET CHAPEL HILL, NC 27514 ####

DATUM: HORIZ.: NAD 83

JOB NO:	32044.0000
DATE:	03/05/2025
DRAWN:	ASL
DESIGNED:	####
REVIEWED:	AO
APPROVED:	####
SCALE:	1" = 140'

BASIN 6						
	Parking, Drives, Walks	54,466	SF			
	ROOF	60,672	SF			
* * * * * * * * * * * * * * * * * * *	GRASS	58,385	SF			
	WOODS	0	SF			
	POND	15,036	SF			
	TOTAL	188,559	SF			

BYPASS BASIN 6						
	Parking, drives, Walks	4,487	SF			
	ROOF	0	SF			
* * * * * * * * * * * * * * * * * * *	GRASS	34,572	SF			
	WOODS	215,461	SF			
	POND	0	SF			
	TOTAL	254,520	SF			

BASIN 7						
	Parking, drives, walks	50,624	SF			
	ROOF	28,800	SF			
* * * * * * * * * * * * * * * * * * * *	GRASS	44,621	SF			
	WOODS	0	SF			
	POND	6,497	SF			
	TOTAL	130,542	SF			

BYPASS BASIN 7						
	Parking, Drives, Walks	13,422	SF			
	ROOF	0	SF			
* * * * * * * * * * * * * * * * * * *	GRASS	52,486	SF			
	WOODS	39,944	SF			
	POND	0	SF			
	TOTAL	105,852	SF			

	ROOF	0	SF
* * * * * * * * * * * * * * * * * * *	GRASS	53,439	SF
	WOODS	230,959	SF
	POND	0	SF
	TOTAL	289,814	SF

PARKING, DRIVES, WALKS

BASIN 8

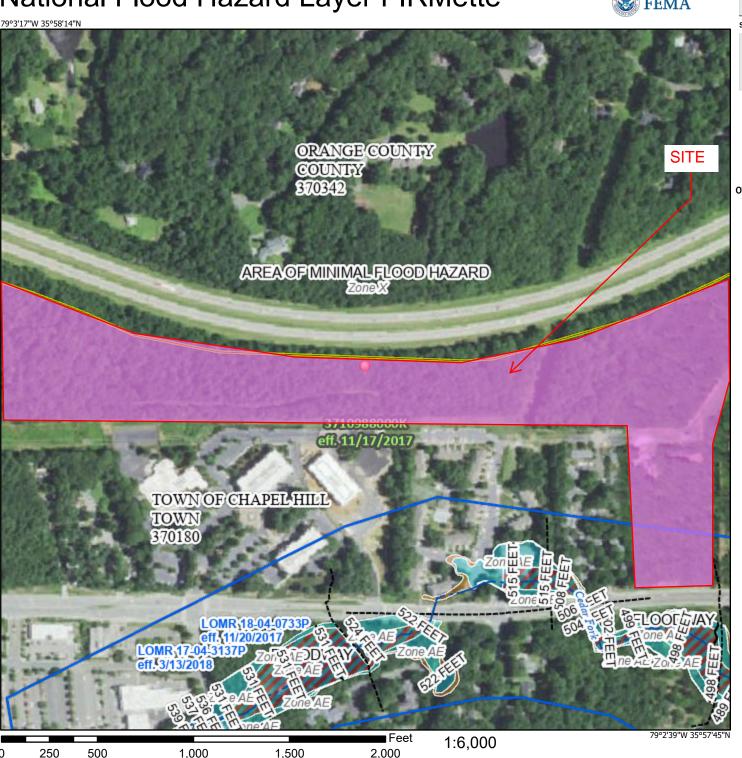
5,416

SF



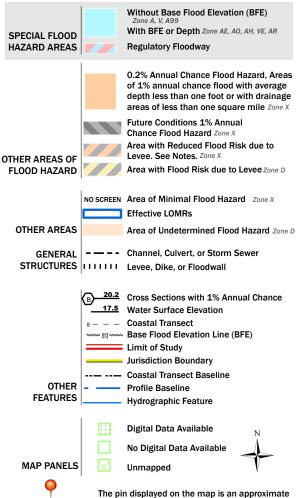
## National Flood Hazard Layer FIRMette





#### Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT



This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

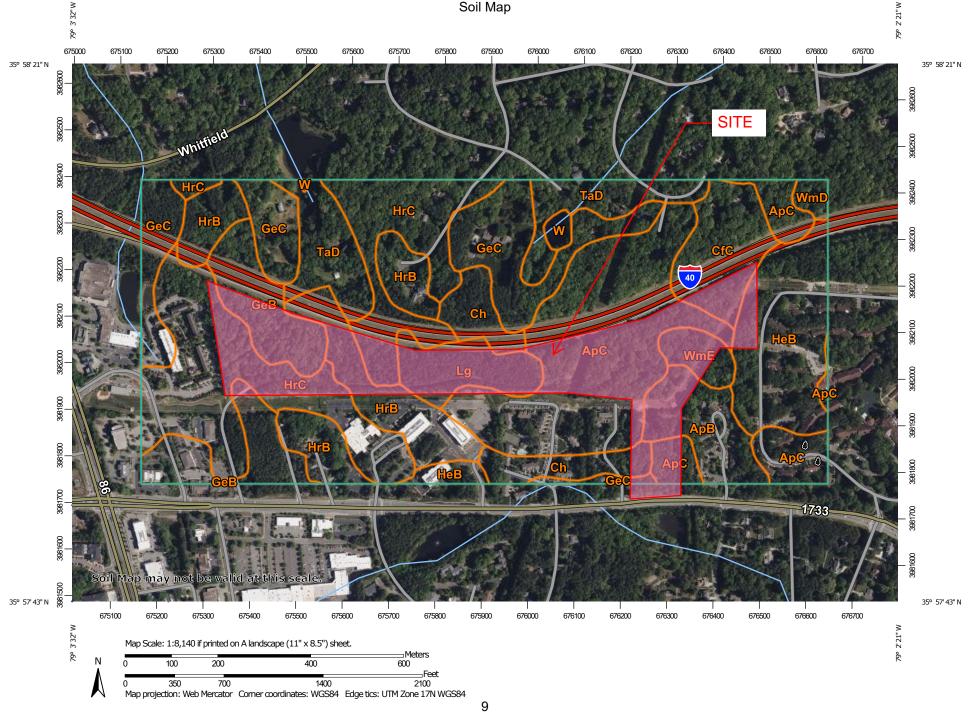
point selected by the user and does not represent

an authoritative property location.

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 11/7/2024 at 8:49 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

## Custom Soil Resource Report Soil Map



## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
АрВ	Appling sandy loam, 2 to 6 percent slopes	9.6	4.0%
ApC	Appling sandy loam, 6 to 10 percent slopes	47.3	19.7%
CfC	Cecil fine sandy loam, 6 to 10 percent slopes	11.0	4.6%
Ch	Chewacla loam, 0 to 2 percent slopes, frequently flooded	9.7	4.0%
GeB	Georgeville silt loam, 2 to 6 percent slopes	18.0	7.5%
GeC	Georgeville silt loam, 6 to 10 percent slopes	25.7	10.7%
HeB	Helena sandy loam, 2 to 8 percent slopes	20.6	8.6%
HrB	Herndon silt loam, 2 to 6 percent slopes	23.7	9.9%
HrC	Herndon silt loam, 6 to 10 percent slopes	44.5	18.6%
Lg	Lignum silt loam, 0 to 3 percent slopes	8.0	3.3%
TaD	Tarrus silt loam, 8 to 15 percent slopes	15.9	6.6%
W	Water	1.1	0.4%
WmD	Wedowee sandy loam, 8 to 15 percent slopes	1.3	0.6%
WmE	Wedowee sandy loam, 15 to 25 percent slopes	3.6	1.5%
Totals for Area of Interest		240.0	100.0%

## **Map Unit Descriptions**

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without

## 5. PEAK FLOW ANALYSIS – HYDROLOGY AND HYDRAULICS MODELING



Project Name: 860 Weaver Dairy

By: ASL

Revised: AO

Project Number: 32044.0000

Date: 3/5/2025

Date: 3/5/2025

#### **Peak Flow Analysis Summary**

Drainage Basin 1	Tc	Area	CN	Q1	Q2	Q25	Q100
Diamage Basin i	(min)	(ac)		(cfs)	(cfs)	(cfs)	(cfs)
Pre-Dev Basin 1	5.9	8.42	79	17.85	24.78	55.13	73.83
Post Basin 1 to SCM 1	5	7.52	89	23.92	30.61	56.50	72.49
Post Dev Bypass 1	5	4.14	60	2.02	4.04	15.11	23.00
Post 1 + Bypass 1				25.94	34.65	71.61	95.49
Post-Dev Basin 1 with Treatment (POA 1)				4.77	7.16	42.15	56.96
Percent difference between Pre- Development and Post Development with Treatment				-73%	-71%	-24%	-23%

Drainage Basin 2A	Тс	Area	CN	Q1	Q2	Q25	Q100
	(min)	(ac)		(cfs)	(cfs)	(cfs)	(cfs)
Pre-Dev Basin 2A	7.7	5.75	79	11.86	16.53	37.06	49.72
Post-Dev Bypass 2A	5.6	2.77	79	5.86	8.13	18.08	24.21
Percent difference between Pre-							
Development and Post Development without Treatment				E40/	E40/	E40/	E40/
Development without Treatment				-51%	-51%	-51%	-51%

Drainage Basin 2B	Tc	Area	CN	Q1	Q2	Q25	Q100
Diamage Basin 2B	(min)	(ac)		(cfs)	(cfs)	(cfs)	(cfs)
Pre-Dev Basin 2B	6.9	3.03	60	1.36	2.85	11.03	16.86
Post Basin 2B to SCM 2	5	3.81	86	10.86	14.21	28.14	35.51
Post Dev Bypass 2B	5	1.34	54	0.18	0.65	3.68	5.99
Post 2B + Bypass 2B				11.04	14.86	31.82	41.50
Post-Dev Basin 2B with Treatment (POA 2)				1.03	1.67	8.72	16.28
Percent difference between Pre- Development and Post Development with Treatment				-24%	-41%	-21%	-3%

Drainage Bosin 2C	Tc	Area	CN	Q1	Q2	Q25	Q100
Drainage Basin 2C	(min)	(ac)		(cfs)	(cfs)	(cfs)	(cfs)
Pre-Dev Basin 2C	7.4	10.74	86	30.55	40.05	79.55	103.18
Post-Dev Bypass 2C	7.4	10.58	86	30.10	39.47	78.39	101.68
Percent difference between Pre-							
Development and Post Development without Treatment				-1%	-1%	-1%	-1%

Drainage Basin 3	Tc	Area	CN	Q1	Q2	Q25	Q100
	(min)	(ac)		(cfs)	(cfs)	(cfs)	(cfs)
Pre-Dev Basin 3	5.1	1.19	78	2.39	3.35	7.58	10.20
Post-Dev Bypass 3	5	1.11	79	2.34	3.25	7.22	9.67
Percent difference between Pre-							
Development and Post Development without Treatment				-2%	-3%	-5%	-5%

#### NOTE FOR BASIN 2B:

IN ORDER TO GENERATE A MORE REALISTIC CN FOR THE FUTURE DEVELOPMENT OF BASIN 2B, 70% OF THE INITIAL POST DEV. BYPASS AREA WAS ASSUMED TO BE DEVELOPED. WITHIN THE 70% OF DEVELOPED AREA, 70% WAS ASSUMED TO BE IMPERVIOUS AND 30% TO BE PERVIOUS AND ADDED TO THE ROAD AND GRASS AREAS DRAINING TO SCM 2.

FOR THE REMAINING 30 % IMPERVIOUS, AROUND 92% WAS ASSUMED TO BE WOODED AND 8% TO BE GRASS (BASED ON THE INITIAL GRASS AND WOODS RATIO IN THE POST DEV. BYPASS 2B )

Drainage Basin 4	Tc (min)	Area (ac)	CN	Q1 (cfs)	Q2 (cfs)	<b>Q25</b> (cfs)	Q100 (cfs)
Pre-Dev Basin 4	8.9	2.99	59	0.97	2.26	9.71	15.12
Post-Dev Bypass 4	5	0.74	67	0.74	1.20	3.48	5.01
Percent difference between Pre- Development and Post Development without Treatment				-24%	-47%	-64%	-67%

Drainage Basin 5	Tc	Area	CN	Q1	Q2	Q25	Q100
Diamage Basin 5	(min)	(ac)		(cfs)	(cfs)	(cfs)	(cfs)
Pre-Dev Basin 5	12.3	7.00	59	2.07	4.56	19.31	30.39
Post Basin 5 to SCM 3	5	2.58	88	7.91	10.20	19.09	24.59
Post Dev Bypass 5	12.3	4.02	59	1.19	2.61	11.07	17.42
Post 5 + Bypass 5				9.10	12.81	30.16	42.01
Post-Dev Basin 5 with Treatment (POA 5)				1.80	3.30	19.04	30.22
Percent difference between Pre- Development and Post							
Development with Treatment				-13%	-28%	-1%	-1%

Drainage Basin 6	Tc	Area	CN	Q1	Q2	Q25	Q100
Diamage Basin o	(min)	(ac)		(cfs)	(cfs)	(cfs)	(cfs)
Pre-Dev Basin 6	12.4	12.76	60	3.49	7.29	29.18	45.38
Post Basin 6 to SCM 4	5	4.33	87	12.81	16.63	31.55	40.81
Post Dev Bypass 6	5	5.84	61	3.25	6.21	22.20	33.49
Post 6 + Bypass 6				16.06	22.84	53.75	74.30
Post-Dev Basin 6 with Treatment (POA 6)				3.25	6.30	24.77	43.63
Percent difference between Pre- Development and Post							
Development with Treatment				-7%	-14%	-15%	-4%

Drainage Basin 7	Tc	Area	CN	Q1	Q2	Q25	Q100
Diamage Basili /	(min)	(ac)		(cfs)	(cfs)	(cfs)	(cfs)
Pre-Dev Basin 7	10.2	5.47	57	1.16	2.89	15.08	24.16
Post Basin 7 to SCM 5	5	3.00	81	6.92	9.42	20.23	26.83
Post Dev Bypass 7	10	2.43	57	0.52	1.29	6.77	10.83
Post 7 + Bypass 7				7.44	10.71	27.00	37.66
Post-Dev Basin 7 with Treatment (POA 7)				0.96	1.80	15.03	23.82
Percent difference between Pre- Development and Post							
Development with Treatment				-17%	-38%	0%	-1%

Drainage Basin 8	Tc	Area	CN	Q1	Q2	Q25	Q100
	(min)	(ac)		(cfs)	(cfs)	(cfs)	(cfs)
Pre-Development	9.1	6.65	62	3.40	6.67	24.39	36.89
Post-Development Basin H	9.1	6.65	62	3.40	6.67	24.39	36.89
Percent difference between Pre-							
Development and Post Development with Treatment				0%	0%	0%	0%

#### 6. RUNOFF VOLUME ANALYSIS



Project Name: 860 Weaver Dairy By: AMO Revised: Project Number: 32044.0000 Date: 11/8/2024 Date:

#### **Runoff Volume Analysis Summary Basin 1**

	V2	V2
	af	(cf)
Pre-Development Basin A	1.276	55,583
Pre-Development Runoff Volume		55,583
Post-Development Basin A area to Sandfilter	1.664	72,484
Post-Development Basin A area to Bypass	0.233	10,149
Post-Development Basin A		82,633
Post Development Runoff Volume		82,633
Sandfilter 2-year Volume		33,134
		11,323
Post Development Runoff Volume with Treatment Percent difference between Pre-Development and Post		49,499
Development with Treatment		-11%

#### **Runoff Volume Analysis Summary Basin 2A**

	V2	V2
	af	(cf)
Pre-Development Basin 2A	0.871	37,941
Pre-Development Runoff Volume		37,941
Post-Development Basin 2A	0.419	18,252
Post Development Runoff Volume		18,252
Percent difference between Pre-Development and Post		-52%

## Runoff Volume Analysis Summary Basin 2B

	V2	V2
	af	(cf)
Pre-Development Basin 2B	0.171	7,449
Pre-Development Runoff Volume	0.171	7,449
Post-Development Basin 2B area to SCM 2	0.757	32,975
Post-Development Basin 2B area to Bypass	0.047	2,047
Post-Development Basin 2B		35,022
Post Development Runoff Volume		35,022
Wet Pond Volume		49,143
Post Development Runoff Volume with Treatment		-14,121
Percent difference between Pre-Development and Post		-290%

## Runoff Volume Analysis Summary Basin 2C

	V2	V2
	af	(cf)
Pre-Development Basin 2C	2.133	92,913
Pre-Development Runoff Volume		92,913
Post-Development Basin 2C	2.102	91,563
Post-Development Basin 2C  Post Development Runoff Volume		91,563 <b>91,563</b>

# Runoff Volume Analysis Summary Basin 3

	V2	V2
	af	(cf)
Pre-Development Basin 3	0.174	7,579
Pre-Development Runoff Volume		7,579
Post-Development Basin 3	0.169	7,362
Post Development Runoff Volume		7,362
Percent difference between Pre-Development and Post		-3%

# Runoff Volume Analysis Summary Basin 4

	V2	V2
	af	(cf)
Pre-Development Basin 4	0.157	6,839
Pre-Development Runoff Volume		6,839
Post-Development Basin 4	0.064	2,788
Post Development Runoff Volume		2,788
Percent difference between Pre-Development and Post		-59%

# Runoff Volume Analysis Summary Basin 5

	V2	V2
	af	(cf)
Pre-Development Basin 5	0.368	16,030
Pre-Development Runoff Volume	0.368	16,030
Post-Development Basin 5 area to SCM 3	0.551	24,002
Post-Development Basin 5 area to Bypass	0.211	9,191
Post-Development Basin 5		33,193
Post Development Runoff Volume		33,193
Wet Pond Volume		27,369
Post Development Runoff Volume with Treatment		5,824
Percent difference between Pre-Development and Post		-64%

# Runoff Volume Analysis Summary Basin 6

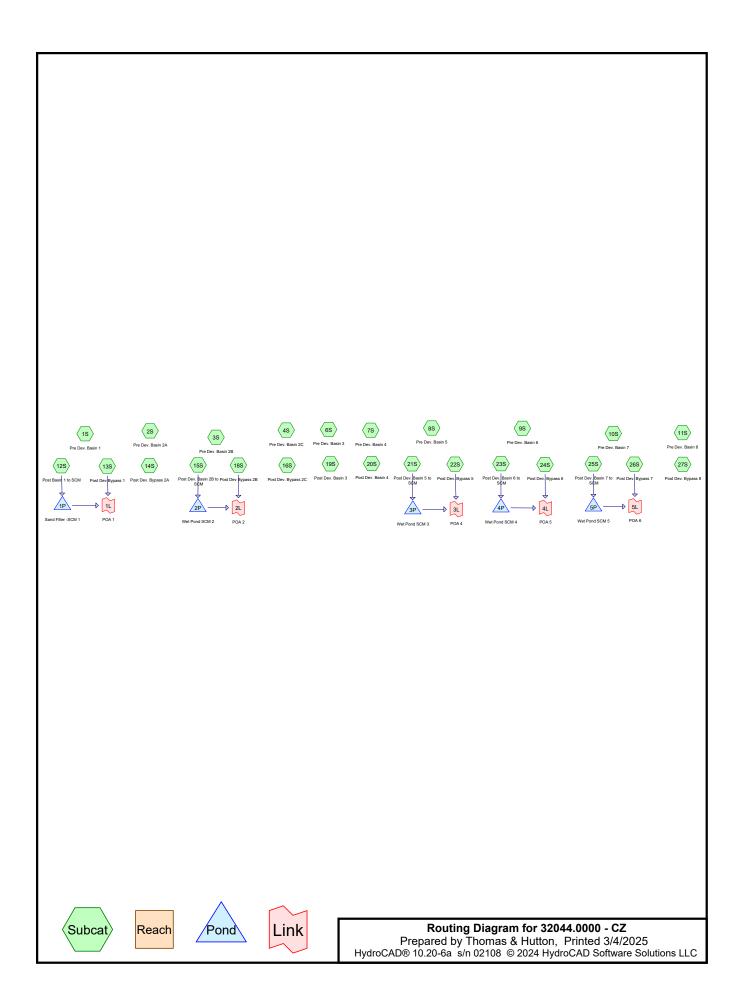
	V2	V2
	af	(cf)
Pre-Development Basin 6	0.570	24,829
Pre-Development Runoff Volume	0.570	24,829
Post-Development Basin 6 area to SCM 4	0.892	38,856
Post-Development Basin 6 area to Bypass	0.352	15,333
Post-Development Basin 6		54,189
Post Development Runoff Volume		54,189
Wet Pond Volume		29,958
Post Development Runoff Volume with Treatment		24,231
Percent difference between Pre-Development and Post		-2%

# Runoff Volume Analysis Summary Basin 7

	V2	V2
	af	(cf)
Pre-Development Basin 7	0.247	10,759
Pre-Development Runoff Volume	0.247	10,759
Post-Development Basin 7 area to SCM 5	0.492	21,432
Post-Development Basin 7 area to Bypass	0.110	4,792
Post-Development Basin 7		26,223
Post Development Runoff Volume		26,223
Wet Pond Volume		24,189
Post Development Runoff Volume with Treatment		2,034
Percent difference between Pre-Development and Post		-81%

# Runoff Volume Analysis Summary Basin 8

	V2	V2
	af	(cf)
Pre-Development Basin 4	0.428	18,644
Pre-Development Runoff Volume		18,644
Post-Development Basin 4	0.428	18,644
Post Development Runoff Volume		18,644
Percent difference between Pre-Development and Post		0%



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# Rainfall Events Listing (selected events)

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	1-yr	Type II 24-hr		Default	24.00	1	3.16	2
2	2-yr	Type II 24-hr		Default	24.00	1	3.82	2
3	25-yr	Type II 24-hr		Default	24.00	1	6.53	2
4	100-yr	Type II 24-hr		Default	24.00	1	8.16	2

# **Area Listing (selected nodes)**

Area	CN	Description
(acres)		(subcatchment-numbers)
2.454	69	50-75% Grass cover, Fair, HSG B (11S, 27S)
2.080	79	50-75% Grass cover, Fair, HSG C (6S, 19S)
2.598	61	>75% Grass cover, Good, HSG B (20S, 23S, 24S)
4.316	56	Brush, Fair, HSG B (7S, 9S, 12S, 13S)
5.029	48	Brush, Good, HSG B (8S, 15S, 18S, 21S, 22S, 25S, 26S)
0.568	73	Brush, Good, HSG D (1S)
0.708	69	Pasture/grassland/range, Fair, HSG B (10S)
2.408	98	Paved parking, HSG A (12S)
5.191	98	Paved parking, HSG B (11S, 13S, 15S, 20S, 21S, 22S, 23S, 24S, 25S, 26S, 27S)
4.499	98	Paved parking, HSG D (4S, 16S)
8.631	98	Roofs, HSG B (12S, 15S, 21S, 23S, 25S)
2.567	98	Roofs, HSG D (4S, 16S)
0.601	98	Water Surface, 0% imp, HSG B (21S, 23S, 25S)
41.888	60	Woods, Fair, HSG B (3S, 7S, 8S, 9S, 11S, 13S, 20S, 22S, 24S, 27S)
0.228	73	Woods, Fair, HSG C (6S)
24.342	79	Woods, Fair, HSG D (1S, 2S, 4S, 14S, 16S)
6.922	55	Woods, Good, HSG B (10S, 18S, 26S)
6.278	82	Woods/grass comb., Fair, HSG D (4S, 16S)
0.941	58	Woods/grass comb., Good, HSG B (15S)
122.250	72	TOTAL AREA

# Soil Listing (selected nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
2.408	HSG A	12S
79.279	HSG B	3S, 7S, 8S, 9S, 10S, 11S, 12S, 13S, 15S, 18S, 20S, 21S, 22S, 23S, 24S, 25S,
		26S, 27S
2.308	HSG C	6S, 19S
38.255	HSG D	1S, 2S, 4S, 14S, 16S
0.000	Other	
122.250		TOTAL AREA

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# **Pipe Listing (selected nodes)**

L	.ine#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Width (inches)	Diam/Height (inches)	Inside-Fill (inches)	Node Name
	1	1P	524.00	523.00	85.0	0.0118	0.011	0.0	36.0	0.0	
	2	2P	526.00	525.55	45.0	0.0100	0.010	0.0	36.0	0.0	
	3	3P	534.00	533.56	44.0	0.0100	0.013	0.0	30.0	0.0	
	4	4P	523.00	521.50	45.0	0.0333	0.011	0.0	36.0	0.0	
	5	5P	513.00	512.00	79.0	0.0127	0.013	0.0	36.0	0.0	

Time span=1.00-200.00 hrs, dt=0.10 hrs, 1991 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: Pre Dev. Basin 1 Flow Length=560'	Runoff Area=366,801 sf 0.00% Impervious Runoff Depth=1.31" Slope=0.0630 '/' Tc=5.9 min CN=79 Runoff=17.85 cfs 0.917 af
Subcatchment2S: Pre Dev. Basin 2A Flow Length=803'	Runoff Area=250,337 sf 0.00% Impervious Runoff Depth=1.31" Slope=0.0650 '/' Tc=7.7 min CN=79 Runoff=11.86 cfs 0.626 af
Subcatchment3S: Pre Dev. Basin 2B Flow Length=577	Runoff Area=132,113 sf 0.00% Impervious Runoff Depth=0.39" ' Slope=0.0451 '/' Tc=6.9 min CN=60 Runoff=1.36 cfs 0.099 af
Subcatchment4S: Pre Dev. Basin 2C Flow Length=753'	Runoff Area=467,738 sf 32.11% Impervious Runoff Depth=1.80" Slope=0.0633 '/' Tc=7.4 min CN=86 Runoff=30.55 cfs 1.611 af
Subcatchment6S: Pre Dev. Basin 3 Flow Length=243	Runoff Area=52,001 sf 0.00% Impervious Runoff Depth=1.24" ' Slope=0.0170 '/' Tc=5.1 min CN=78 Runoff=2.39 cfs 0.124 af
Subcatchment7S: Pre Dev. Basin 4 Flow Length=561	Runoff Area=130,388 sf 0.00% Impervious Runoff Depth=0.36" Slope=0.0221 '/' Tc=8.9 min CN=59 Runoff=0.97 cfs 0.090 af
Subcatchment8S: Pre Dev. Basin 5 Flow Length=998'	Runoff Area=305,128 sf 0.00% Impervious Runoff Depth=0.36" Slope=0.0301 '/' Tc=12.3 min CN=59 Runoff=2.07 cfs 0.210 af
Subcatchment9S: Pre Dev. Basin 6 Flow Length=1,222'	Runoff Area=441,055 sf 0.00% Impervious Runoff Depth=0.39" Slope=0.0441 '/' Tc=12.4 min CN=60 Runoff=3.49 cfs 0.331 af
Subcatchment10S: Pre Dev. Basin 7 Flow Length=977'	Runoff Area=238,293 sf 0.00% Impervious Runoff Depth=0.30" Slope=0.0468 '/' Tc=10.2 min CN=57 Runoff=1.16 cfs 0.135 af
Subcatchment11S: Pre Dev. Basin 8 Flow Length=826	Runoff Area=289,814 sf 1.87% Impervious Runoff Depth=0.46" Slope=0.0447 '/' Tc=9.1 min CN=62 Runoff=3.40 cfs 0.257 af
Subcatchment12S: Post Basin 1 to SCM	Runoff Area=327,690 sf 77.83% Impervious Runoff Depth=2.05" Tc=5.0 min CN=89 Runoff=23.92 cfs 1.282 af
Subcatchment13S: Post Dev Bypass1	Runoff Area=180,474 sf 3.97% Impervious Runoff Depth=0.39" Tc=5.0 min CN=60 Runoff=2.02 cfs 0.136 af
Subcatchment14S: Post Dev. Bypass 2A Flow Length=421	Runoff Area=120,581 sf 0.00% Impervious Runoff Depth=1.31" Slope=0.0411 '/' Tc=5.6 min CN=79 Runoff=5.86 cfs 0.301 af
Subcatchment15S: Post Dev. Basin 2B	Runoff Area=166,052 sf 71.42% Impervious Runoff Depth=1.80" Tc=5.0 min CN=86 Runoff=10.86 cfs 0.572 af
Subcatchment16S: Post Dev. Bypass 2C Flow Length=753'	Runoff Area=460,928 sf 34.20% Impervious Runoff Depth=1.80" Slope=0.0633 '/' Tc=7.4 min CN=86 Runoff=30.10 cfs 1.588 af
Subcatchment18S: Post Dev Bypass 2B	Runoff Area=58,575 sf 0.00% Impervious Runoff Depth=0.21" Tc=5.0 min CN=54 Runoff=0.18 cfs 0.024 af

Type II 24-hr	1-yr Rain	fall=3.16"
	Printed	3/4/2025

Runoff Area=174,898 sf 7.09% Impervious Runoff Depth=0.36"

Tc=12.3 min CN=59 Runoff=1.19 cfs 0.120 af

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Depth=1.31" cfs 0.121 af

Depth=0.67" cfs 0.041 af

Depth=1.96" cfs 0.421 af

<b>32044.0000 - CZ</b> Prepared by Thomas & Hutton	Type II 24-hr 1-yr Ra Printe
HydroCAD® 10.20-6a s/n 02108 © 2024 Hyd	roCAD Software Solutions LLC
Subcatchment19S: Post Dev. Basin 3	Runoff Area=48,549 sf 0.00% Impervious Runoff I Tc=5.0 min CN=79 Runoff=2.34
Subcatchment20S: Post Dev. Basin 4	Runoff Area=32,195 sf 15.62% Impervious Runoff I Tc=5.0 min CN=67 Runoff=0.74
Subcatchment21S: Post Dev. Basin 5 to	Runoff Area=112,324 sf 76.09% Impervious Runoff I Tc=5.0 min CN=88 Runoff=7.91

Subcatchment22S: Post Dev. Bypass 5

Subcatchment23S: Post Dev. Basin 6 to	Runoff Area=188,559 sf	61.06% Imperv	vious Runoff Depth=1.88"
	Tc=5.0	) min CN=87	Runoff=12.81 cfs 0.678 af

Subcatchment24S: Post Dev. Bypass 6	Runoff Area=254,520 sf	1.76% Impervio	us Runoff Depth=0.43"
	Tc=5.0	min CN=61 F	Runoff=3.25 cfs 0.208 af

Subcatchment25S: Post Dev. Basin 7 to	Runoff Area=130,542 sf	60.84% Impervio	ous Runoff Depth=1.44"
	Tc=5	0 min CN=81 I	Runoff=6 92 cfs 0 359 af

Subcatchment 26S: Post Dev. Bypass 7	Runoff Area=105,852 sf	12.68% Imper	vious Runoff Depth=0.30"
	Tc=10	.0 min CN=57	Runoff=0.52 cfs 0.060 af

Subcatchment27S: Post Dev. Bypass 8	Runoff Area=289,8	814 sf 1.87	% Impervi	ous Runoff Depth=0.46"
Flow Length=826'	Slope=0.0447 '/'	Tc=9.1 min	CN=62	Runoff=3.40 cfs 0.257 af

Pond 1P: Sand Filter -SCM 1	Peak Elev=527.67'	Storage=25,039 cf	Inflow=23.92 cfs	1.282 af
			Outflow=3.07 cfs	1.282 af

Pond 2P: Wet Pond SCM 2	Peak Elev=527.04'	Storage=44,570 cf	Inflow=10.86 cfs	0.572 af
			Outflow=0.97 cfs	0.572 af

Pond 3P: Wet Pond SCM 3	Peak Elev=535.62'	Storage=24,226 cf	Inflow=7.91 cfs	0.421 af
		(	Outflow=0.64 cfs	0.421 af

Pond 4P: Wet Pond SCM 4	Peak Elev=524.24'	Storage=22,875 cf	Inflow=12.81 cfs 0.678 af
		_	Outflow=0.17 cfs 0.353 af

Pond 5P: Wet Pond SCM 5	Peak Elev=514.44'	Storage=20,772 cf	Inflow=6.92 cfs	0.359 af
		(	Outflow=0.47 cfs	0.359 af

Link 1L: POA 1	Inflow=4.77 cfs 1.418 af
	Primary=4.77 cfs 1.418 af

Link 2L: POA 2	Inflow=1.03 cfs 0.596 af
	Primary=1.03 cfs 0.596 af

Link 3L: POA 4 Inflow=1.80 cfs 0.542 af Primary=1.80 cfs 0.542 af **32044.0000 - CZ**Prepared by Thomas & Hutton

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Type II 24-hr 1-yr Rainfall=3.16"

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**Link 4L: POA 5**Inflow=3.25 cfs 0.561 af
Primary=3.25 cfs 0.561 af

**Link 5L: POA 6**Inflow=0.96 cfs 0.419 af
Primary=0.96 cfs 0.419 af

Total Runoff Area = 122.250 ac Runoff Volume = 10.569 af Average Runoff Depth = 1.04" 80.94% Pervious = 98.954 ac 19.06% Impervious = 23.297 ac

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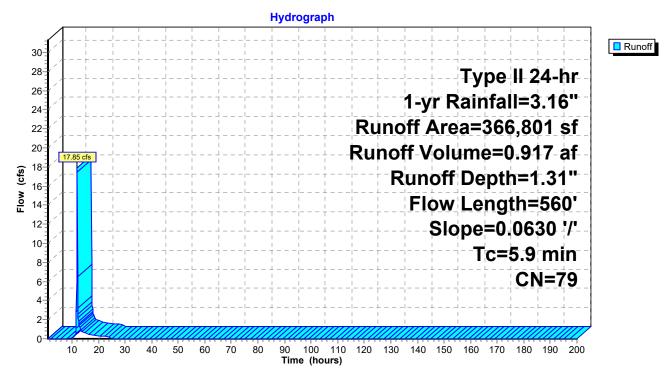
# Summary for Subcatchment 1S: Pre Dev. Basin 1

Runoff = 17.85 cfs @ 11.98 hrs, Volume= 0.917 af, Depth= 1.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

_	Α	rea (sf)	CN	Description		
	3	42,042	79	Woods, Fai	r, HSG D	
_		24,759	73	Brush, Goo	d, HSG D	
	3	66,801	79	Weighted A	verage	
	3	66,801		100.00% P	ervious Are	a
	Тс	Length	Slope	,	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.9	560	0.0630	1.58		Kirpich Method, Estimated Tc
						General overland flow k= 2.00

#### Subcatchment 1S: Pre Dev. Basin 1



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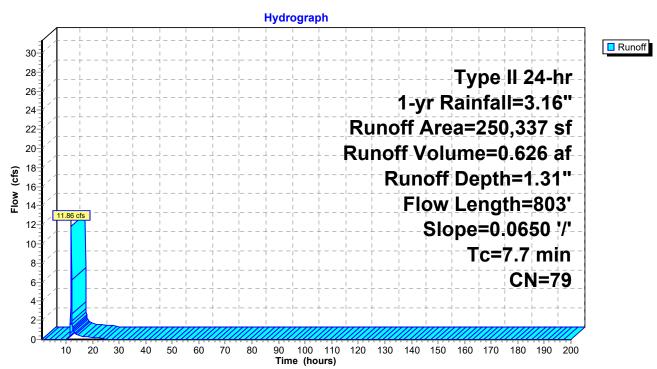
## Summary for Subcatchment 2S: Pre Dev. Basin 2A

Runoff = 11.86 cfs @ 11.99 hrs, Volume= 0.626 af, Depth= 1.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

A	rea (sf)	CN	Description		
2	50,337	79	Woods, Fai	r, HSG D	
2	50,337		100.00% P	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description
7.7	803	0.0650	1.74		Kirpich Method, Estimated Tc General overland flow k= 2.00

#### Subcatchment 2S: Pre Dev. Basin 2A



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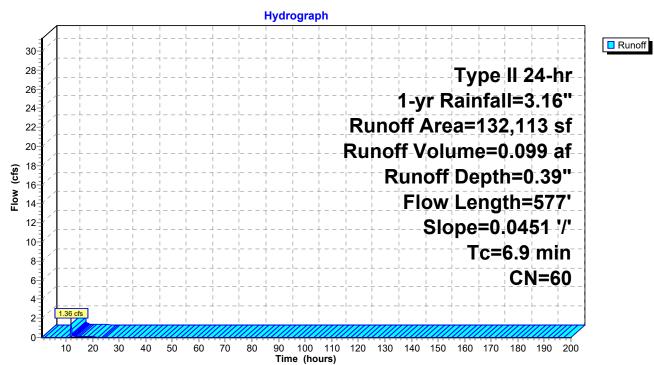
## Summary for Subcatchment 3S: Pre Dev. Basin 2B

Runoff = 1.36 cfs @ 12.01 hrs, Volume= 0.099 af, Depth= 0.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

_	Α	rea (sf)	CN [	Description		
	1	32,113	60 V	Voods, Fai	r, HSG B	
	1	32,113	1	100.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	6.9	577	0.0451	1.40		Kirpich Method, Estimated Tc General overland flow k= 2.00

#### Subcatchment 3S: Pre Dev. Basin 2B



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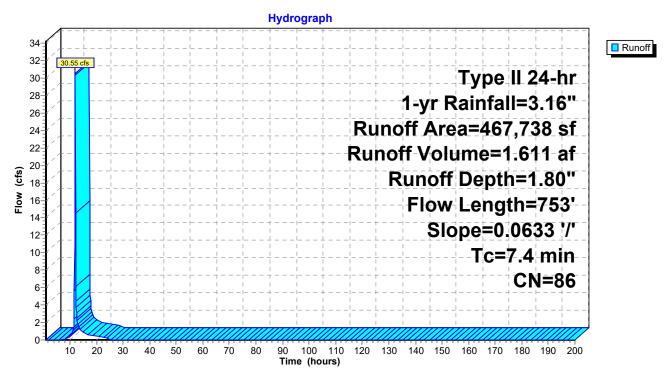
## Summary for Subcatchment 4S: Pre Dev. Basin 2C

Runoff = 30.55 cfs @ 11.99 hrs, Volume= 1.611 af, Depth= 1.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

	Area (sf)	CN [	Description						
	173,689	79 V	Voods, Fai	r, HSG D					
	94,275	98 F	Paved park	ing, HSG D					
	143,869	82 V	Voods/gras	ss comb., F	Fair, HSG D				
	55,905	98 F	Roofs, HSC	B D					
	467,738	86 V	Veighted A	verage					
;	317,558	6	7.89% Per	vious Area	l				
	150,180	3	2.11% Imp	ervious Ar	rea				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
7.4	753	0.0633	1.69		Kirpich Method, Estimated Tc				
					General overland flow k= 2.00				

#### Subcatchment 4S: Pre Dev. Basin 2C



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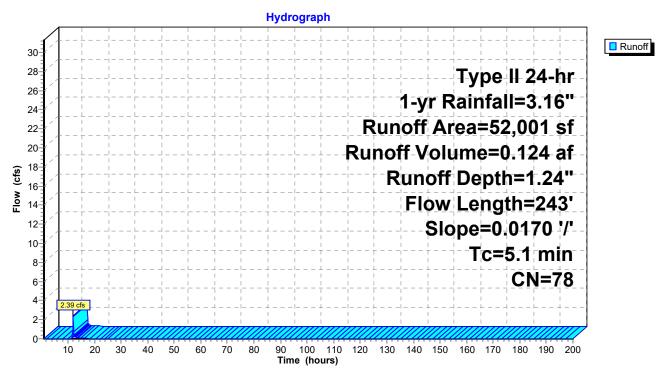
## Summary for Subcatchment 6S: Pre Dev. Basin 3

Runoff = 2.39 cfs @ 11.97 hrs, Volume= 0.124 af, Depth= 1.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

A	rea (sf)	CN	Description						
	9,925	73	Woods, Fai	r, HSG C					
	42,076	79	50-75% Gra	ass cover, f	Fair, HSG C				
	52,001	78	Weighted A	verage					
	52,001		100.00% Pe	ervious Are	a				
Tc	Length	Slope	•	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.1	243	0.0170	0.79		Kirpich Method, Estimated Tc				
					General overland flow k= 2.00				

#### Subcatchment 6S: Pre Dev. Basin 3



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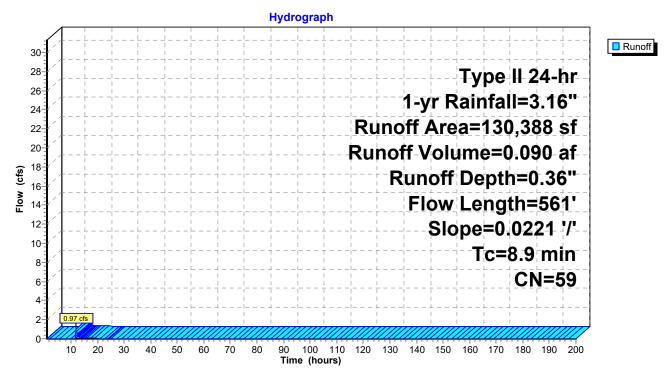
# Summary for Subcatchment 7S: Pre Dev. Basin 4

Runoff = 0.97 cfs @ 12.04 hrs, Volume= 0.090 af, Depth= 0.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

	Area (sf)	CN	Description		
	104,102	60	Woods, Fai	r, HSG B	
	26,286	56	Brush, Fair,	, HSG B	
	130,388	59	Weighted A	verage	
	130,388		100.00% P	ervious Are	ea
Tc	Length	Slope	,	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
8.9	561	0.0221	1.06		Kirpich Method,
					General overland flow k= 2.00

#### Subcatchment 7S: Pre Dev. Basin 4



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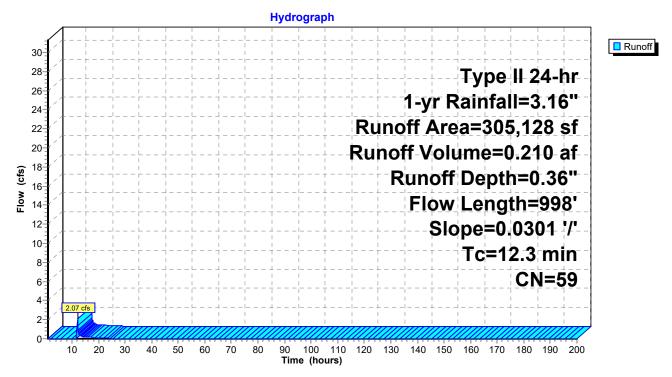
#### Summary for Subcatchment 8S: Pre Dev. Basin 5

Runoff = 2.07 cfs @ 12.10 hrs, Volume= 0.210 af, Depth= 0.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

A	rea (sf)	CN	Description		
2	67,919	60	Woods, Fai	r, HSG B	
	37,209	48	Brush, Goo	d, HSG B	
3	05,128	59	Weighted A	verage	
3	05,128		100.00% Pe	ervious Are	a
Tc	Length	Slope	,	Capacity	Description
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)	
12.3	998	0.030	1 1.36		Kirpich Method, Estimated Tc
					General overland flow k= 2.00

#### Subcatchment 8S: Pre Dev. Basin 5



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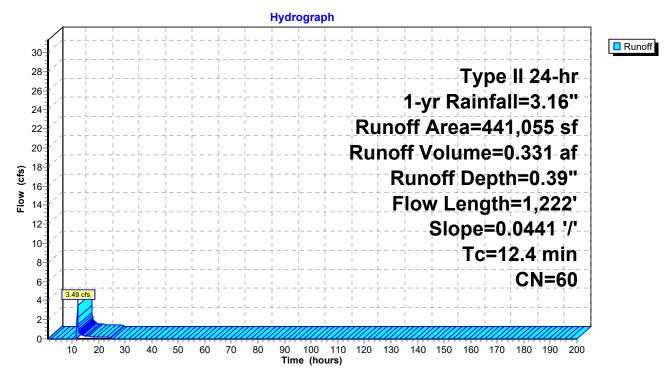
## Summary for Subcatchment 9S: Pre Dev. Basin 6

Runoff = 3.49 cfs @ 12.10 hrs, Volume= 0.331 af, Depth= 0.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

	Α	rea (sf)	CN	Description		
	4	01,238	60	Woods, Fai	r, HSG B	
		39,817	56	Brush, Fair,	, HSG B	
	4	41,055	60	Weighted A	verage	
	4	41,055		100.00% P	ervious Are	ea
	Тс	Length	Slope	,	Capacity	Description
(	(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)	
	12.4	1,222	0.044	1.65		Kirpich Method, Estimated Tc
						General overland flow k= 2.00

#### Subcatchment 9S: Pre Dev. Basin 6



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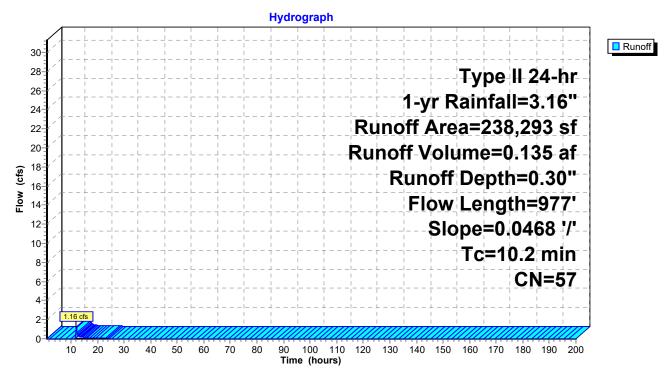
## Summary for Subcatchment 10S: Pre Dev. Basin 7

Runoff = 1.16 cfs @ 12.09 hrs, Volume= 0.135 af, Depth= 0.30"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

A	rea (sf)	CN I	Description			
2	07,447	55 \	Noods, Go	od, HSG B		
	30,846	69 I	Pasture/gra	ssland/rang	ge, Fair, HSG B	
2	38,293	57 \	<b>Neighted A</b>	verage		
2	38,293	•	100.00% Pe	ervious Are	ea	
Tc	Length	Slope	,	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
10.2	977	0.0468	1.60		Kirpich Method, Estimated Tc	
					General overland flow k= 2.00	

#### Subcatchment 10S: Pre Dev. Basin 7



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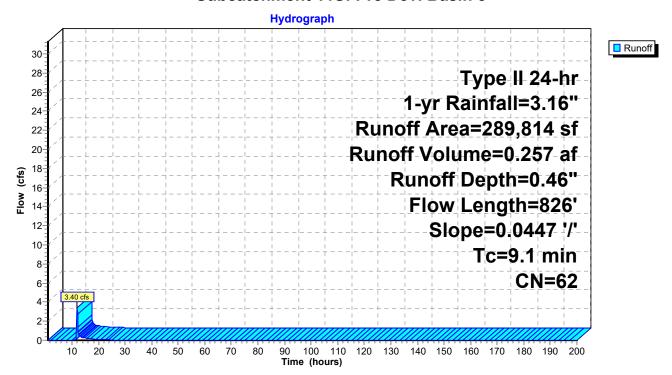
## Summary for Subcatchment 11S: Pre Dev. Basin 8

Runoff = 3.40 cfs @ 12.03 hrs, Volume= 0.257 af, Depth= 0.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

	rea (sf)	CN [	Description							
	230,959	60 ۱	Voods, Fai	r, HSG B						
	53,439	69 5	50-75% Gra	ass cover, l	Fair, HSG B					
	5,416	98 F	Paved park	ing, HSG E	3					
	289,814	62 \	Veighted A	verage						
2	284,398	ç	98.13% Pei	rvious Area	a					
	5,416	1	.87% Impe	ervious Are	ea					
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
9.1	826	0.0447	1.51		Kirpich Method,					
					General overland flow k= 2.00					

#### Subcatchment 11S: Pre Dev. Basin 8



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## **Summary for Subcatchment 12S: Post Basin 1 to SCM**

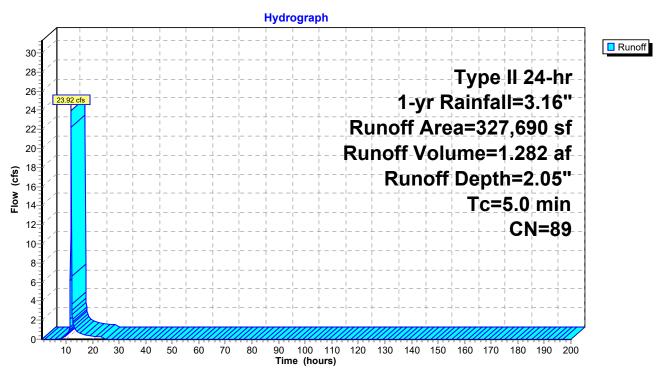
Runoff = 23.92 cfs @ 11.95 hrs, Volume= 1.282 af, Depth= 2.05"

Routed to Pond 1P: Sand Filter -SCM 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

Area	(sf) CN	N De	Description							
104	,903 98	8 Pa	aved parki	ng, HSG A	A					
72	,655 56	6 Br	rush, Fair,	HSG B						
150	,132 98	8 R	oofs, HSG	В						
327	,690 89	9 W	eighted A	verage						
72	,655	22	2.17% Per	vious Area	a					
255	,035	77	7.83% lmp	ervious Ar	rea					
	•	lope	Velocity	Capacity	Description					
(min)	(feet) (	ft/ft)	(ft/sec)	(cfs)						
5.0					Direct Entry,					

#### Subcatchment 12S: Post Basin 1 to SCM



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## Summary for Subcatchment 13S: Post Dev Bypass 1

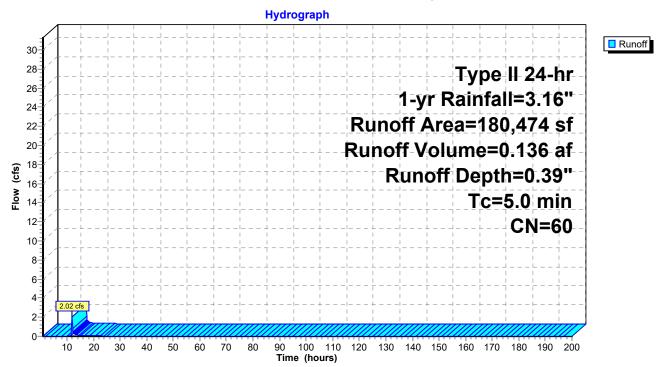
Runoff = 2.02 cfs @ 12.00 hrs, Volume= 0.136 af, Depth= 0.39"

Routed to Link 1L: POA 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

Are	ea (sf)	CN	Description					
12	4,058	60	Woods, Fai	r, HSG B				
4	9,243	56	Brush, Fair,	HSG B				
	7,173	98	Paved park	ing, HSG B	3			
18	0,474	60	Weighted A	verage				
17	3,301		96.03% Pei	vious Area	a			
•	7,173		3.97% Impe	ervious Area	ea			
	Length	Slope	•	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.0					Direct Entry,			

## **Subcatchment 13S: Post Dev Bypass 1**



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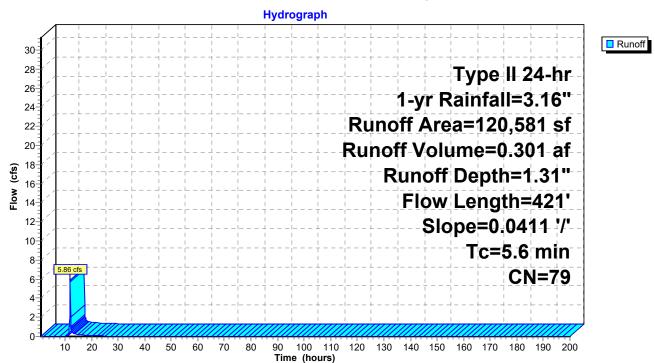
## Summary for Subcatchment 14S: Post Dev. Bypass 2A

Runoff = 5.86 cfs @ 11.97 hrs, Volume= 0.301 af, Depth= 1.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

A	rea (sf)	CN I	Description		
1	20,581	79 \	Noods, Fai	r, HSG D	
1	20,581	•	100.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.6	421	0.0411	1.25		Kirpich Method, Estimated Tc General overland flow k= 2.00

# Subcatchment 14S: Post Dev. Bypass 2A



# Summary for Subcatchment 15S: Post Dev. Basin 2B to SCM

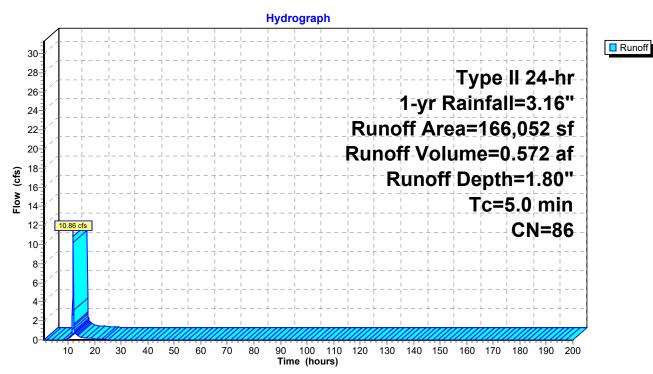
Runoff = 10.86 cfs @ 11.96 hrs, Volume= 0.572 af, Depth= 1.80"

Routed to Pond 2P: Wet Pond SCM 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

Area (sf)	CN	Description							
22,914	98	Paved parking, HSG B							
6,465	48	Brush, Good, HSG B							
95,673	98	Roofs, HSG B							
41,000	58	Woods/grass comb., Good, HSG B							
166,052	86	Weighted Average							
47,465		28.58% Pervious Area							
118,587		71.42% Impervious Area							
Tc Length	Slop	pe Velocity Capacity Description							
(min) (feet)	(ft/	ft) (ft/sec) (cfs)							
5.0		Direct Entry.							

#### Subcatchment 15S: Post Dev. Basin 2B to SCM



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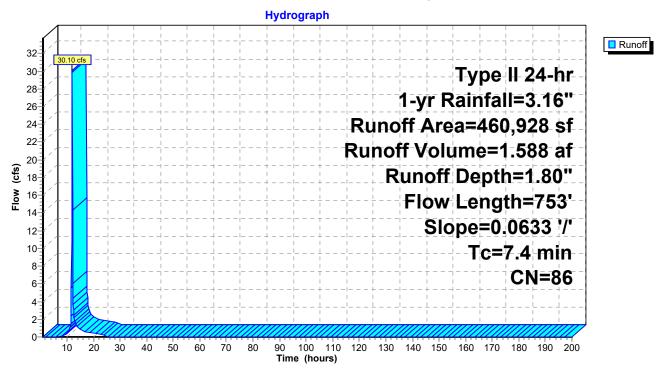
## Summary for Subcatchment 16S: Post Dev. Bypass 2C

Runoff = 30.10 cfs @ 11.99 hrs, Volume= 1.588 af, Depth= 1.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

A	rea (sf)	CN E	Description					
1	73,689	79 V	Voods, Fai	r, HSG D				
1	01,714	98 F	Paved park	ing, HSG D	)			
1	29,620	82 V	Voods/gras	s comb., F	Fair, HSG D			
	55,905	98 F	Roofs, HSC	G D				
4	60,928	86 V	Weighted Average					
3	303,309 65.80% Pervious Area				1			
1	57,619	3	4.20% Imp	ervious Ar	rea			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
7.4	753	0.0633	1.69		Kirpich Method, Estimated Tc			
					General overland flow k= 2.00			

## Subcatchment 16S: Post Dev. Bypass 2C



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## Summary for Subcatchment 18S: Post Dev Bypass 2B

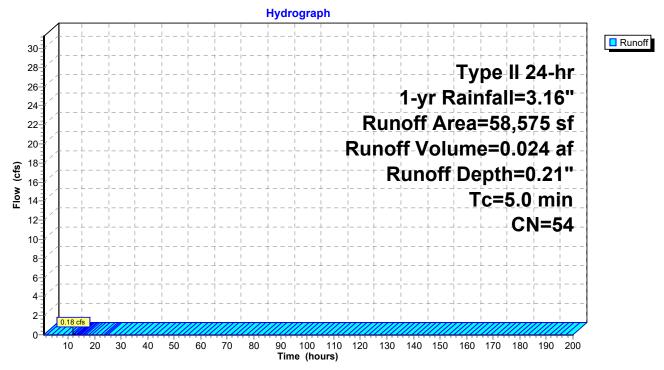
Runoff = 0.18 cfs @ 12.02 hrs, Volume= 0.024 af, Depth= 0.21"

Routed to Link 2L: POA 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

_	Α	rea (sf)	CN	Description						
		54,125	55	Woods, Go	od, HSG B					
_		4,450	48	Brush, Goo	Brush, Good, HSG B					
		58,575	54	Weighted Average						
		58,575		100.00% Pe	ervious Are	ea				
	Tc	Length	Slope	e Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)					
	5.0					Direct Entry				

# Subcatchment 18S: Post Dev Bypass 2B



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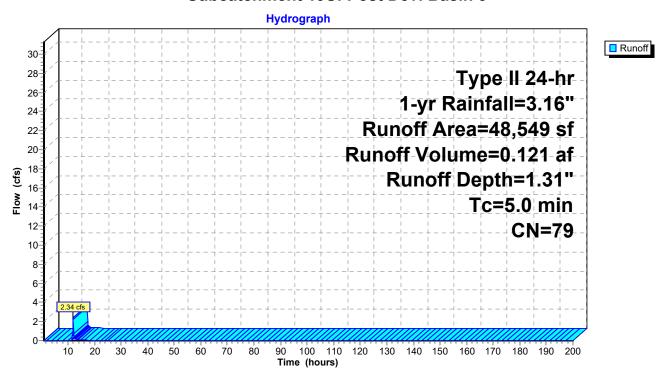
## Summary for Subcatchment 19S: Post Dev. Basin 3

Runoff = 2.34 cfs @ 11.97 hrs, Volume= 0.121 af, Depth= 1.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

A	rea (sf)	CN E	Description					
	48,549	79 5	50-75% Grass cover, Fair, HSG C					
	48,549	1	100.00% Pervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0	•				Direct Entry,			

#### Subcatchment 19S: Post Dev. Basin 3



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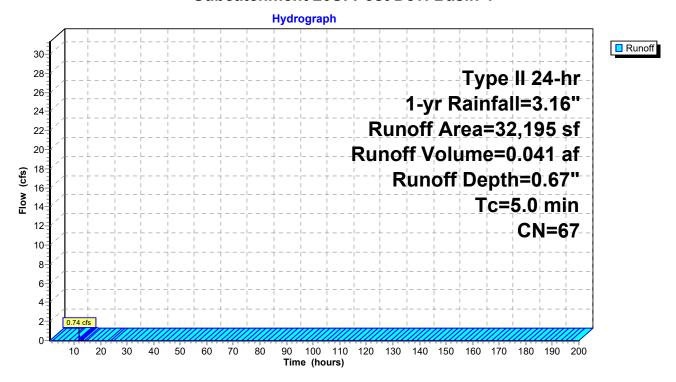
#### Summary for Subcatchment 20S: Post Dev. Basin 4

Runoff = 0.74 cfs @ 11.98 hrs, Volume= 0.041 af, Depth= 0.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

Area (sf)	CN	Description	Description						
6,966	60	Woods, Fai	r, HSG B						
20,201	61	>75% Gras	s cover, Go	Good, HSG B					
5,028	98	Paved park	ing, HSG B	В					
32,195	67	Weighted A	Weighted Average						
27,167		84.38% Per	vious Area	a					
5,028		15.62% Imp	ervious Ar	ırea					
Tc Length	n Slo <sub>l</sub>	pe Velocity	Capacity	/ Description					
(min) (feet)	) (ft/	ft) (ft/sec)	(cfs)						
5.0				Direct Entry.					

#### Subcatchment 20S: Post Dev. Basin 4



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## Summary for Subcatchment 21S: Post Dev. Basin 5 to SCM

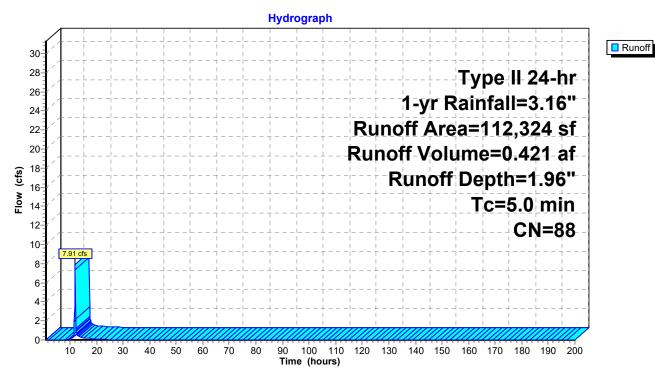
Runoff = 7.91 cfs @ 11.96 hrs, Volume= 0.421 af, Depth= 1.96"

Routed to Pond 3P: Wet Pond SCM 3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

Area (sf)	CN	Description	Description						
40,704	98	Roofs, HSC	B						
22,215	48	Brush, Goo	d, HSG B						
44,766	98	Paved park	ing, HSG E	В					
4,639	98	Water Surfa	ace, 0% imp	np, HSG B					
112,324	88	Weighted A	Weighted Average						
26,854		23.91% Pe	rvious Area	a					
85,470		76.09% Imp	ervious Ar	rea					
Tc Length	Slop	e Velocity	Capacity	Description					
(min) (feet)	(ft/	ft) (ft/sec)	(cfs)						
5.0				Direct Entry,					

#### Subcatchment 21S: Post Dev. Basin 5 to SCM



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#### Summary for Subcatchment 22S: Post Dev. Bypass 5

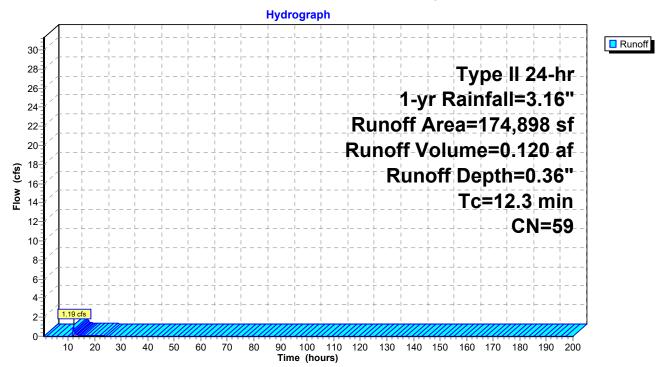
Runoff = 1.19 cfs @ 12.10 hrs, Volume= 0.120 af, Depth= 0.36"

Routed to Link 3L: POA 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

A	rea (sf)	CN	Description					
1	10,888	60	Woods, Fai	r, HSG B				
	51,602	48	Brush, Goo	d, HSG B				
	12,408	98	Paved park	ing, HSG B	3			
1	74,898	59	Weighted A	verage				
1	62,490		92.91% Per	vious Area	a a constant of the constant o			
	12,408		7.09% Impe	ervious Are	ea			
Tc	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)				
12.3					Direct Entry,			

## Subcatchment 22S: Post Dev. Bypass 5



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## Summary for Subcatchment 23S: Post Dev. Basin 6 to SCM

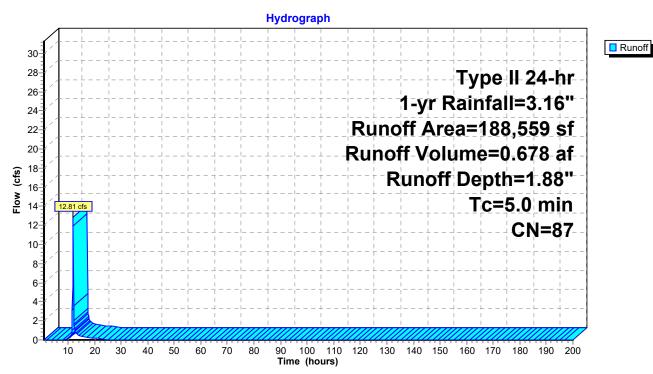
Runoff = 12.81 cfs @ 11.96 hrs, Volume= 0.678 af, Depth= 1.88"

Routed to Pond 4P: Wet Pond SCM 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

 Α	rea (sf)	CN	Description							
	54,466	98	Paved park	ing, HSG B	3					
	58,385	61	>75% Gras	s cover, Go	ood, HSG B					
	60,672	98	Roofs, HSG	βB						
	15,036	98	Water Surfa	ace, 0% imp	o, HSG B					
1	88,559	87	Weighted Average							
	73,421		38.94% Per	rvious Area						
1	15,138		61.06% Imp	pervious Ar	ea					
_										
	Length	Slope	,	Capacity	Description					
 (min)	(feet)	(ft/ft	(ft/sec)	(cfs)						
5.0					Direct Entry,					

#### Subcatchment 23S: Post Dev. Basin 6 to SCM



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## Summary for Subcatchment 24S: Post Dev. Bypass 6

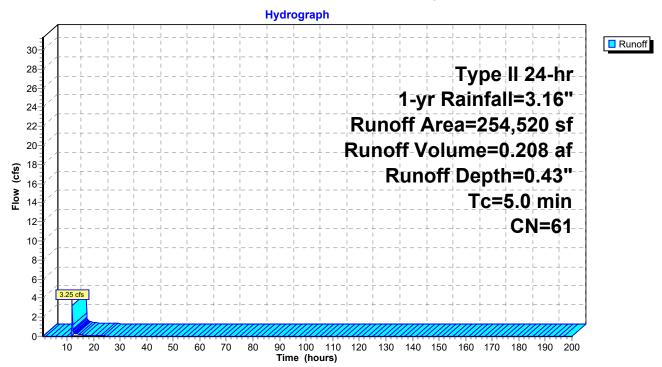
Runoff = 3.25 cfs @ 12.00 hrs, Volume= 0.208 af, Depth= 0.43"

Routed to Link 4L: POA 5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

Are	ea (sf)	CN	Description					
21	5,461	60	Woods, Fai	r, HSG B				
3	4,572	61	>75% Gras	s cover, Go	ood, HSG B			
	4,487	98	Paved park	ing, HSG E	В			
25	4,520	61	Weighted A	verage				
25	0,033		98.24% Per	vious Area	a			
	4,487		1.76% Impe	rvious Are	ea			
	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.0					Direct Entry,			

## Subcatchment 24S: Post Dev. Bypass 6



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## Summary for Subcatchment 25S: Post Dev. Basin 7 to SCM

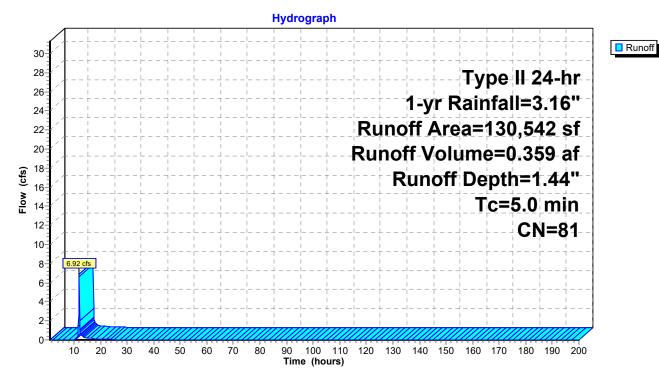
Runoff = 6.92 cfs @ 11.96 hrs, Volume= 0.359 af, Depth= 1.44"

Routed to Pond 5P: Wet Pond SCM 5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

Area (sf)	CN	Description						
50,624	98	Paved parki	ng, HSG B	3				
44,621	48	Brush, Good	d, HSG B					
28,800	98	Roofs, HSG	В					
6,497	98	Water Surfa	ce, 0% imp	p, HSG B				
130,542	81	Weighted A	Weighted Average					
51,118		39.16% Per	vious Area	a				
79,424		60.84% Imp	ervious Ar	rea				
Tc Length	Slop	oe Velocity	Capacity	Description				
(min) (feet)	(ft/	ft) (ft/sec)	(cfs)					
5.0				Direct Entry				

#### Subcatchment 25S: Post Dev. Basin 7 to SCM



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# Summary for Subcatchment 26S: Post Dev. Bypass 7

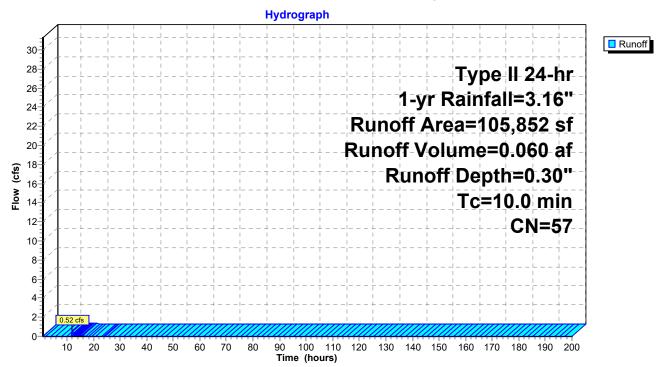
Runoff = 0.52 cfs @ 12.09 hrs, Volume= 0.060 af, Depth= 0.30"

Routed to Link 5L: POA 6

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

Ar	ea (sf)	CN	Description						
3	39,944	55	Woods, Go	od, HSG B	3				
5	52,486	48	48 Brush, Good, HSG B						
1	13,422	98 Paved parking, HSG B							
105,852 57 Weighted Average				verage					
92,430			87.32% Pervious Area						
1	13,422		12.68% Imp	ervious Ar	rea				
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
10.0					Direct Entry,				

## Subcatchment 26S: Post Dev. Bypass 7



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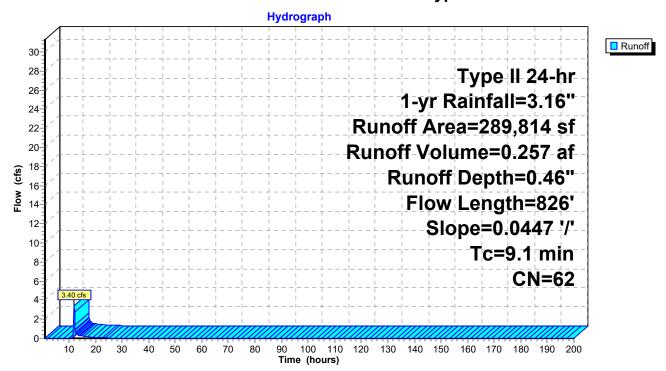
## Summary for Subcatchment 27S: Post Dev. Bypass 8

Runoff = 3.40 cfs @ 12.03 hrs, Volume= 0.257 af, Depth= 0.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 1-yr Rainfall=3.16"

	Area (sf) CN Description								
230,959		60 ۱	Woods, Fair, HSG B						
53,439		69 5	50-75% Grass cover, Fair, HSG B						
5,416 98 Paved parking, HSG B				ing, HSG E	3				
289,814		62 \	Weighted Average						
284,398		ç	98.13% Pervious Area						
5,416		1	1.87% Impervious Area						
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
9.1	826	0.0447	1.51		Kirpich Method,				
					General overland flow k= 2.00				

#### Subcatchment 27S: Post Dev. Bypass 8



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## **Summary for Pond 1P: Sand Filter -SCM 1**

Inflow Area = 7.523 ac, 77.83% Impervious, Inflow Depth = 2.05" for 1-yr event

Inflow = 23.92 cfs @ 11.95 hrs, Volume= 1.282 af

Outflow = 3.07 cfs @ 12.31 hrs, Volume= 1.282 af, Atten= 87%, Lag= 21.0 min

Primary = 3.07 cfs @ 12.31 hrs, Volume= 1.282 af

Routed to Link 1L: POA 1

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Peak Elev= 527.67' @ 12.31 hrs Surf.Area= 6,825 sf Storage= 25,039 cf

Plug-Flow detention time= 113.6 min calculated for 1.281 af (100% of inflow)

Center-of-Mass det. time= 114.3 min ( 921.9 - 807.6 )

Volume	Inve	rt Avail.Sto	rage Storage Description				
#1	524.0	0' 68,25	0 cf Custom	Stage Data (Pi	rismatic)Listed below (Recalc) x 65		
Elevatio	et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
524.0	_	105	0	0			
534.0	00	105	1,050	1,050			
Device #1	Routing Primary	Invert 524.00'	Outlet Device	d Culvert			
			L= 85.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 524.00' / 523.00' S= 0.0118 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 7.07 sf				
#2	#2 Device 1 524.00		· · · · · ·				
#3	Device 1	528.90'	60.0" W x 8.0" H Vert. Main Orifice C= 0.600 Limited to weir flow at low heads				
#4	#4 Device 1 533.00'		25.0' long Overflow 2 End Contraction(s)				

Primary OutFlow Max=3.07 cfs @ 12.31 hrs HW=527.67' (Free Discharge)

**1=Culvert** (Passes 3.07 cfs of 50.11 cfs potential flow)

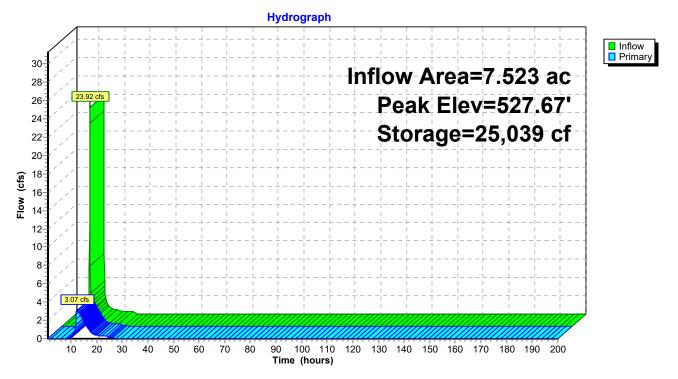
**2=Drawdown** (Orifice Controls 3.07 cfs @ 8.79 fps)

—3=Main Orifice (Controls 0.00 cfs)

-4=Overflow (Controls 0.00 cfs)

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Pond 1P: Sand Filter -SCM 1



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### **Summary for Pond 2P: Wet Pond SCM 2**

Inflow Area = 3.812 ac, 71.42% Impervious, Inflow Depth = 1.80" for 1-yr event

Inflow = 10.86 cfs @ 11.96 hrs, Volume= 0.572 af

Outflow = 0.97 cfs @ 12.53 hrs, Volume= 0.572 af, Atten= 91%, Lag= 34.2 min

Primary = 0.97 cfs @ 12.53 hrs, Volume= 0.572 af

Routed to Link 2L: POA 2

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

Starting Elev= 526.00' Surf.Area= 11,086 sf Storage= 32,033 cf

Peak Elev= 527.04' @ 12.53 hrs Surf.Area= 13,101 sf Storage= 44,570 cf (12,538 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= 241.7 min (1,060.7 - 819.1)

Volume	Inve	rt Avail.Sto	rage Stora	ge Description	
#1	522.00	0' 92,42	29 cf Cust	9 cf Custom Stage Data (Prismatic)Listed below (Recalc)	
Classatia	(	Court Augus	In a Ctarra	Cum Stana	
Elevation		Surf.Area	Inc.Store		
(fee		(sq-ft)	(cubic-feet)	(cubic-feet)	
522.0	00	5,067	0	•	
523.0	00	7,010	6,039	6,039	
524.0	00	7,727	7,369	13,407	
525.0	00	9,219	8,473	21,880	
526.0	00	11,086	10,153	32,033	
527.0	00	13,027	12,057	44,089	
528.0	00	15,043	14,035	58,124	
529.0	00	17,134	16,089	74,213	
530.0	00	19,299	18,217	92,429	
Device	Device Routing Inv		Outlet Dev	rices	
#1	Primary	526.00'		and Culvert	
π ι	i ililiai y	320.00			headwall, Ke= 0.500
					525.55' S= 0.0100 '/' Cc= 0.900
					ight & clean, Flow Area= 7.07 sf
#2	Davisa 1	526 00'			600 Limited to weir flow at low heads
#2 Device 1		526.00'			
#3	Device 1	527.40'			flow Orifice X 2.00 C= 0.600
11.4	<b>D</b>	500.001		weir flow at low hea	
#4	Device 1	528.60'		.0" Horiz. Overflo	
			Limited to	weir flow at low hea	ads

Primary OutFlow Max=0.97 cfs @ 12.53 hrs HW=527.04' (Free Discharge)

**-1=Culvert** (Passes 0.97 cfs of 7.17 cfs potential flow)

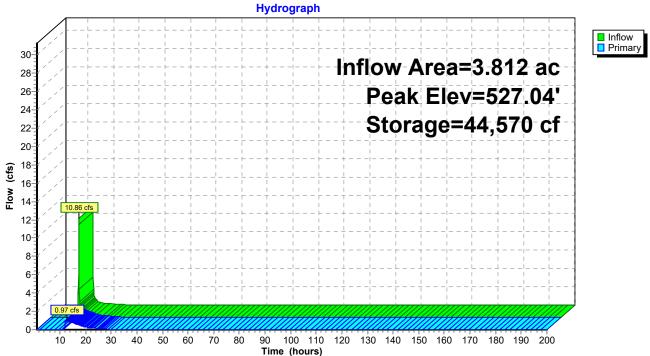
**2=Drawdown** (Orifice Controls 0.97 cfs @ 4.21 fps)

-3=Peakflow Orifice (Controls 0.00 cfs)

-4=Overflow (Controls 0.00 cfs)

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### Pond 2P: Wet Pond SCM 2





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### **Summary for Pond 3P: Wet Pond SCM 3**

Inflow Area = 2.579 ac, 76.09% Impervious, Inflow Depth = 1.96" for 1-yr event

Inflow 7.91 cfs @ 11.96 hrs, Volume= 0.421 af

0.64 cfs @ 12.58 hrs, Volume= Outflow 0.421 af, Atten= 92%, Lag= 37.6 min

Primary 0.64 cfs @ 12.58 hrs, Volume= 0.421 af

Routed to Link 3L: POA 4

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

Starting Elev= 534.00' Surf.Area= 3,130 sf Storage= 15,183 cf

Peak Elev= 535.62' @ 12.58 hrs Surf.Area= 7,739 sf Storage= 24,226 cf (9,043 cf above start)

Plug-Flow detention time= 718.3 min calculated for 0.073 af (17% of inflow)

Center-of-Mass det. time= 175.1 min (986.7 - 811.6)

Volume	Inve	rt Avail.Sto	rage Storage	Description	
#1	530.0	0' 52,86	62 cf Custom	Stage Data (Pr	rismatic)Listed below (Recalc)
Flavetia		Count Amara	In a Chara	Cura Stara	
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(fee		(sq-ft)	(cubic-feet)	(cubic-feet)	
530.0		2,212	0	0	
531.0	00	3,200	2,706	2,706	
532.0	00	4,144	3,672	6,378	
533.0	00	5,168	4,656	11,034	
534.0	00	3,130	4,149	15,183	
535.0	00	6,262	4,696	19,879	
536.0	00	8,640	7,451	27,330	
537.0	00	4,960	6,800	34,130	
538.0	00	9,920	7,440	41,570	
539.0		12,664	11,292	52,862	
Device	Routing	Invert	Outlet Devices	S	
#1	Primary	534.00'	30.0" Round	Culvert	
	•		L= 44.0' RCF	P, square edge h	neadwall, Ke= 0.500
			Inlet / Outlet Ir	nvert= 534.00' /	533.56' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Con	crete pipe, bend	ds & connections, Flow Area= 4.91 sf
#2	Device 1	534.00'		1 1 1	600 Limited to weir flow at low heads
#3	Device 1	536.30'			
				r flow at low hea	
#4	Device 1	537.60'		Horiz. Overflov	

Limited to weir flow at low heads

Primary OutFlow Max=0.64 cfs @ 12.58 hrs HW=535.62' (Free Discharge)

**-1=Culvert** (Passes 0.64 cfs of 12.62 cfs potential flow)

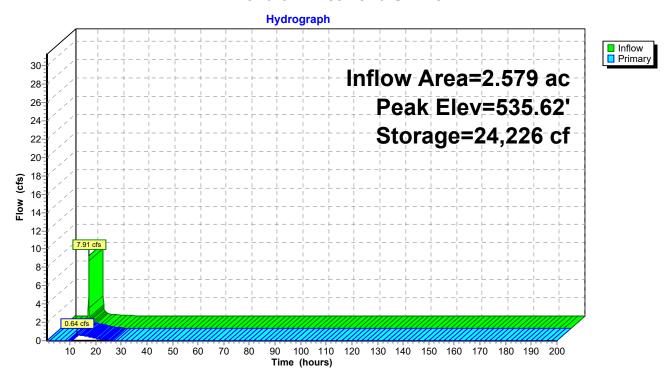
**2=Drawdown** (Orifice Controls 0.64 cfs @ 5.76 fps)

-3=Peak Flow (Controls 0.00 cfs)

**-4=Overflow** (Controls 0.00 cfs)

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Pond 3P: Wet Pond SCM 3



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### Summary for Pond 4P: Wet Pond SCM 4

Inflow Area = 4.329 ac, 61.06% Impervious, Inflow Depth = 1.88" for 1-yr event

Inflow = 12.81 cfs @ 11.96 hrs, Volume= 0.678 af

Outflow = 0.17 cfs @ 19.46 hrs, Volume= 0.353 af, Atten= 99%, Lag= 450.5 min

Primary = 0.17 cfs @ 19.46 hrs, Volume= 0.353 af

Routed to Link 4L: POA 5

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Peak Elev= 524.24' @ 19.46 hrs Surf.Area= 8,083 sf Storage= 22,875 cf

Plug-Flow detention time= 917.8 min calculated for 0.353 af (52% of inflow)

Center-of-Mass det. time= 805.9 min (1,621.3 - 815.4)

Volume	Inve	ert Avail.Sto	rage Storage l	Description	
#1	519.0	00' 67,2	35 cf Custom	Stage Data (Pr	ismatic)Listed below (Recalc)
Elevation	on	Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
519.0	00	1,472	0	0	
520.0	00	2,352	1,912	1,912	
521.0	00	3,406	2,879	4,791	
522.0		4,636	4,021	8,812	
523.0		6,046	5,341	14,153	
524.0		7,648	6,847	21,000	
525.0		9,474	8,561	29,561	
526.0		11,446	10,460	40,021	
527.0		13,570	12,508	52,529	
528.0	00	15,842	14,706	67,235	
Device	Routing	Invert	Outlet Devices	3	
#1	Primary	523.00'	36.0" Round	Culvert	

Device	Routing	Invert	Outlet Devices
#1	Primary	523.00'	36.0" Round Culvert
			L= 45.0' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 523.00' / 521.50' S= 0.0333 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 7.07 sf
#2	Device 1	523.00'	<b>2.5" Vert. Drawdown</b> C= 0.600 Limited to weir flow at low heads
#3	Device 1	525.10'	<b>36.0" W x 3.0" H Vert. Peakflow</b> C= 0.600
			Limited to weir flow at low heads
#4	Device 1	526.50'	<b>48.0"</b> x <b>48.0"</b> Horiz. Overflow C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=0.17 cfs @ 19.46 hrs HW=524.24' (Free Discharge)

**-1=Culvert** (Passes 0.17 cfs of 10.43 cfs potential flow)

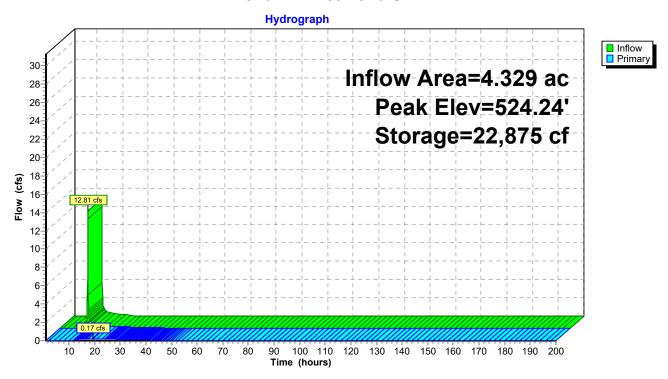
**2=Drawdown** (Orifice Controls 0.17 cfs @ 5.13 fps)

-3=Peakflow (Controls 0.00 cfs)

-4=Overflow (Controls 0.00 cfs)

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Pond 4P: Wet Pond SCM 4



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## **Summary for Pond 5P: Wet Pond SCM 5**

Inflow Area = 2.997 ac, 60.84% Impervious, Inflow Depth = 1.44" for 1-yr event

Inflow = 6.92 cfs @ 11.96 hrs, Volume= 0.359 af

Outflow = 0.47 cfs @ 12.88 hrs, Volume= 0.359 af, Atten= 93%, Lag= 55.2 min

Primary = 0.47 cfs @ 12.88 hrs, Volume= 0.359 af

Routed to Link 5L: POA 6

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

Starting Elev= 513.00' Surf.Area= 4,340 sf Storage= 13,131 cf

Peak Elev= 514.44' @ 12.88 hrs Surf.Area= 5,982 sf Storage= 20,772 cf (7,641 cf above start)

Plug-Flow detention time= 854.2 min calculated for 0.058 af (16% of inflow)

Center-of-Mass det. time= 222.6 min ( 1,058.6 - 836.0 )

Volume	Inve	rt Avail.Sto	rage Sto	orage Description
#1	509.00	0' 45,4	75 cf <b>C</b> u	stom Stage Data (Prismatic)Listed below (Recalc)
Clayatia	n (	Surf.Area	Inc.Sto	ora Cum Stora
Elevatio				
(fee		(sq-ft)	(cubic-fe	<del></del>
509.0		2,355		0 0
510.0		2,789	2,5	
511.0	0	3,517	3,1	53 5,725
512.0	0	3,477	3,4	97 9,222
513.0	0	4,340	3,9	09 13,131
514.0	0	5,752	5,0	46 18,177
515.0	0	6,271	6,0	12 24,188
516.0	0	6,813	6,5	42 30,730
517.0	0	7,377	7,0	95 37,825
518.0		7,922	7,6	·
Device	Routing	Invert	Outlet D	evices
#1	Primary	513.00'	36.0" R	ound Culvert
			L= 79.0'	RCP, square edge headwall, Ke= 0.500
			Inlet / O	utlet Invert= 513.00' / 512.00' S= 0.0127 '/' Cc= 0.900
			n= 0.013	3 Concrete pipe, bends & connections, Flow Area= 7.07 sf
#2	Device 1	513.00'		t. Drawdown C= 0.600 Limited to weir flow at low heads
#3	Device 1	515.10'		x 3.0" H Vert. Peakflow Orifice X 2.00 C= 0.600
		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		to weir flow at low heads
#4	Device 1	516.70'		<b>48.0" Horiz. Overflow</b> C= 0.600
		5.5.7.6		to weir flow at low heads

Primary OutFlow Max=0.47 cfs @ 12.88 hrs HW=514.44' (Free Discharge)

**\_1=Culvert** (Passes 0.47 cfs of 13.74 cfs potential flow)

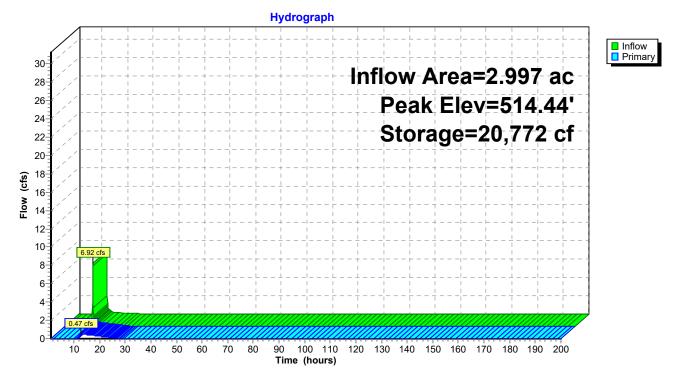
**2=Drawdown** (Orifice Controls 0.47 cfs @ 5.44 fps)

-3=Peakflow Orifice (Controls 0.00 cfs)

**-4=Overflow** (Controls 0.00 cfs)

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### Pond 5P: Wet Pond SCM 5



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# Summary for Link 1L: POA 1

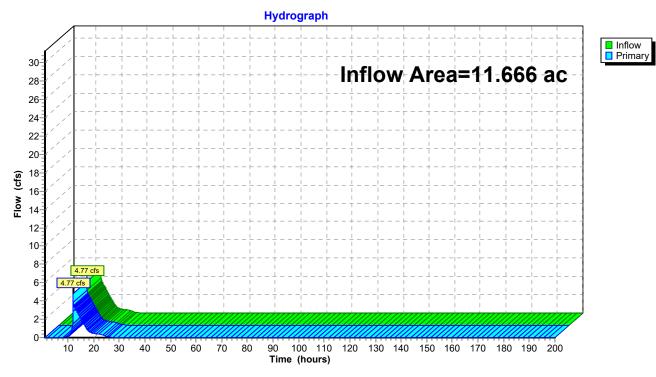
Inflow Area = 11.666 ac, 51.60% Impervious, Inflow Depth = 1.46" for 1-yr event

Inflow = 4.77 cfs @ 12.02 hrs, Volume= 1.418 af

Primary = 4.77 cfs @ 12.02 hrs, Volume= 1.418 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

### Link 1L: POA 1



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# **Summary for Link 2L: POA 2**

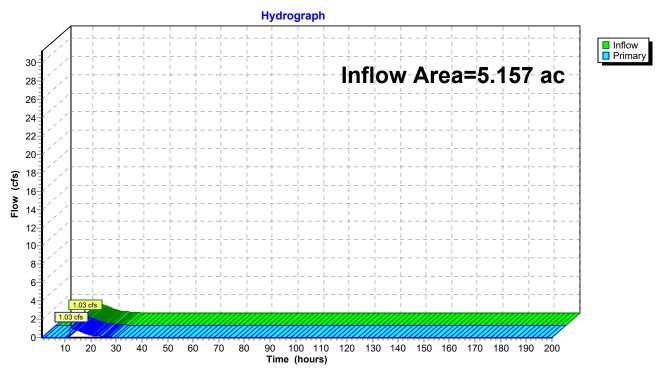
Inflow Area = 5.157 ac, 52.79% Impervious, Inflow Depth = 1.39" for 1-yr event

Inflow = 1.03 cfs @ 12.36 hrs, Volume= 0.596 af

Primary = 1.03 cfs @ 12.36 hrs, Volume= 0.596 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

### Link 2L: POA 2



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# Summary for Link 3L: POA 4

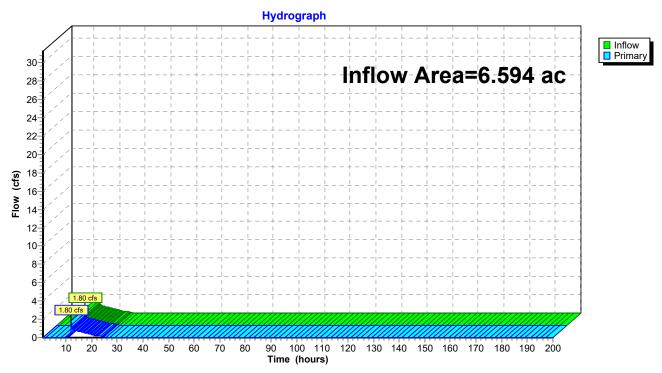
Inflow Area = 6.594 ac, 34.08% Impervious, Inflow Depth = 0.99" for 1-yr event

Inflow = 1.80 cfs @ 12.11 hrs, Volume= 0.542 af

Primary = 1.80 cfs @ 12.11 hrs, Volume= 0.542 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

### Link 3L: POA 4



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# **Summary for Link 4L: POA 5**

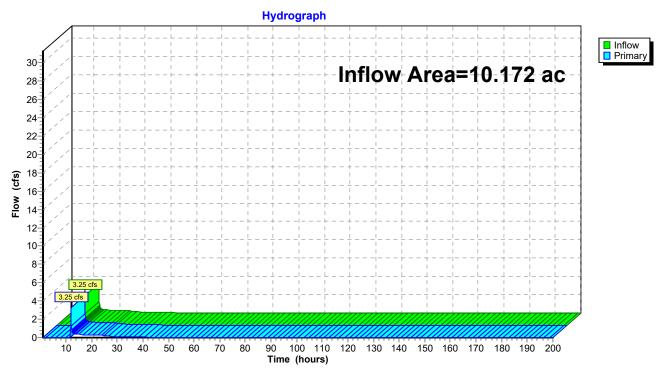
Inflow Area = 10.172 ac, 27.00% Impervious, Inflow Depth = 0.66" for 1-yr event

Inflow = 3.25 cfs @ 12.00 hrs, Volume= 0.561 af

Primary = 3.25 cfs @ 12.00 hrs, Volume= 0.561 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

### Link 4L: POA 5



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# Summary for Link 5L: POA 6

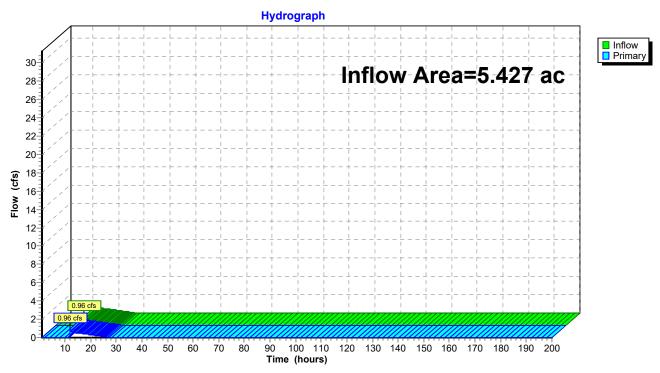
Inflow Area = 5.427 ac, 39.28% Impervious, Inflow Depth = 0.93" for 1-yr event

Inflow = 0.96 cfs @ 12.10 hrs, Volume= 0.419 af

Primary = 0.96 cfs @ 12.10 hrs, Volume= 0.419 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

### Link 5L: POA 6



Tc=5.0 min CN=54 Runoff=0.65 cfs 0.047 af

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Time span=1.00-200.00 hrs, dt=0.10 hrs, 1991 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: Pre Dev. Basin 1         Runoff Area=366,801 sf         0.00% Impervious         Runoff Depth=1.82"           Flow Length=560'         Slope=0.0630 '/'         Tc=5.9 min         CN=79         Runoff=24.78 cfs         1.276 af
Subcatchment2S: Pre Dev. Basin 2A Flow Length=803' Runoff Area=250,337 sf 0.00% Impervious Runoff Depth=1.82" Slope=0.0650 '/' Tc=7.7 min CN=79 Runoff=16.53 cfs 0.871 af
Subcatchment3S: Pre Dev. Basin 2B Flow Length=577' Runoff Area=132,113 sf 0.00% Impervious Runoff Depth=0.68" Slope=0.0451 '/' Tc=6.9 min CN=60 Runoff=2.85 cfs 0.171 af
Subcatchment4S: Pre Dev. Basin 2C Runoff Area=467,738 sf 32.11% Impervious Runoff Depth=2.38" Flow Length=753' Slope=0.0633 '/' Tc=7.4 min CN=86 Runoff=40.05 cfs 2.133 af
Subcatchment6S: Pre Dev. Basin 3 Runoff Area=52,001 sf 0.00% Impervious Runoff Depth=1.74" Slope=0.0170 '/' Tc=5.1 min CN=78 Runoff=3.35 cfs 0.174 af
Subcatchment7S: Pre Dev. Basin 4 Runoff Area=130,388 sf 0.00% Impervious Runoff Depth=0.63" Slope=0.0221 '/' Tc=8.9 min CN=59 Runoff=2.26 cfs 0.157 af
Subcatchment8S: Pre Dev. Basin 5 Flow Length=998' Runoff Area=305,128 sf 0.00% Impervious Runoff Depth=0.63" Slope=0.0301 '/' Tc=12.3 min CN=59 Runoff=4.56 cfs 0.368 af
Subcatchment9S: Pre Dev. Basin 6 Runoff Area=441,055 sf 0.00% Impervious Runoff Depth=0.68" Flow Length=1,222' Slope=0.0441 '/' Tc=12.4 min CN=60 Runoff=7.29 cfs 0.570 af
Subcatchment10S: Pre Dev. Basin 7 Runoff Area=238,293 sf 0.00% Impervious Runoff Depth=0.54" Flow Length=977' Slope=0.0468 '/' Tc=10.2 min CN=57 Runoff=2.89 cfs 0.247 af
Subcatchment11S: Pre Dev. Basin 8 Flow Length=826' Runoff Area=289,814 sf 1.87% Impervious Runoff Depth=0.77" Slope=0.0447 '/' Tc=9.1 min CN=62 Runoff=6.67 cfs 0.428 af
Subcatchment12S: Post Basin 1 to SCM Runoff Area=327,690 sf 77.83% Impervious Runoff Depth=2.65" Tc=5.0 min CN=89 Runoff=30.61 cfs 1.664 af
Subcatchment13S: Post Dev Bypass 1 Runoff Area=180,474 sf 3.97% Impervious Runoff Depth=0.68" Tc=5.0 min CN=60 Runoff=4.04 cfs 0.233 af
Subcatchment14S: Post Dev. Bypass 2A Flow Length=421' Runoff Area=120,581 sf 0.00% Impervious Runoff Depth=1.82" Slope=0.0411 '/' Tc=5.6 min CN=79 Runoff=8.13 cfs 0.419 af
Subcatchment15S: Post Dev. Basin 2B Runoff Area=166,052 sf 71.42% Impervious Runoff Depth=2.38" Tc=5.0 min CN=86 Runoff=14.21 cfs 0.757 af
Subcatchment16S: Post Dev. Bypass 2C Runoff Area=460,928 sf 34.20% Impervious Runoff Depth=2.38" Flow Length=753' Slope=0.0633 '/' Tc=7.4 min CN=86 Runoff=39.47 cfs 2.102 af
Subcatchment18S: Post Dev Bypass 2B Runoff Area=58,575 sf 0.00% Impervious Runoff Depth=0.42"

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Primary=3.30 cfs 0.761 af

Runoff Area=48,549 sf 0.00% Impervious Runoff Depth=1.82" Subcatchment19S: Post Dev. Basin 3 Tc=5.0 min CN=79 Runoff=3.25 cfs 0.169 af Subcatchment20S: Post Dev. Basin 4 Runoff Area=32,195 sf 15.62% Impervious Runoff Depth=1.04" Tc=5.0 min CN=67 Runoff=1.20 cfs 0.064 af Subcatchment21S: Post Dev. Basin 5 to Runoff Area=112,324 sf 76.09% Impervious Runoff Depth=2.56" Tc=5.0 min CN=88 Runoff=10.20 cfs 0.551 af Subcatchment22S: Post Dev. Bypass 5 Runoff Area=174,898 sf 7.09% Impervious Runoff Depth=0.63" Tc=12.3 min CN=59 Runoff=2.61 cfs 0.211 af Subcatchment 23S: Post Dev. Basin 6 to Runoff Area=188,559 sf 61.06% Impervious Runoff Depth=2.47" Tc=5.0 min CN=87 Runoff=16.63 cfs 0.892 af Runoff Area=254,520 sf 1.76% Impervious Runoff Depth=0.72" Subcatchment24S: Post Dev. Bypass 6 Tc=5.0 min CN=61 Runoff=6.21 cfs 0.352 af Subcatchment25S: Post Dev. Basin 7 to Runoff Area=130,542 sf 60.84% Impervious Runoff Depth=1.97" Tc=5.0 min CN=81 Runoff=9.42 cfs 0.492 af Runoff Area=105,852 sf 12.68% Impervious Runoff Depth=0.54" Subcatchment26S: Post Dev. Bypass 7 Tc=10.0 min CN=57 Runoff=1.29 cfs 0.110 af Runoff Area=289.814 sf 1.87% Impervious Runoff Depth=0.77" Subcatchment27S: Post Dev. Bypass 8 Flow Length=826' Slope=0.0447 '/' Tc=9.1 min CN=62 Runoff=6.67 cfs 0.428 af Pond 1P: Sand Filter -SCM 1 Peak Elev=528.85' Storage=33,134 cf Inflow=30.61 cfs 1.664 af Outflow=3.57 cfs 1.664 af Pond 2P: Wet Pond SCM 2 Peak Elev=527.38' Storage=49,143 cf Inflow=14.21 cfs 0.757 af Outflow=1.17 cfs 0.757 af Pond 3P: Wet Pond SCM 3 Peak Elev=536.00' Storage=27,369 cf Inflow=10.20 cfs 0.551 af Outflow=0.72 cfs 0.551 af Pond 4P: Wet Pond SCM 4 Peak Elev=525.04' Storage=29,958 cf Inflow=16.63 cfs 0.892 af Outflow=0.23 cfs 0.567 af Pond 5P: Wet Pond SCM 5 Peak Elev=515.00' Storage=24,189 cf Inflow=9.42 cfs 0.492 af Outflow=0.57 cfs 0.492 af Link 1L: POA 1 Inflow=7.16 cfs 1.897 af Primary=7.16 cfs 1.897 af Link 2L: POA 2 Inflow=1.67 cfs 0.804 af Primary=1.67 cfs 0.804 af Link 3L: POA 4 Inflow=3.30 cfs 0.761 af **32044.0000 - CZ**Prepared by Thomas & Hutton

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**Link 4L: POA 5**Inflow=6.30 cfs 0.919 af
Primary=6.30 cfs 0.919 af

**Link 5L: POA 6**Inflow=1.80 cfs 0.602 af Primary=1.80 cfs 0.602 af

Total Runoff Area = 122.250 ac Runoff Volume = 14.884 af Average Runoff Depth = 1.46" 80.94% Pervious = 98.954 ac 19.06% Impervious = 23.297 ac

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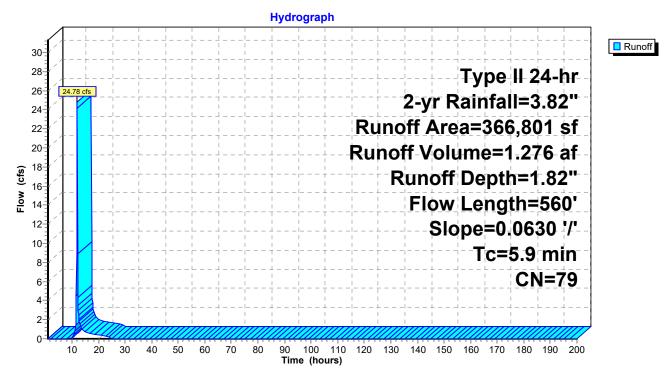
### Summary for Subcatchment 1S: Pre Dev. Basin 1

Runoff = 24.78 cfs @ 11.97 hrs, Volume= 1.276 af, Depth= 1.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

A	rea (sf)	CN I	Description		
3	42,042	79 \	Noods, Fai	r, HSG D	
	24,759	73 I	Brush, Goo	d, HSG D	
3	66,801	79 \	Neighted A	verage	
3	66,801	•	100.00% Pe	ervious Are	a
Tc	Length	Slope	,	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.9	560	0.0630	1.58		Kirpich Method, Estimated Tc
					General overland flow k= 2.00

#### Subcatchment 1S: Pre Dev. Basin 1



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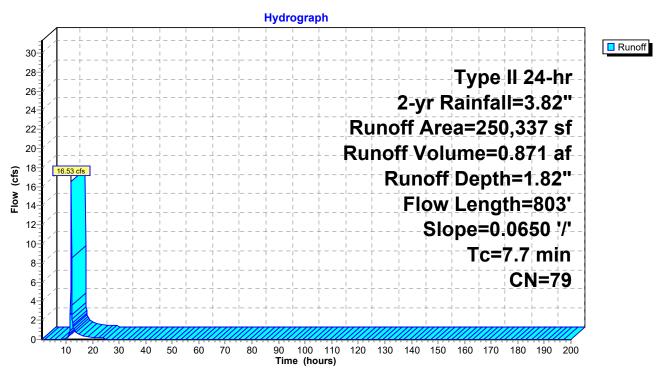
## Summary for Subcatchment 2S: Pre Dev. Basin 2A

Runoff = 16.53 cfs @ 11.99 hrs, Volume= 0.871 af, Depth= 1.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

A	rea (sf)	CN E	Description		
2	50,337	79 V	Voods, Fai	r, HSG D	
2	50,337	100.00% Pervious Are			a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.7	803	0.0650	1.74	(010)	Kirpich Method, Estimated Tc General overland flow k= 2.00

#### Subcatchment 2S: Pre Dev. Basin 2A



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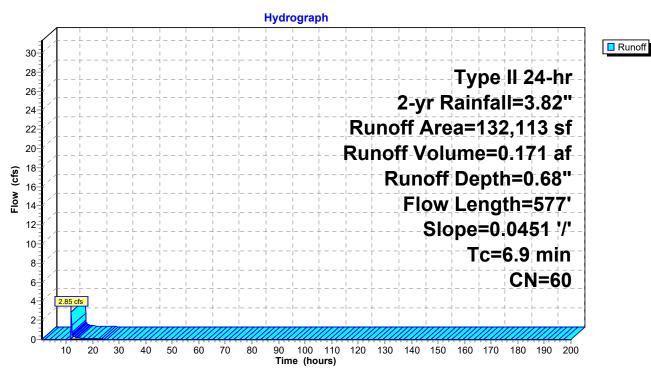
## Summary for Subcatchment 3S: Pre Dev. Basin 2B

Runoff = 2.85 cfs @ 12.00 hrs, Volume= 0.171 af, Depth= 0.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

_	Α	rea (sf)	CN [	Description		
	1	32,113	60 \	Noods, Fai	r, HSG B	
	1	32,113	1	100.00% Pe	ervious Are	ea e
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	6.9	577	0.0451	1.40	(===)	Kirpich Method, Estimated Tc General overland flow k= 2.00

#### Subcatchment 3S: Pre Dev. Basin 2B



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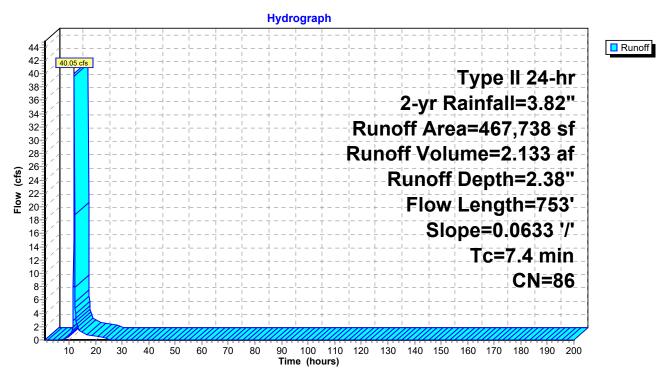
## Summary for Subcatchment 4S: Pre Dev. Basin 2C

Runoff = 40.05 cfs @ 11.98 hrs, Volume= 2.133 af, Depth= 2.38"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

	Area (sf)	CN [	Description				
	173,689	79 V	Voods, Fai	r, HSG D			
	94,275	98 F	Paved park	ing, HSG D			
	143,869	82 V	Voods/gras	ss comb., F	Fair, HSG D		
	55,905	98 F	Roofs, HSC	B D			
	467,738	86 V	Veighted A	verage			
;	317,558	6	7.89% Per	vious Area	l		
	150,180	3	32.11% Impervious Area				
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
7.4	753	0.0633	1.69		Kirpich Method, Estimated Tc		
					General overland flow k= 2.00		

#### Subcatchment 4S: Pre Dev. Basin 2C



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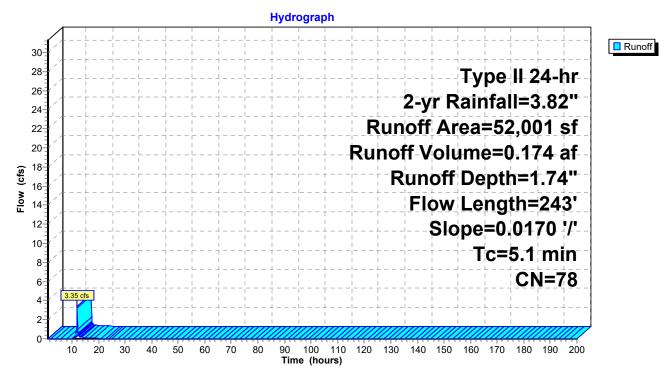
## Summary for Subcatchment 6S: Pre Dev. Basin 3

Runoff = 3.35 cfs @ 11.96 hrs, Volume= 0.174 af, Depth= 1.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

	Area (sf)	CN	Description				
	9,925	73	Woods, Fai	r, HSG C			
	42,076	79	50-75% Gra	ass cover, f	Fair, HSG C		
	52,001	78	Weighted Average				
	52,001		100.00% Pe	ervious Are	a		
To	9	Slope	,	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
5.1	243	0.0170	0.79		Kirpich Method, Estimated Tc		
					General overland flow k= 2.00		

#### Subcatchment 6S: Pre Dev. Basin 3



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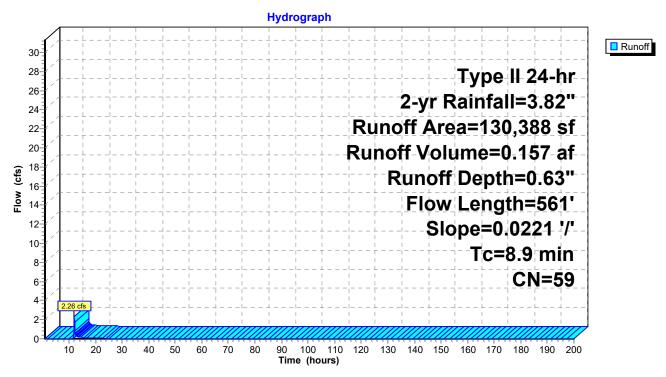
## Summary for Subcatchment 7S: Pre Dev. Basin 4

Runoff = 2.26 cfs @ 12.03 hrs, Volume= 0.157 af, Depth= 0.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

A	rea (sf)	CN	Description			
1	04,102	60	Woods, Fai	r, HSG B		
	26,286	56	Brush, Fair,	HSG B		
1	30,388	59	Weighted A	verage		
1	30,388		100.00% Pe	ervious Are	a	
Tc	Length	Slope	,	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
8.9	561	0.0221	1.06		Kirpich Method,	
					General overland flow	k= 2.00

#### Subcatchment 7S: Pre Dev. Basin 4



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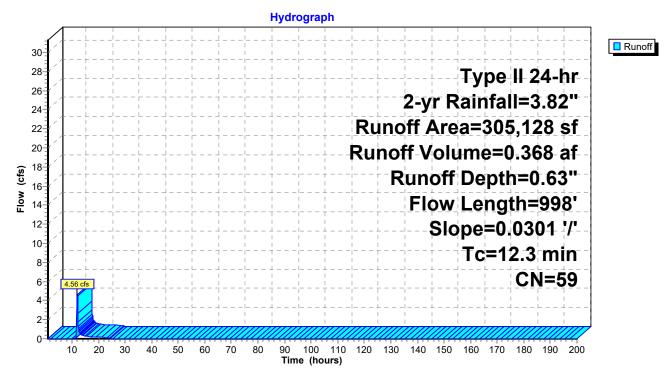
## Summary for Subcatchment 8S: Pre Dev. Basin 5

Runoff = 4.56 cfs @ 12.09 hrs, Volume= 0.368 af, Depth= 0.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

A	rea (sf)	CN	Description		
2	67,919	60	Woods, Fai	r, HSG B	
	37,209	48	Brush, Goo	d, HSG B	
3	05,128	59	Weighted A	verage	
3	05,128		100.00% Pe	ervious Are	a
Tc	Length	Slope	,	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.3	998	0.0301	1.36		Kirpich Method, Estimated Tc
					General overland flow k= 2.00

#### Subcatchment 8S: Pre Dev. Basin 5



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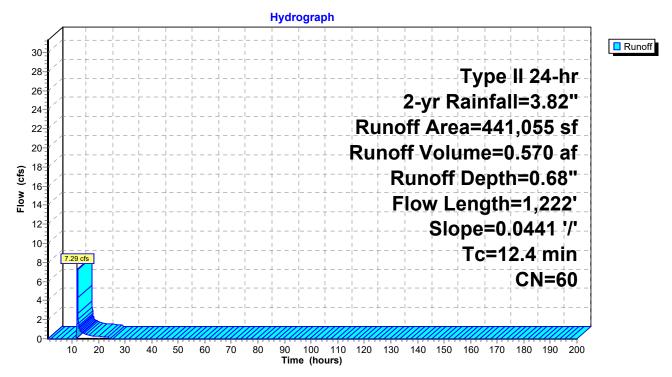
## Summary for Subcatchment 9S: Pre Dev. Basin 6

Runoff = 7.29 cfs @ 12.08 hrs, Volume= 0.570 af, Depth= 0.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

	Α	rea (sf)	CN	Description		
	4	01,238	60	Woods, Fai	r, HSG B	
		39,817	56	Brush, Fair,	, HSG B	
	4	41,055	60	Weighted A	verage	
	4	41,055		100.00% P	ervious Are	ea
	Тс	Length	Slope	,	Capacity	Description
(	(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)	
	12.4	1,222	0.044	1.65		Kirpich Method, Estimated Tc
						General overland flow k= 2.00

#### Subcatchment 9S: Pre Dev. Basin 6



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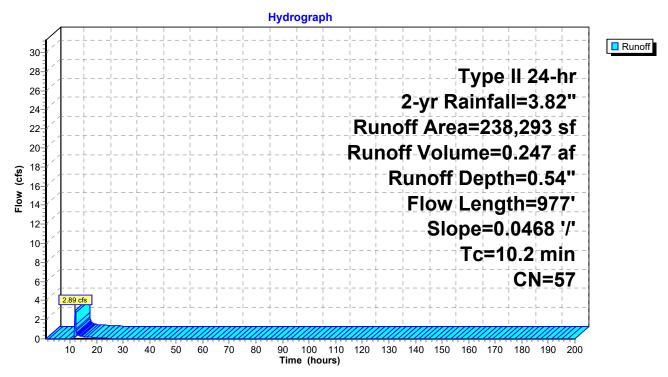
## Summary for Subcatchment 10S: Pre Dev. Basin 7

Runoff = 2.89 cfs @ 12.06 hrs, Volume= 0.247 af, Depth= 0.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

Aı	rea (sf)	CN	Description						
2	07,447	55	Woods, Go	od, HSG B					
	30,846	69	Pasture/gra	ssland/rang	ge, Fair, HSG B				
2	38,293	57	Weighted A	verage					
2	38,293		100.00% Pervious Area						
_				_					
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
10.2	977	0.0468	1.60		Kirpich Method, Estimated Tc				
					General overland flow k= 2.00				

#### Subcatchment 10S: Pre Dev. Basin 7



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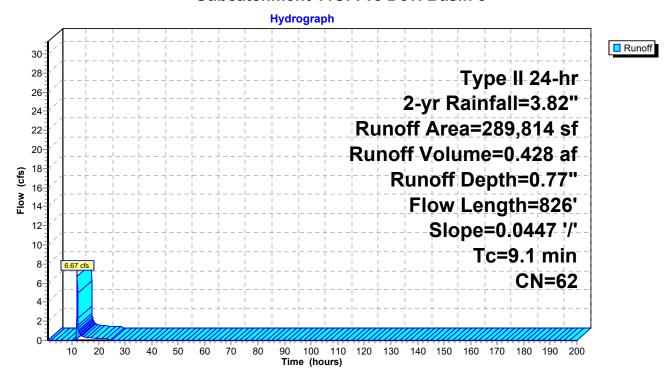
### Summary for Subcatchment 11S: Pre Dev. Basin 8

Runoff = 6.67 cfs @ 12.02 hrs, Volume= 0.428 af, Depth= 0.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

	rea (sf)	CN [	Description						
	230,959	60 ۱	Voods, Fai	r, HSG B					
	53,439	69 5	50-75% Gra	ass cover, l	Fair, HSG B				
	5,416	98 F	Paved park	ing, HSG E	3				
	289,814	62 \	Veighted A	verage					
2	284,398	ç	98.13% Pei	rvious Area	a				
	5,416	1	.87% Impe	ervious Are	ea				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
9.1	826	0.0447	1.51		Kirpich Method,				
					General overland flow k= 2.00				

#### Subcatchment 11S: Pre Dev. Basin 8



# Summary for Subcatchment 12S: Post Basin 1 to SCM

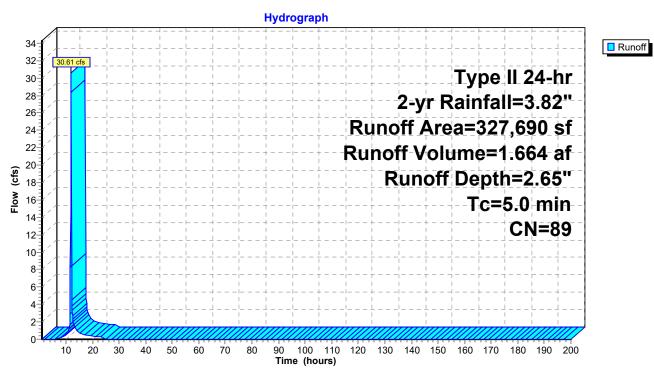
Runoff = 30.61 cfs @ 11.95 hrs, Volume= 1.664 af, Depth= 2.65"

Routed to Pond 1P: Sand Filter -SCM 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

Area	(sf) CN	N De	Description							
104	,903 98	8 Pa	aved parki	ng, HSG A	A					
72	,655 56	6 Br	rush, Fair,	HSG B						
150	,132 98	8 R	oofs, HSG	В						
327	,690 89	9 W	eighted A	verage						
72	,655	22	2.17% Per	vious Area	a					
255	,035	77	7.83% lmp	ervious Ar	rea					
	•	lope	Velocity	Capacity	Description					
(min)	(feet) (	ft/ft)	(ft/sec)	(cfs)						
5.0					Direct Entry,					

#### Subcatchment 12S: Post Basin 1 to SCM



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## Summary for Subcatchment 13S: Post Dev Bypass 1

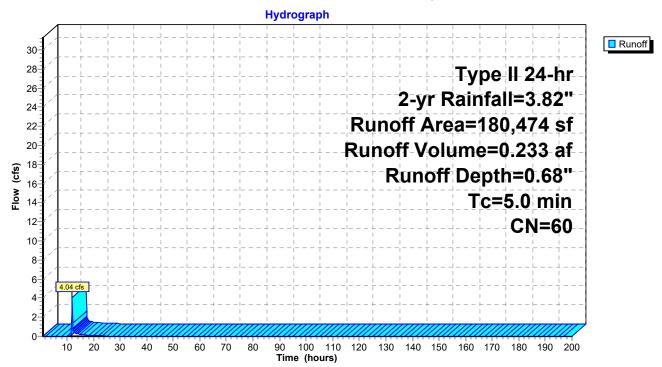
Runoff = 4.04 cfs @ 11.99 hrs, Volume= 0.233 af, Depth= 0.68"

Routed to Link 1L: POA 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

Are	ea (sf)	CN	Description						
12	4,058	60	Woods, Fai	r, HSG B					
4	9,243	56	Brush, Fair,	HSG B					
	7,173	98	Paved park	ing, HSG B	3				
18	0,474	60	Weighted A	verage					
17	3,301		96.03% Pei	vious Area	a				
•	7,173		3.97% Impe	ervious Area	ea				
	Length	Slope	•	Capacity	Description				
(min)	(feet)	(ft/ft)	ft) (ft/sec) (cfs)						
5.0					Direct Entry,				

## **Subcatchment 13S: Post Dev Bypass 1**



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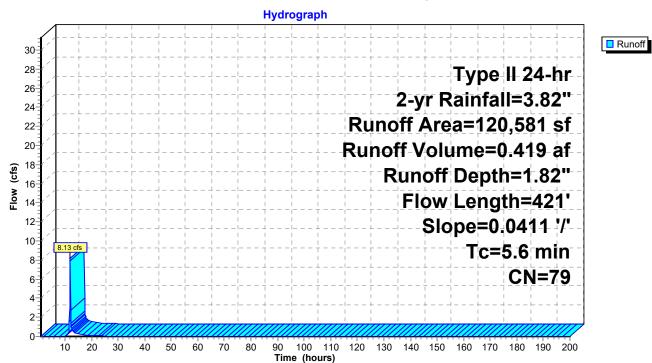
# Summary for Subcatchment 14S: Post Dev. Bypass 2A

Runoff = 8.13 cfs @ 11.97 hrs, Volume= 0.419 af, Depth= 1.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

_	Α	rea (sf)	CN	Description		
	1	20,581	79	Woods, Fai	r, HSG D	
	1	20,581		100.00% P	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	5.6	421	0.0411	1.25		Kirpich Method, Estimated Tc General overland flow k= 2.00

# Subcatchment 14S: Post Dev. Bypass 2A



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# Summary for Subcatchment 15S: Post Dev. Basin 2B to SCM

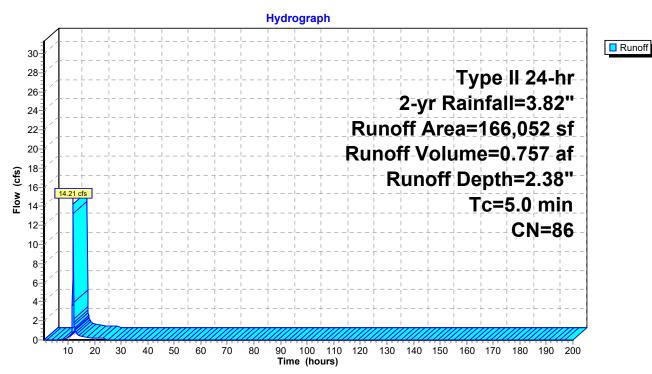
Runoff = 14.21 cfs @ 11.96 hrs, Volume= 0.757 af, Depth= 2.38"

Routed to Pond 2P: Wet Pond SCM 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

Area (sf)	CN	Description						
22,914	98	Paved parking	, HSG B	В				
6,465	48	Brush, Good, I	HSG B					
95,673	98	Roofs, HSG B						
41,000	58	Woods/grass o	comb., G	Good, HSG B				
166,052	86	Weighted Aver	age					
47,465		28.58% Pervio	us Area	a				
118,587		71.42% Imperv	ious Ar	rea				
Tc Length	Slop	e Velocity Ca	apacity	Description				
(min) (feet)	(ft/	(t) (ft/sec) (cfs)						
5.0				Direct Entry				

#### Subcatchment 15S: Post Dev. Basin 2B to SCM



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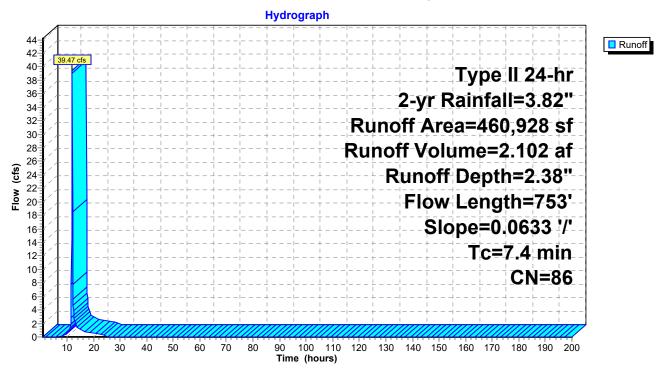
# Summary for Subcatchment 16S: Post Dev. Bypass 2C

Runoff = 39.47 cfs @ 11.98 hrs, Volume= 2.102 af, Depth= 2.38"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

Are	ea (sf)	CN D	escription		
17	3,689	79 V	Voods, Fai	r, HSG D	
10	1,714	98 P	aved park	ing, HSG D	
12	9,620	82 V	Voods/gras	s comb., F	air, HSG D
5	5,905	98 F	Roofs, HSG	D D	
46	0,928	86 V	Veighted A	verage	
30	3,309	6	5.80% Per	vious Area	
15	7,619	3	4.20% Imp	ervious Ar	ea
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.4	753	0.0633	1.69		Kirpich Method, Estimated Tc
					General overland flow k= 2.00

# Subcatchment 16S: Post Dev. Bypass 2C



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# Summary for Subcatchment 18S: Post Dev Bypass 2B

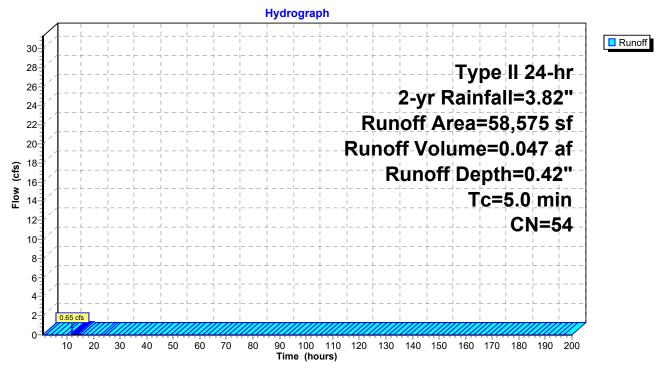
Runoff = 0.65 cfs @ 12.00 hrs, Volume= 0.047 af, Depth= 0.42"

Routed to Link 2L: POA 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

_	Α	rea (sf)	CN	Description						
_		54,125	55	Woods, Go	od, HSG B	}				
_		4,450	48	Brush, Goo	d, HSG B					
_		58,575	54	Weighted Average						
		58,575		100.00% Pe	ervious Are	ea				
	Tc	Length	Slop	e Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)					
	5.0					Direct Entry.				

# Subcatchment 18S: Post Dev Bypass 2B



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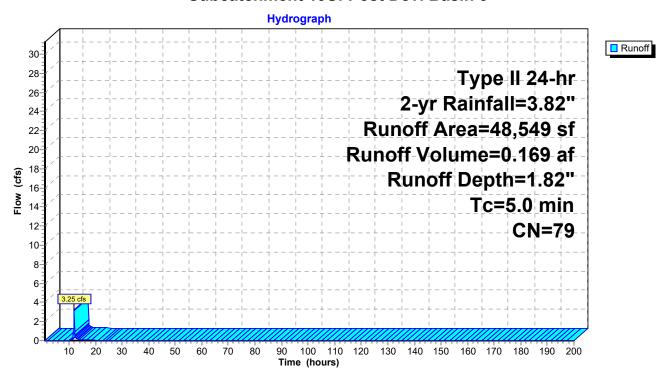
### Summary for Subcatchment 19S: Post Dev. Basin 3

Runoff = 3.25 cfs @ 11.96 hrs, Volume= 0.169 af, Depth= 1.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

A	rea (sf)	CN E	Description							
	48,549	79 5	50-75% Grass cover, Fair, HSG C							
	48,549	1	100.00% Pervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
5.0					Direct Entry,					

#### Subcatchment 19S: Post Dev. Basin 3



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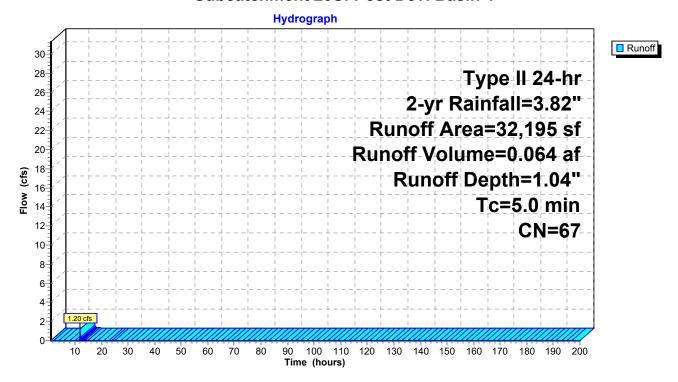
## Summary for Subcatchment 20S: Post Dev. Basin 4

Runoff = 1.20 cfs @ 11.98 hrs, Volume= 0.064 af, Depth= 1.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

Area (sf)	CN	Description			
6,966	60	Woods, Fair, HSG B			
20,201	61	>75% Grass cover, Good, HSG B			
5,028	98	Paved parking, HSG B			
32,195	67	67 Weighted Average			
27,167	•	84.38% Pervious Area			
5,028	}	15.62% Impervious Area			
Tc Lengt	h Slo <sub>l</sub>	pe Velocity	Capacity	/ Description	
(min) (feet	t) (ft/	ft) (ft/sec)	(cfs)		
5.0				Direct Entry.	

#### Subcatchment 20S: Post Dev. Basin 4



# Summary for Subcatchment 21S: Post Dev. Basin 5 to SCM

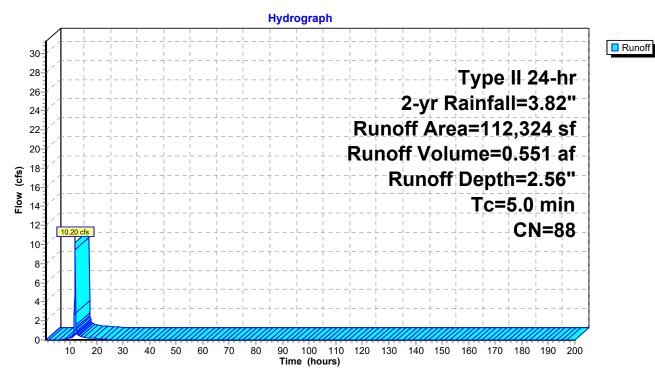
Runoff = 10.20 cfs @ 11.95 hrs, Volume= 0.551 af, Depth= 2.56"

Routed to Pond 3P: Wet Pond SCM 3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

Area (sf)	CN	Description					
40,704	98	Roofs, HSG B					
22,215	48	Brush, Good, HSG B					
44,766	98	Paved parking, HSG B					
4,639	98	Water Surface, 0% imp, HSG B					
112,324	88	Weighted Average					
26,854		23.91% Pervious Area					
85,470		76.09% Impervious Area					
Tc Length	Slo	pe Velocity Capacity Description					
(min) (feet)	(ft/	/ft) (ft/sec) (cfs)					
5.0		Direct Entry					

#### Subcatchment 21S: Post Dev. Basin 5 to SCM



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# Summary for Subcatchment 22S: Post Dev. Bypass 5

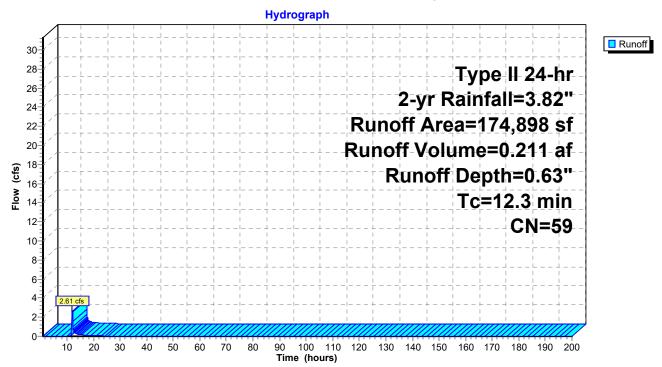
Runoff = 2.61 cfs @ 12.09 hrs, Volume= 0.211 af, Depth= 0.63"

Routed to Link 3L: POA 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

A	rea (sf)	CN	Description				
1	10,888	60	Woods, Fai	r, HSG B			
	51,602	48	Brush, Goo	d, HSG B			
	12,408	98	Paved park	ing, HSG B	3		
1	74,898	59	Weighted A	verage			
1	62,490		92.91% Per	vious Area	a a constant of the constant o		
	12,408		7.09% Impe	ervious Are	ea		
Tc	Length	Slope	e Velocity	Capacity	Description		
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)			
12.3					Direct Entry,		

# Subcatchment 22S: Post Dev. Bypass 5



# Summary for Subcatchment 23S: Post Dev. Basin 6 to SCM

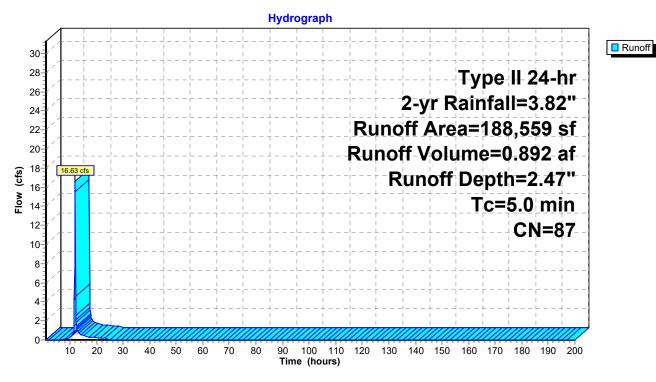
Runoff = 16.63 cfs @ 11.95 hrs, Volume= 0.892 af, Depth= 2.47"

Routed to Pond 4P: Wet Pond SCM 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

Area (st	f) CN	Description	Description					
54,46	6 98	Paved park	ing, HSG B	3				
58,38	5 61	>75% Gras	s cover, Go	ood, HSG B				
60,67	2 98	Roofs, HSG	βB					
15,03	6 98	Water Surfa	ace, 0% imp	o, HSG B				
188,55	9 87	Weighted Average						
73,42	1	38.94% Per	vious Area					
115,13	8	61.06% Imp	ervious Ar	ea				
Tc Leng	ıth Slo <sub>l</sub>	pe Velocity	Capacity	Description				
(min) (fee	et) (ft/	ft) (ft/sec)	(cfs)					
5.0				Direct Entry,				

#### Subcatchment 23S: Post Dev. Basin 6 to SCM



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# Summary for Subcatchment 24S: Post Dev. Bypass 6

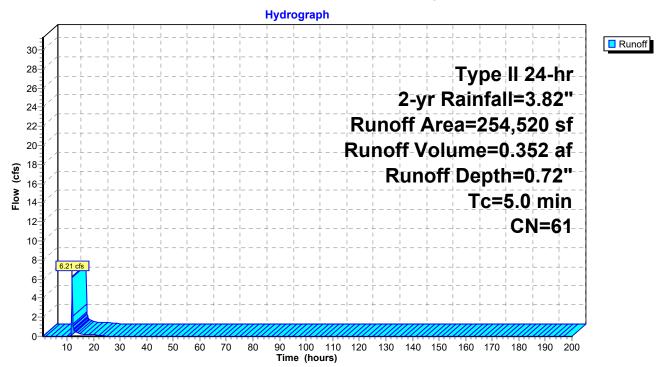
Runoff = 6.21 cfs @ 11.99 hrs, Volume= 0.352 af, Depth= 0.72"

Routed to Link 4L: POA 5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

A	rea (sf)	CN	Description					
2	15,461	60	Woods, Fai	r, HSG B				
	34,572	61	>75% Gras	s cover, Go	ood, HSG B			
	4,487	98	Paved park	ing, HSG E	3			
2	54,520	61	Weighted A	verage				
2	50,033		98.24% Per	vious Area	a			
	4,487		1.76% Impe	rvious Are	ea			
Tc	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.0					Direct Entry,			

# Subcatchment 24S: Post Dev. Bypass 6



# Summary for Subcatchment 25S: Post Dev. Basin 7 to SCM

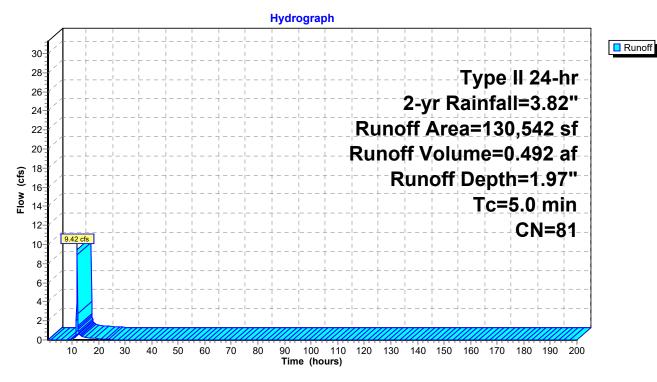
Runoff = 9.42 cfs @ 11.96 hrs, Volume= 0.492 af, Depth= 1.97"

Routed to Pond 5P: Wet Pond SCM 5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

Area (sf)	CN	Description					
50,624	98	Paved parki	ng, HSG B	3			
44,621	48	Brush, Good	d, HSG B				
28,800	98	Roofs, HSG	В				
6,497	98	Water Surfa	ce, 0% imp	p, HSG B			
130,542	81	Weighted Average					
51,118		39.16% Per	vious Area	a			
79,424		60.84% Imp	ervious Ar	rea			
Tc Length	Slop	oe Velocity	Capacity	Description			
(min) (feet)	(ft/	ft) (ft/sec)	(cfs)				
5.0				Direct Entry			

#### Subcatchment 25S: Post Dev. Basin 7 to SCM



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# Summary for Subcatchment 26S: Post Dev. Bypass 7

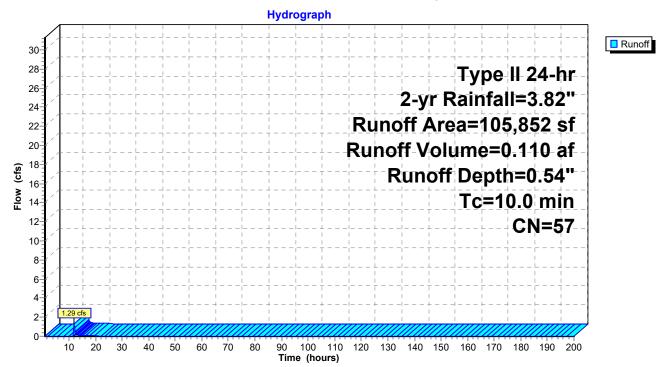
Runoff = 1.29 cfs @ 12.05 hrs, Volume= 0.110 af, Depth= 0.54"

Routed to Link 5L: POA 6

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

A	rea (sf)	CN I	Description				
	39,944	55 \	Noods, Go	od, HSG B			
	52,486	48 I	Brush, Goo	d, HSG B			
	13,422	98 I	Paved park	ing, HSG B	3		
1	05,852	57 \	Weighted Average				
	92,430	8	37.32% Per	rvious Area	a		
	13,422	•	12.68% Imp	pervious Ar	rea		
_				_			
Tc	Length	Slope	•	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
10.0					Direct Entry,		

# Subcatchment 26S: Post Dev. Bypass 7



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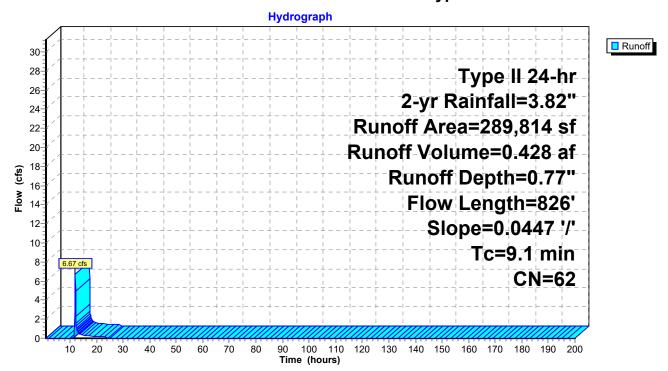
### Summary for Subcatchment 27S: Post Dev. Bypass 8

Runoff = 6.67 cfs @ 12.02 hrs, Volume= 0.428 af, Depth= 0.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 2-yr Rainfall=3.82"

A	rea (sf)	CN [	Description					
2	230,959	60 V	Voods, Fai	r, HSG B				
	53,439	69 5	50-75% Gra	ass cover, I	Fair, HSG B			
	5,416	98 F	Paved park	ing, HSG E	3			
	289,814	62 V	Veighted A	verage				
2	284,398	ç	8.13% Per	vious Area	a			
	5,416	1	.87% Impe	ervious Are	ea			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
9.1	826	0.0447	1.51		Kirpich Method,			
					General overland flow k= 2.00			

#### Subcatchment 27S: Post Dev. Bypass 8



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### Summary for Pond 1P: Sand Filter -SCM 1

Inflow Area = 7.523 ac, 77.83% Impervious, Inflow Depth = 2.65" for 2-yr event

Inflow = 30.61 cfs @ 11.95 hrs, Volume= 1.664 af

Outflow = 3.57 cfs @ 12.35 hrs, Volume= 1.664 af, Atten= 88%, Lag= 23.8 min

Primary = 3.57 cfs @ 12.35 hrs, Volume= 1.664 af

Routed to Link 1L: POA 1

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Peak Elev= 528.85' @ 12.35 hrs Surf.Area= 6,825 sf Storage= 33,134 cf

Plug-Flow detention time= 123.2 min calculated for 1.664 af (100% of inflow)

Center-of-Mass det. time= 121.5 min ( 921.6 - 800.2 )

Volume	Inve	rt Avail.Sto	rage Storage	Description	
#1	524.0	0' 68,25	50 cf Custom	n Stage Data (Pi	rismatic)Listed below (Recalc) x 65
Elevatio (fee	t)	Surf.Area (sq-ft) 105	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
534.0	-	105	1,050	1,050	
Device	Routing	Invert	Outlet Device	es	
#1	Primary	524.00'	Inlet / Outlet	P, square edge l Invert= 524.00' /	neadwall, Ke= 0.500 523.00' S= 0.0118 '/' Cc= 0.900 ght & clean, Flow Area= 7.07 sf
#2	Device 1	524.00'	8.0" Vert. Dr	awdown C= 0.	600 Limited to weir flow at low heads
#3	Device 1	528.90'		<b>0" H Vert. Main</b> eir flow at low hea	Orifice C= 0.600
#4	Device 1	533.00'		verflow 2 End 0	

Primary OutFlow Max=3.57 cfs @ 12.35 hrs HW=528.85' (Free Discharge)

-1=Culvert (Passes 3.57 cfs of 62.30 cfs potential flow)

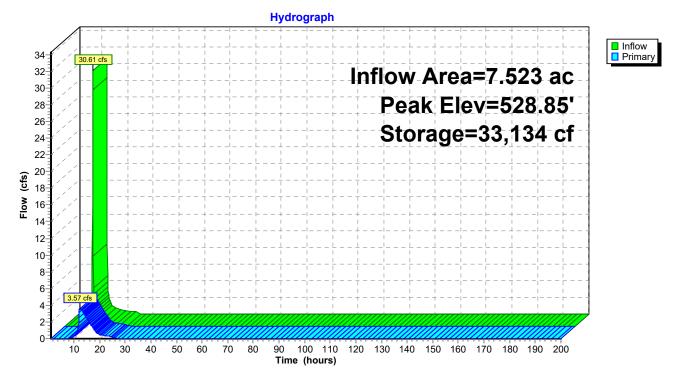
**2=Drawdown** (Orifice Controls 3.57 cfs @ 10.23 fps)

—3=Main Orifice (Controls 0.00 cfs)

**-4=Overflow** ( Controls 0.00 cfs)

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Pond 1P: Sand Filter -SCM 1



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### **Summary for Pond 2P: Wet Pond SCM 2**

Inflow Area = 3.812 ac, 71.42% Impervious, Inflow Depth = 2.38" for 2-yr event

Inflow = 14.21 cfs @ 11.96 hrs, Volume= 0.757 af

Outflow = 1.17 cfs @ 12.57 hrs, Volume= 0.757 af, Atten= 92%, Lag= 36.6 min

Primary = 1.17 cfs @ 12.57 hrs, Volume= 0.757 af

Routed to Link 2L: POA 2

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

Starting Elev= 526.00' Surf.Area= 11,086 sf Storage= 32,033 cf

Peak Elev= 527.38' @ 12.57 hrs Surf.Area= 13,787 sf Storage= 49,143 cf (17,111 cf above start)

Plug-Flow detention time= 2,092.3 min calculated for 0.022 af (3% of inflow)

Center-of-Mass det. time= 242.3 min (1,053.4 - 811.1)

Volume	Inve	rt Avail.Sto	rage S	orage	Description	
#1	522.0	0' 92,4	29 cf <b>C</b>	ustom	Stage Data (P	rismatic)Listed below (Recalc)
Clayatia	.n. (	Curf Araa	In a Ct	oro	Cum Store	
Elevatio		Surf.Area	Inc.St		Cum.Store	
(fee		(sq-ft)	(cubic-fe		(cubic-feet)	
522.0		5,067		0	0	
523.0		7,010		)39	6,039	
524.0	0	7,727	7,3	369	13,407	
525.0	0	9,219	8,4	173	21,880	
526.0	0	11,086		153	32,033	
527.0	0	13,027		)57	44,089	
528.0	0	15,043		)35	58,124	
529.0	0	17,134		089	74,213	
530.0	0	19,299	18,2	217	92,429	
Device	Routing	Invert	Outlet I	Device	s	
#1	Primary	526.00'	36.0"	Round	l Culvert	
	,		L= 45.0	' RCI	P. square edge	headwall, Ke= 0.500
						525.55' S= 0.0100 '/' Cc= 0.900
						ight & clean, Flow Area= 7.07 sf
#2	Device 1	526.00'				600 Limited to weir flow at low heads
#3	Device 1	527.40'		-		flow Orifice X 2.00 C= 0.600
., 0	2200	321.10			ir flow at low hea	
#4	Device 1	528.60'			Horiz. Overflo	
	20.100	320.00			ir flow at low hea	

Primary OutFlow Max=1.17 cfs @ 12.57 hrs HW=527.38' (Free Discharge)

-1=Culvert (Passes 1.17 cfs of 11.56 cfs potential flow)

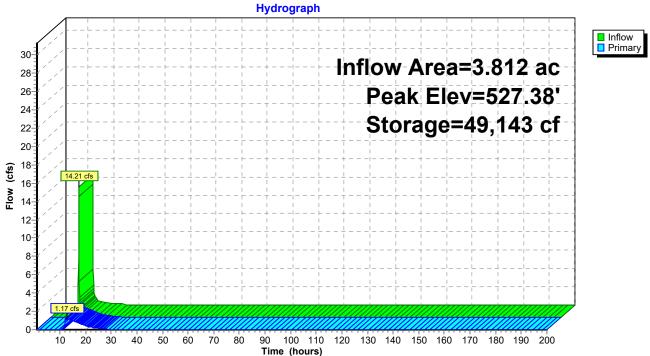
**2=Drawdown** (Orifice Controls 1.17 cfs @ 5.06 fps)

-3=Peakflow Orifice (Controls 0.00 cfs)

**-4=Overflow** (Controls 0.00 cfs)

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#### Pond 2P: Wet Pond SCM 2





Volume

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## **Summary for Pond 3P: Wet Pond SCM 3**

Inflow Area = 2.579 ac, 76.09% Impervious, Inflow Depth = 2.56" for 2-yr event

Inflow = 10.20 cfs @ 11.95 hrs, Volume= 0.551 af

Outflow = 0.72 cfs @ 12.68 hrs, Volume= 0.551 af, Atten= 93%, Lag= 43.8 min

Primary = 0.72 cfs @ 12.68 hrs, Volume= 0.551 af

Routed to Link 3L: POA 4

Invert

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

Starting Elev= 534.00' Surf.Area= 3,130 sf Storage= 15,183 cf

Peak Elev= 536.00' @ 12.68 hrs Surf.Area= 8,624 sf Storage= 27,369 cf (12,186 cf above start)

Plug-Flow detention time= 582.5 min calculated for 0.202 af (37% of inflow)

Avail Storage Storage Description

Center-of-Mass det. time= 202.2 min ( 1,006.2 - 804.0 )

volume	IIIVE	arı Avalı.Sıd	rage Storag	ge Description
#1	530.0	00' 52,8	62 cf Custo	om Stage Data (Prismatic)Listed below (Recalc)
Elevatio		Surf.Area	Inc.Store	Cum.Store
(fee	÷()	(sq-ft)	(cubic-feet)	(cubic-feet)
530.0	00	2,212	0	0
531.0	00	3,200	2,706	2,706
532.0	00	4,144	3,672	6,378
533.0	00	5,168	4,656	11,034
534.0	00	3,130	4,149	15,183
535.0	00	6,262	4,696	19,879
536.0	00	8,640	7,451	27,330
537.0	00	4,960	6,800	34,130
538.0	00	9,920	7,440	41,570
539.0	00	12,664	11,292	52,862
Device	Routing	Invert	Outlet Devi	ces
#1	Primary	534.00'	30.0" Rou	nd Culvert
	,		L= 44.0' R	CP, square edge headwall, Ke= 0.500
				et Invert= 534.00' / 533.56' S= 0.0100 '/' Cc= 0.900
			n= 0.013 C	Concrete pipe, bends & connections, Flow Area= 4.91 sf
#2	Device 1	534.00'		<b>Drawdown</b> C= 0.600 Limited to weir flow at low heads
#3	Device 1	536.30'	42.0" W x 3	3.0" H Vert. Peak Flow X 2.00 C= 0.600
			Limited to w	veir flow at low heads
#4	Device 1	537.60'	48.0" x 48.	0" Horiz. Overflow C= 0.600
			Limited to w	veir flow at low heads

Primary OutFlow Max=0.72 cfs @ 12.68 hrs HW=536.00' (Free Discharge)

**-1=Culvert** (Passes 0.72 cfs of 17.63 cfs potential flow)

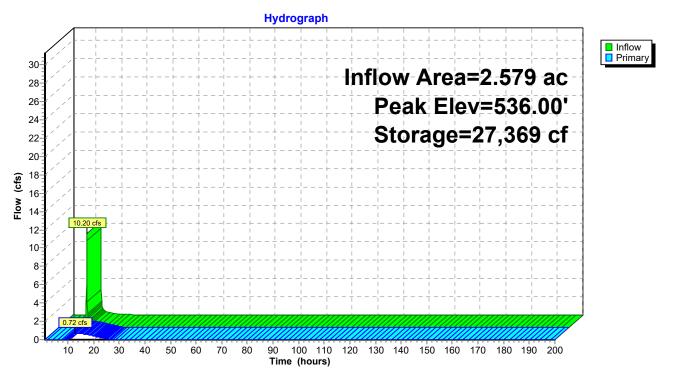
**2=Drawdown** (Orifice Controls 0.72 cfs @ 6.49 fps)

-3=Peak Flow (Controls 0.00 cfs)

**-4=Overflow** (Controls 0.00 cfs)

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Pond 3P: Wet Pond SCM 3



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### Summary for Pond 4P: Wet Pond SCM 4

Inflow Area = 4.329 ac, 61.06% Impervious, Inflow Depth = 2.47" for 2-yr event

Inflow = 16.63 cfs @ 11.95 hrs, Volume= 0.892 af

Outflow = 0.23 cfs @ 19.20 hrs, Volume= 0.567 af, Atten= 99%, Lag= 435.0 min

Primary = 0.23 cfs @ 19.20 hrs, Volume= 0.567 af

Routed to Link 4L: POA 5

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Peak Elev= 525.04' @ 19.20 hrs Surf.Area= 9,556 sf Storage= 29,958 cf

Plug-Flow detention time= 1,078.5 min calculated for 0.567 af (64% of inflow)

Center-of-Mass det. time= 976.0 min (1,783.6 - 807.6)

Volume	Invert	Avail.St	torage	Storage	Description	
#1	519.00'	67,	235 cf	Custom	n Stage Data (Pr	rismatic)Listed below (Recalc)
Floretion	Cum	f Araa	lna	Ctoro	Cum Store	
Elevation	Sui	f.Area		.Store	Cum.Store	
(feet)		(sq-ft)	(cubic	c-feet)	(cubic-feet)	
519.00		1,472		0	0	
520.00		2,352		1,912	1,912	
521.00		3,406		2,879	4,791	
522.00		4,636		4,021	8,812	
523.00		6,046		5,341	14,153	
524.00		7,648		6,847	21,000	
525.00		9,474		8,561	29,561	
526.00	1	1,446	1	0,460	40,021	
527.00	1	3,570	1	2,508	52,529	
528.00	1	5,842	1	4,706	67,235	
Davisa D	45		4 0 4	at Davida		

Device	Routing	Invert	Outlet Devices
#1	Primary	523.00'	36.0" Round Culvert
	•		L= 45.0' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 523.00' / 521.50' S= 0.0333 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 7.07 sf
#2	Device 1	523.00'	<b>2.5" Vert. Drawdown</b> C= 0.600 Limited to weir flow at low heads
#3	Device 1	525.10'	<b>36.0" W x 3.0" H Vert. Peakflow</b> C= 0.600
			Limited to weir flow at low heads
#4	Device 1	526.50'	<b>48.0" x 48.0" Horiz. Overflow</b> C= 0.600
			I imited to weir flow at low heads

Primary OutFlow Max=0.23 cfs @ 19.20 hrs HW=525.04' (Free Discharge)

-1=Culvert (Passes 0.23 cfs of 24.92 cfs potential flow)

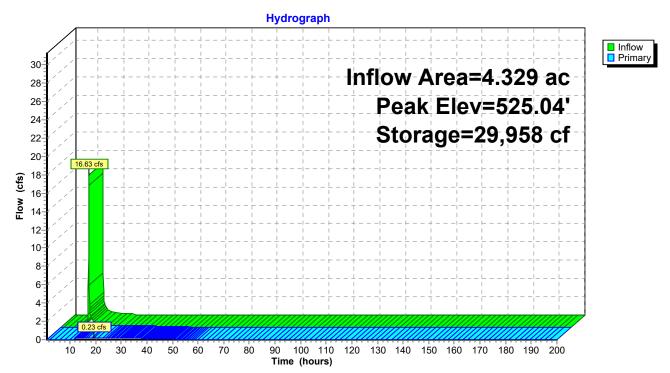
**2=Drawdown** (Orifice Controls 0.23 cfs @ 6.70 fps)

-3=Peakflow (Controls 0.00 cfs)

-4=Overflow (Controls 0.00 cfs)

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Pond 4P: Wet Pond SCM 4



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# **Summary for Pond 5P: Wet Pond SCM 5**

Inflow Area = 2.997 ac, 60.84% Impervious, Inflow Depth = 1.97" for 2-yr event

Inflow = 9.42 cfs @ 11.96 hrs, Volume= 0.492 af

Outflow = 0.57 cfs @ 13.01 hrs, Volume= 0.492 af, Atten= 94%, Lag= 63.1 min

Primary = 0.57 cfs @ 13.01 hrs, Volume= 0.492 af

Routed to Link 5L: POA 6

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

Starting Elev= 513.00' Surf.Area= 4,340 sf Storage= 13,131 cf

Peak Elev= 515.00' @ 13.01 hrs Surf.Area= 6,271 sf Storage= 24,189 cf (11,059 cf above start)

Plug-Flow detention time= 670.1 min calculated for 0.191 af (39% of inflow)

Center-of-Mass det. time= 257.6 min (1,084.5 - 826.9)

Volume	Inve	rt Avail.Sto	rage Storag	je Description				
#1	509.0	0' 45,4	75 cf Custo	m Stage Data (P	rismatic)Listed below (Recalc)			
Elevation	an s	Surf.Area	Inc.Store	Cum.Store				
(fee		(sq-ft)	(cubic-feet)	(cubic-feet)				
509.0		2,355	0	0				
510.0		2,789	2,572	2,572				
511.0		3,517	3,153	5,725				
512.0	00	3,477	3,497	9,222				
513.0		4,340	3,909	13,131				
514.0		5,752	5,046	18,177				
515.0		6,271	6,012	24,188				
516.0		6,813	6,542	30,730				
517.0		7,377	7,095	37,825				
518.0	00	7,922	7,650	45,475				
Device	Routing	Invert	Outlet Device	ces				
#1	Primary	513.00'	36.0" Rour	nd Culvert				
	•		L= 79.0' R	CP, square edge	headwall, Ke= 0.500			
			Inlet / Outlet	t Invert= 513.00' /	512.00' S= 0.0127 '/' Cc= 0.900			
			n= 0.013 C	n= 0.013 Concrete pipe, bends & connections, Flow Area= 7.07 sf				
#2	Device 1	513.00'	4.0" Vert. D	rawdown C= 0.	.600 Limited to weir flow at low heads			
#3	Device 1	515.10'	38.0" W x 3	3.0" H Vert. Peak	flow Orifice X 2.00 C= 0.600			
				eir flow at low hea				
#4	Device 1	516.70'	48.0" x 48.0	" Horiz. Overflo	<b>w</b> C= 0.600			
			Limited to w	eir flow at low hea	ads			

Primary OutFlow Max=0.57 cfs @ 13.01 hrs HW=515.00' (Free Discharge)

**-1=Culvert** (Passes 0.57 cfs of 24.10 cfs potential flow)

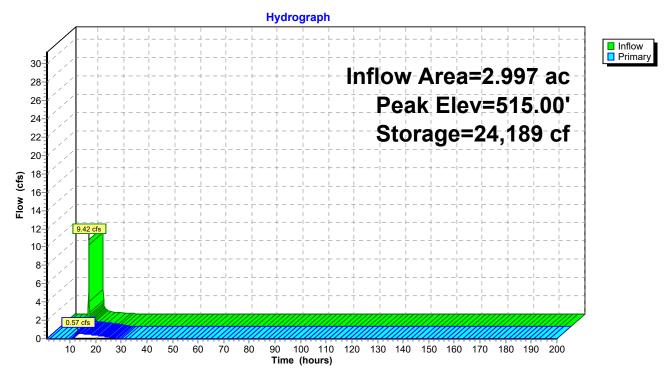
**2=Drawdown** (Orifice Controls 0.57 cfs @ 6.52 fps)

-3=Peakflow Orifice (Controls 0.00 cfs)

-4=Overflow (Controls 0.00 cfs)

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### Pond 5P: Wet Pond SCM 5



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# Summary for Link 1L: POA 1

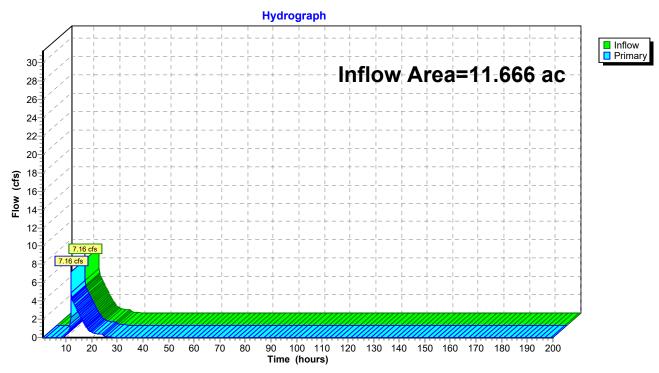
Inflow Area = 11.666 ac, 51.60% Impervious, Inflow Depth = 1.95" for 2-yr event

Inflow = 7.16 cfs @ 12.00 hrs, Volume= 1.897 af

Primary = 7.16 cfs @ 12.00 hrs, Volume= 1.897 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

### Link 1L: POA 1



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# **Summary for Link 2L: POA 2**

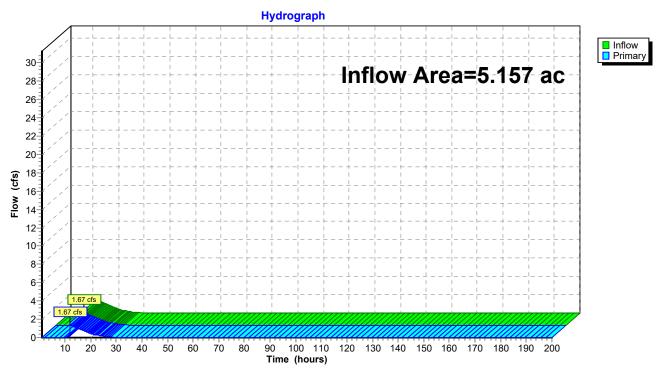
Inflow Area = 5.157 ac, 52.79% Impervious, Inflow Depth = 1.87" for 2-yr event

Inflow = 1.67 cfs @ 12.02 hrs, Volume= 0.804 af

Primary = 1.67 cfs @ 12.02 hrs, Volume= 0.804 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

## Link 2L: POA 2



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# **Summary for Link 3L: POA 4**

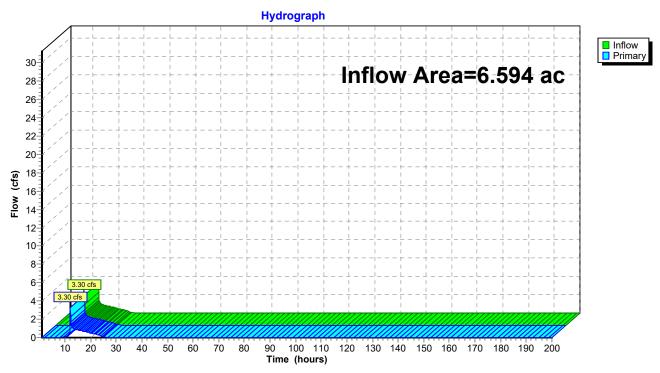
Inflow Area = 6.594 ac, 34.08% Impervious, Inflow Depth = 1.39" for 2-yr event

Inflow = 3.30 cfs @ 12.09 hrs, Volume= 0.761 af

Primary = 3.30 cfs @ 12.09 hrs, Volume= 0.761 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

## Link 3L: POA 4



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# Summary for Link 4L: POA 5

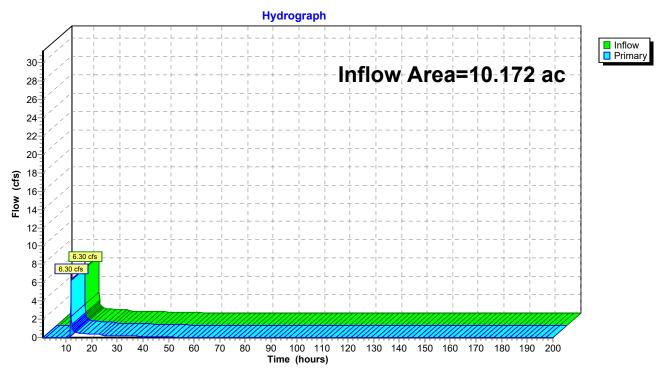
Inflow Area = 10.172 ac, 27.00% Impervious, Inflow Depth = 1.08" for 2-yr event

Inflow = 6.30 cfs @ 11.99 hrs, Volume= 0.919 af

Primary = 6.30 cfs @ 11.99 hrs, Volume= 0.919 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

### Link 4L: POA 5



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# Summary for Link 5L: POA 6

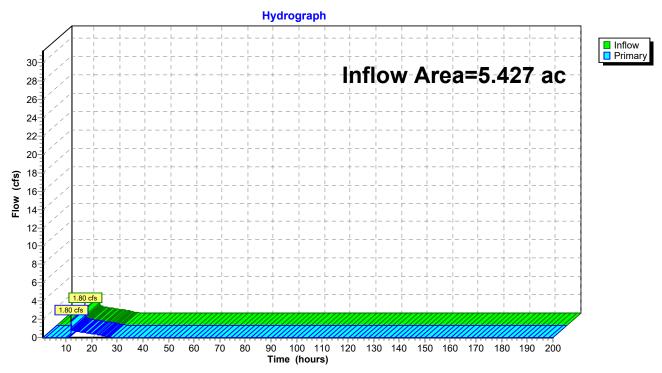
Inflow Area = 5.427 ac, 39.28% Impervious, Inflow Depth = 1.33" for 2-yr event

Inflow = 1.80 cfs @ 12.06 hrs, Volume= 0.602 af

Primary = 1.80 cfs @ 12.06 hrs, Volume= 0.602 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

### Link 5L: POA 6



Tc=5.0 min CN=54 Runoff=3.68 cfs 0.196 af

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Time span=1.00-200.00 hrs, dt=0.10 hrs, 1991 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

	off Area=366,801 sf 0.00% Impervious Runoff Depth=4.16" =0.0630 '/' Tc=5.9 min CN=79 Runoff=55.13 cfs 2.917 af
	off Area=250,337 sf 0.00% Impervious Runoff Depth=4.16" =0.0650 '/' Tc=7.7 min CN=79 Runoff=37.06 cfs 1.991 af
	off Area=132,113 sf 0.00% Impervious Runoff Depth=2.28" =0.0451 '/' Tc=6.9 min CN=60 Runoff=11.03 cfs 0.575 af
	Area=467,738 sf 32.11% Impervious Runoff Depth=4.91" =0.0633 '/' Tc=7.4 min CN=86 Runoff=79.55 cfs 4.398 af
	off Area=52,001 sf 0.00% Impervious Runoff Depth=4.05" e=0.0170 '/' Tc=5.1 min CN=78 Runoff=7.58 cfs 0.403 af
	off Area=130,388 sf 0.00% Impervious Runoff Depth=2.19" a=0.0221 '/' Tc=8.9 min CN=59 Runoff=9.71 cfs 0.545 af
	off Area=305,128 sf 0.00% Impervious Runoff Depth=2.19" 0.0301 '/' Tc=12.3 min CN=59 Runoff=19.31 cfs 1.276 af
	off Area=441,055 sf 0.00% Impervious Runoff Depth=2.28" 0.0441 '/' Tc=12.4 min CN=60 Runoff=29.18 cfs 1.921 af
	off Area=238,293 sf 0.00% Impervious Runoff Depth=2.01" 0.0468 '/' Tc=10.2 min CN=57 Runoff=15.08 cfs 0.915 af
	off Area=289,814 sf 1.87% Impervious Runoff Depth=2.46" =0.0447 '/' Tc=9.1 min CN=62 Runoff=24.39 cfs 1.364 af
Subcatchment12S: Post Basin 1 to SCM Runoff	F Area=327,690 sf 77.83% Impervious Runoff Depth=5.25" Tc=5.0 min CN=89 Runoff=56.50 cfs 3.291 af
Subcatchment13S: Post Dev Bypass 1 Runo	off Area=180,474 sf 3.97% Impervious Runoff Depth=2.28" Tc=5.0 min CN=60 Runoff=15.11 cfs 0.786 af
	off Area=120,581 sf 0.00% Impervious Runoff Depth=4.16" =0.0411 '/' Tc=5.6 min CN=79 Runoff=18.08 cfs 0.959 af
Subcatchment15S: Post Dev. Basin 2B Runoff	F Area=166,052 sf 71.42% Impervious Runoff Depth=4.91" Tc=5.0 min CN=86 Runoff=28.14 cfs 1.561 af
	Area=460,928 sf 34.20% Impervious Runoff Depth=4.91" =0.0633 '/' Tc=7.4 min CN=86 Runoff=78.39 cfs 4.334 af
Subcatchment18S: Post Dev Bypass 2B Run	off Area=58,575 sf 0.00% Impervious Runoff Depth=1.75"

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Runoff Area=48,549 sf 0.00% Impervious Runoff Depth=4.16" Subcatchment19S: Post Dev. Basin 3 Tc=5.0 min CN=79 Runoff=7.22 cfs 0.386 af Subcatchment20S: Post Dev. Basin 4 Runoff Area=32,195 sf 15.62% Impervious Runoff Depth=2.94" Tc=5.0 min CN=67 Runoff=3.48 cfs 0.181 af Subcatchment21S: Post Dev. Basin 5 to Runoff Area=112,324 sf 76.09% Impervious Runoff Depth=5.14" Tc=5.0 min CN=88 Runoff=19.09 cfs 1.104 af Subcatchment22S: Post Dev. Bypass 5 Runoff Area=174,898 sf 7.09% Impervious Runoff Depth=2.19" Tc=12.3 min CN=59 Runoff=11.07 cfs 0.731 af Subcatchment23S: Post Dev. Basin 6 to Runoff Area=188,559 sf 61.06% Impervious Runoff Depth=5.03" Tc=5.0 min CN=87 Runoff=31.55 cfs 1.813 af Runoff Area=254,520 sf 1.76% Impervious Runoff Depth=2.37" Subcatchment24S: Post Dev. Bypass 6 Tc=5.0 min CN=61 Runoff=22.20 cfs 1.153 af Subcatchment25S: Post Dev. Basin 7 to Runoff Area=130,542 sf 60.84% Impervious Runoff Depth=4.37" Tc=5.0 min CN=81 Runoff=20.23 cfs 1.091 af Runoff Area=105,852 sf 12.68% Impervious Runoff Depth=2.01" Subcatchment26S: Post Dev. Bypass 7 Tc=10.0 min CN=57 Runoff=6.77 cfs 0.406 af Runoff Area=289.814 sf 1.87% Impervious Runoff Depth=2.46" Subcatchment27S: Post Dev. Bypass 8 Flow Length=826' Slope=0.0447 '/' Tc=9.1 min CN=62 Runoff=24.39 cfs 1.364 af Peak Elev=531.55' Storage=51,523 cf Inflow=56.50 cfs 3.291 af Pond 1P: Sand Filter -SCM 1 Outflow=28.94 cfs 3.291 af Pond 2P: Wet Pond SCM 2 Peak Elev=528.36' Storage=63,678 cf Inflow=28.14 cfs 1.561 af Outflow=5.86 cfs 1.561 af Pond 3P: Wet Pond SCM 3 Peak Elev=537.18' Storage=35,089 cf Inflow=19.09 cfs 1.104 af Outflow=8.22 cfs 1.104 af Pond 4P: Wet Pond SCM 4 Peak Elev=526.41' Storage=44,889 cf Inflow=31.55 cfs 1.813 af Outflow=4.23 cfs 1.488 af Pond 5P: Wet Pond SCM 5 Peak Elev=516.22' Storage=32,230 cf Inflow=20.23 cfs 1.091 af Outflow=8.33 cfs 1.091 af Link 1L: POA 1 Inflow=42.15 cfs 4.077 af Primary=42.15 cfs 4.077 af Link 2L: POA 2 Inflow=8.72 cfs 1.757 af Primary=8.72 cfs 1.757 af Link 3L: POA 4 Inflow=19.04 cfs 1.835 af Primary=19.04 cfs 1.835 af **32044.0000 - CZ**Prepared by Thomas & Hutton

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**Link 4L: POA 5**Inflow=24.77 cfs 2.641 af
Primary=24.77 cfs 2.641 af

**Link 5L: POA 6**Inflow=15.03 cfs 1.498 af
Primary=15.03 cfs 1.498 af

Total Runoff Area = 122.250 ac Runoff Volume = 35.661 af Average Runoff Depth = 3.50" 80.94% Pervious = 98.954 ac 19.06% Impervious = 23.297 ac

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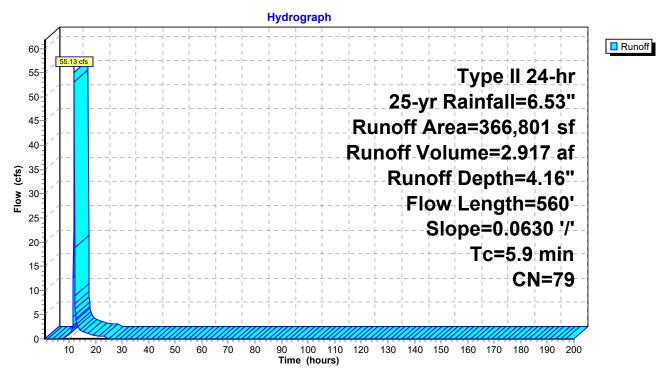
# Summary for Subcatchment 1S: Pre Dev. Basin 1

Runoff = 55.13 cfs @ 11.97 hrs, Volume= 2.917 af, Depth= 4.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

A	rea (sf)	CN I	Description					
3	42,042	79 \	Noods, Fai	r, HSG D				
	24,759	73 I	Brush, Good, HSG D					
3	66,801	79 \	Neighted A	verage				
3	66,801		100.00% Pe	ervious Are	a			
Tc	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.9	560	0.0630	1.58		Kirpich Method, Estimated Tc			
					General overland flow k= 2.00			

#### Subcatchment 1S: Pre Dev. Basin 1



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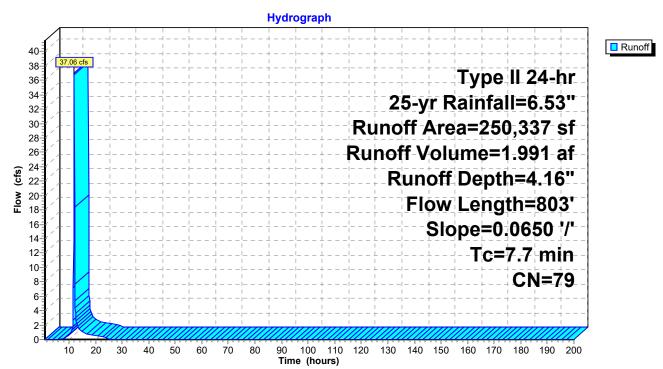
### Summary for Subcatchment 2S: Pre Dev. Basin 2A

Runoff = 37.06 cfs @ 11.99 hrs, Volume= 1.991 af, Depth= 4.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

A	rea (sf)	CN [	Description		
2	250,337	79 V	Voods, Fai	r, HSG D	
2	50,337	1	00.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.7	803	0.0650	1.74	, ,	Kirpich Method, Estimated Tc General overland flow k= 2.00

#### Subcatchment 2S: Pre Dev. Basin 2A



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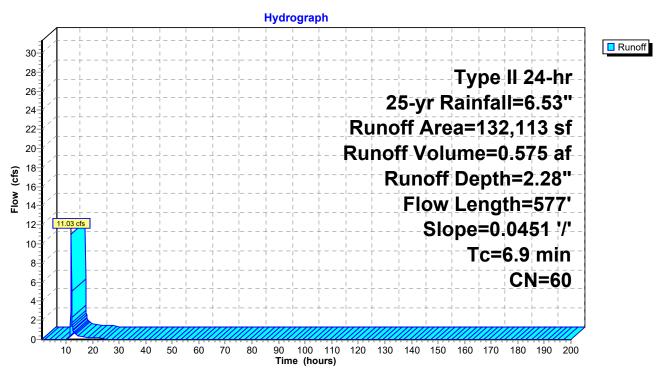
# Summary for Subcatchment 3S: Pre Dev. Basin 2B

Runoff = 11.03 cfs @ 11.99 hrs, Volume= 0.575 af, Depth= 2.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

_	Α	rea (sf)	CN [	Description		
	1	32,113	60 V	Voods, Fai	r, HSG B	
	1	32,113	1	100.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	6.9	577	0.0451	1.40		Kirpich Method, Estimated Tc General overland flow k= 2.00

#### Subcatchment 3S: Pre Dev. Basin 2B



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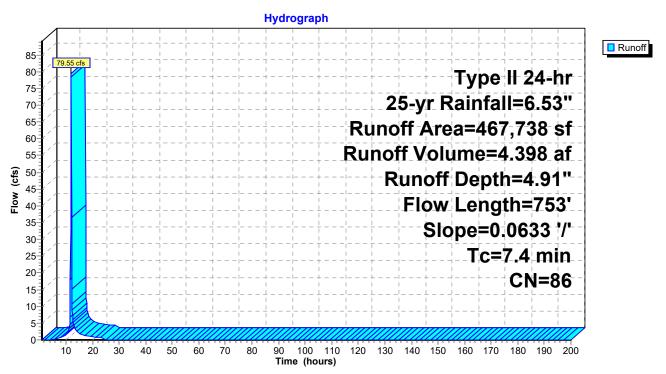
# Summary for Subcatchment 4S: Pre Dev. Basin 2C

Runoff = 79.55 cfs @ 11.98 hrs, Volume= 4.398 af, Depth= 4.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

Area	(sf)	CN D	escription					
173,	689	79 V	/oods, Fai	r, HSG D				
94,	275	98 P	aved park	ing, HSG D				
143,	869	82 V	/oods/gras	s comb., F	air, HSG D			
55,	905							
467,	738	86 V	/eighted A	verage				
317,	558	6	67.89% Pervious Area					
150,	180	3	2.11% lmp	ervious Ar	ea			
Tc Le	ngth	Slope	Velocity	Capacity	Description			
(min) (	feet)	(ft/ft)	(ft/sec)	(cfs)				
7.4	753	0.0633	1.69		Kirpich Method, Estimated Tc			
					General overland flow k= 2.00			

#### Subcatchment 4S: Pre Dev. Basin 2C



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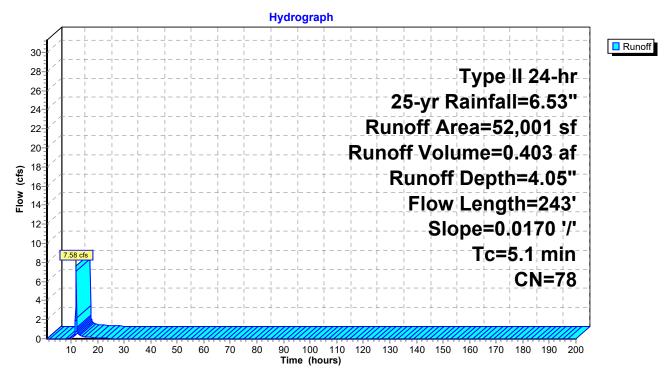
### Summary for Subcatchment 6S: Pre Dev. Basin 3

Runoff = 7.58 cfs @ 11.96 hrs, Volume= 0.403 af, Depth= 4.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

A	rea (sf)	CN	Description					
	9,925	73	Woods, Fair, HSG C					
	42,076	79	50-75% Gra	ass cover, F	Fair, HSG C			
	52,001	78	8 Weighted Average					
	52,001		100.00% P	ervious Are	a			
Tc	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.1	243	0.0170	0.79		Kirpich Method, Estimated Tc			
					General overland flow k= 2.00			

#### Subcatchment 6S: Pre Dev. Basin 3



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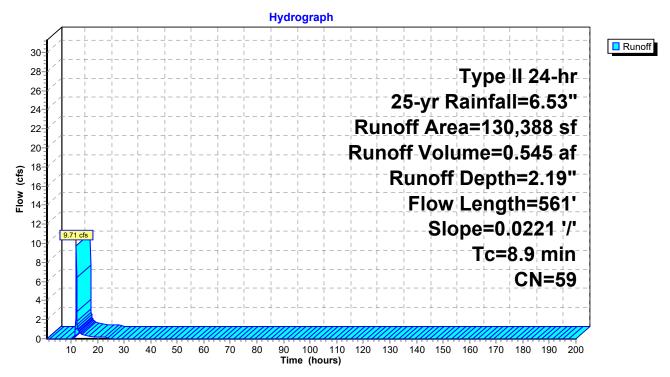
# Summary for Subcatchment 7S: Pre Dev. Basin 4

Runoff = 9.71 cfs @ 12.01 hrs, Volume= 0.545 af, Depth= 2.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

 Α	rea (sf)	CN	Description					
1	04,102	60	Woods, Fai	r, HSG B				
	26,286	56	Brush, Fair, HSG B					
1	30,388	59	Weighted A	verage				
1	30,388		100.00% P	ervious Are	a			
Тс	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
8.9	561	0.0221	1.06		Kirpich Method,			
					General overland flow k= 2.00			

#### Subcatchment 7S: Pre Dev. Basin 4



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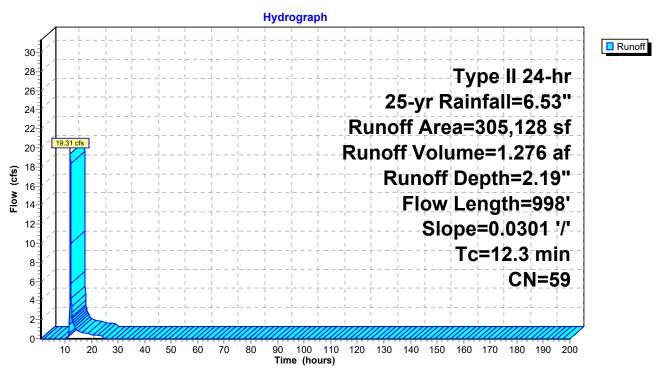
# Summary for Subcatchment 8S: Pre Dev. Basin 5

Runoff = 19.31 cfs @ 12.05 hrs, Volume= 1.276 af, Depth= 2.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

A	rea (sf)	CN	Description		
2	267,919	60	Woods, Fai	r, HSG B	
	37,209	48	Brush, Goo	d, HSG B	
3	305,128	59	Weighted A	verage	
3	305,128		100.00% Pe	ervious Are	a
Tc	Length	Slope	,	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.3	998	0.0301	1.36		Kirpich Method, Estimated Tc
					General overland flow k= 2.00

#### Subcatchment 8S: Pre Dev. Basin 5



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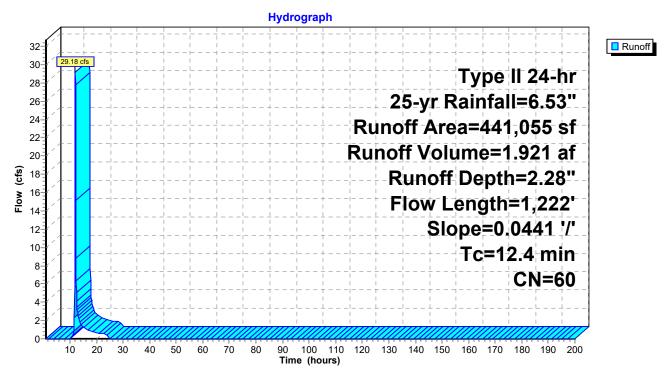
### Summary for Subcatchment 9S: Pre Dev. Basin 6

Runoff = 29.18 cfs @ 12.05 hrs, Volume= 1.921 af, Depth= 2.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

	Area (sf)	CN	Description					
	401,238	60	Woods, Fai	r, HSG B				
	39,817	56	Brush, Fair, HSG B					
	441,055	60	Weighted A	verage				
	441,055		100.00% P	ervious Are	a			
Tc	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
12.4	1,222	0.0441	1.65		Kirpich Method, Estimated Tc			
					General overland flow k= 2.00			

#### Subcatchment 9S: Pre Dev. Basin 6



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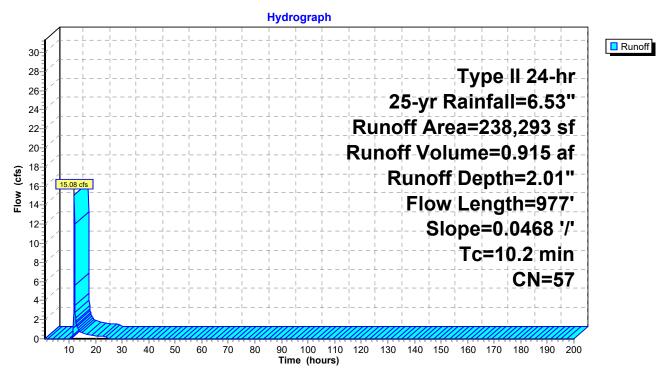
# Summary for Subcatchment 10S: Pre Dev. Basin 7

Runoff = 15.08 cfs @ 12.02 hrs, Volume= 0.915 af, Depth= 2.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

	Α	rea (sf)	CN I	Description					
	2	07,447	55 \	Woods, Good, HSG B					
		30,846	69 I	Pasture/gra	ssland/ran	ge, Fair, HSG B			
	2	38,293	57 \	Veighted A	verage				
	2	238,293 100.00% Pervious Are			ervious Are	ea			
	Tc	Length	Slope	,	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	10.2	977	0.0468	1.60		Kirpich Method, Estimated Tc			
						General overland flow k= 2.00			

#### Subcatchment 10S: Pre Dev. Basin 7



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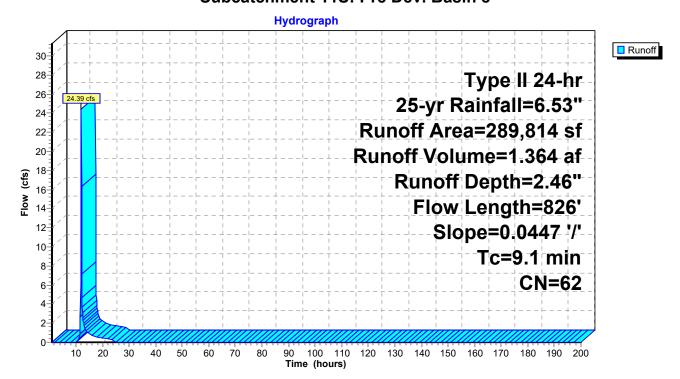
# Summary for Subcatchment 11S: Pre Dev. Basin 8

Runoff = 24.39 cfs @ 12.01 hrs, Volume= 1.364 af, Depth= 2.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

A	rea (sf)	CN [	Description					
2	30,959	60 \	Voods, Fai	r, HSG B				
	53,439	69 5	50-75% Grass cover, Fair, HSG B					
	5,416	98 F	Paved park	ing, HSG B	3			
2	89,814	62 \	Veighted A	verage				
2	284,398 98.13% Pervious Area				ì			
	5,416	•	1.87% Impe	ervious Are	a			
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
9.1	826	0.0447	1.51		Kirpich Method,			
					General overland flow k= 2.00			

#### Subcatchment 11S: Pre Dev. Basin 8



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# Summary for Subcatchment 12S: Post Basin 1 to SCM

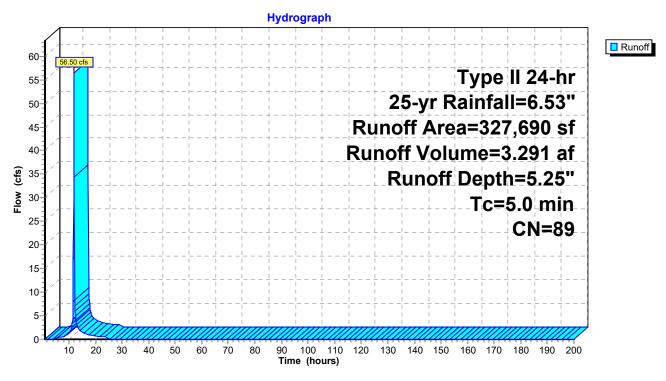
Runoff = 56.50 cfs @ 11.95 hrs, Volume= 3.291 af, Depth= 5.25"

Routed to Pond 1P: Sand Filter -SCM 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

Area	(sf) CN	N De	escription			
104	04,903 98 Paved parking, HSG A			ng, HSG A	A	
72	655 56 Brush, Fair, HSG B					
150	,132 98	98 Roofs, HSG B				
327,690 89		9 W	Weighted Average			
72	,655	22	22.17% Pervious Area			
255,035		77	77.83% Impervious Area			
	•	lope	Velocity	Capacity	Description	
(min)	(feet) (	ft/ft)	(ft/sec)	(cfs)		
5.0					Direct Entry,	

#### Subcatchment 12S: Post Basin 1 to SCM



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## Summary for Subcatchment 13S: Post Dev Bypass 1

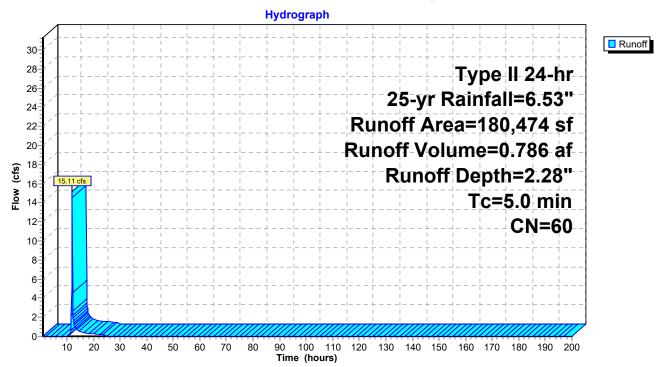
Runoff = 15.11 cfs @ 11.97 hrs, Volume= 0.786 af, Depth= 2.28"

Routed to Link 1L: POA 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

Are	ea (sf)	CN	Description				
12	4,058	60	Woods, Fai	r, HSG B			
4	9,243	56	Brush, Fair,	HSG B			
	7,173	98	Paved park	ing, HSG B	3		
18	0,474	60	Weighted A	verage			
17	3,301		96.03% Pei	vious Area	a		
•	7,173		3.97% Impe	ervious Area	ea		
	Length	Slope	•	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
5.0					Direct Entry,		

## **Subcatchment 13S: Post Dev Bypass 1**



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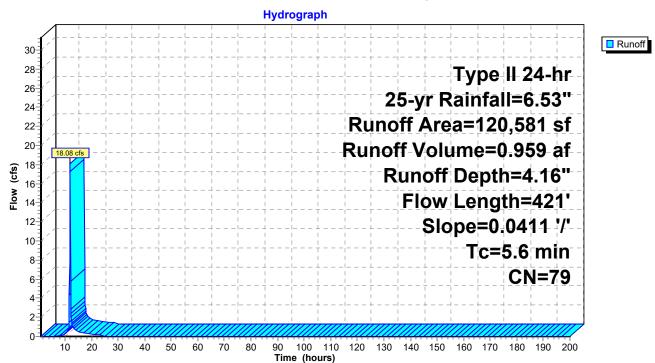
## Summary for Subcatchment 14S: Post Dev. Bypass 2A

Runoff = 18.08 cfs @ 11.96 hrs, Volume= 0.959 af, Depth= 4.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

A	rea (sf)	CN I	Description		
1	20,581	79 \	Noods, Fai	r, HSG D	
1	120,581		100.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.6	421	0.0411	1.25		Kirpich Method, Estimated Tc General overland flow k= 2.00

# Subcatchment 14S: Post Dev. Bypass 2A



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## Summary for Subcatchment 15S: Post Dev. Basin 2B to SCM

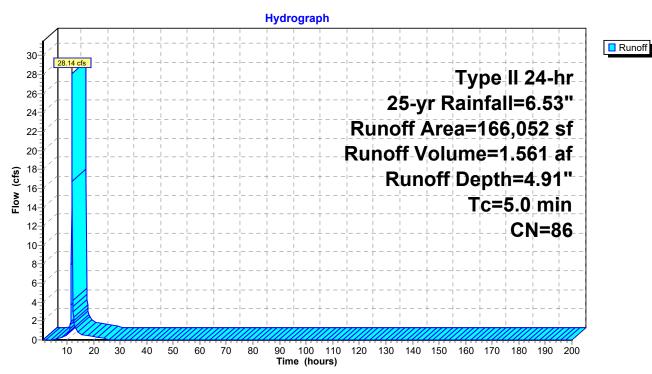
Runoff = 28.14 cfs @ 11.95 hrs, Volume= 1.561 af, Depth= 4.91"

Routed to Pond 2P: Wet Pond SCM 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

Area (sf)	CN	Description						
22,914	98	Paved parking, HSG B						
6,465	48	Brush, Good, HSG B						
95,673	98	Roofs, HSG B						
41,000	58	Woods/grass comb., Good, HSG B						
166,052	86	Weighted Average						
47,465		28.58% Pervious Area						
118,587		71.42% Impervious Area						
Tc Length	Slop	pe Velocity Capacity Description						
(min) (feet)	(ft/	(ft/sec) (cfs)						
5.0	Direct Entry.							

#### Subcatchment 15S: Post Dev. Basin 2B to SCM



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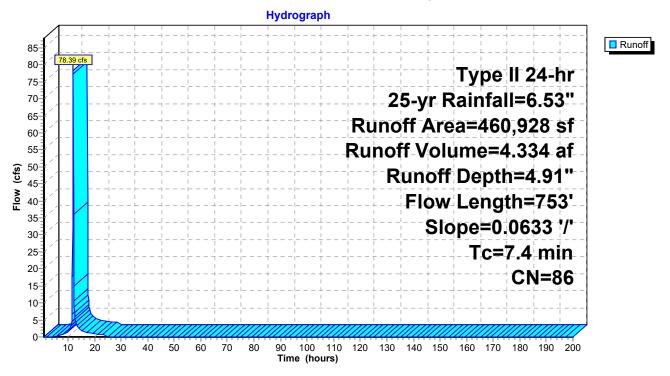
## Summary for Subcatchment 16S: Post Dev. Bypass 2C

Runoff = 78.39 cfs @ 11.98 hrs, Volume= 4.334 af, Depth= 4.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

A	rea (sf)	CN E	escription					
1	73,689	79 V	Voods, Fai	r, HSG D				
1	01,714	98 F	Paved park	ing, HSG D	)			
1	29,620	82 V	Voods/gras	s comb., F	Fair, HSG D			
	55,905	98 F	Roofs, HSC	G D				
4	60,928	86 V	Veighted A	verage				
3	303,309	6	65.80% Pervious Area					
1	157,619		4.20% Imp	ervious Ar	rea			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
7.4	753	0.0633	1.69		Kirpich Method, Estimated Tc			
					General overland flow k= 2.00			

# Subcatchment 16S: Post Dev. Bypass 2C



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## Summary for Subcatchment 18S: Post Dev Bypass 2B

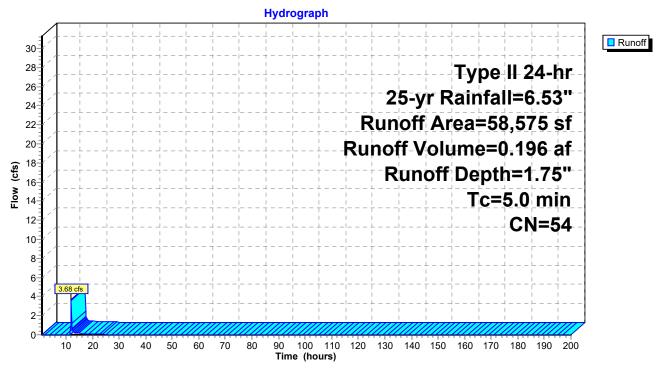
Runoff = 3.68 cfs @ 11.98 hrs, Volume= 0.196 af, Depth= 1.75"

Routed to Link 2L: POA 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

_	Α	rea (sf)	CN	Description					
_		54,125	55	Woods, Go	od, HSG B				
_		4,450	48	Brush, Goo	d, HSG B				
_	58,575 54 Weighted Average				verage				
		58,575		100.00% Pe	ervious Are	ea			
	Tc	Length	Slope	e Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	) (ft/sec)	(cfs)				
	5.0					Direct Entry			

# Subcatchment 18S: Post Dev Bypass 2B



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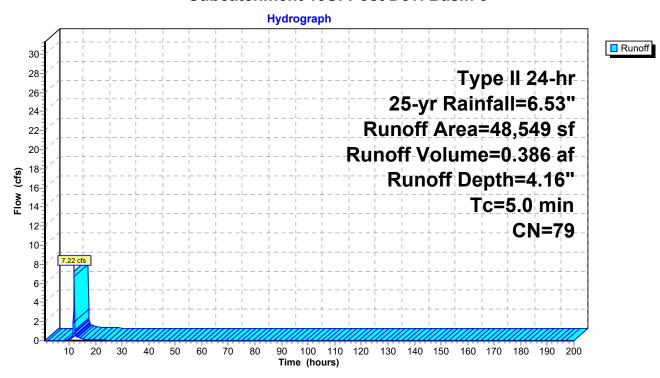
## Summary for Subcatchment 19S: Post Dev. Basin 3

Runoff = 7.22 cfs @ 11.96 hrs, Volume= 0.386 af, Depth= 4.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

 Α	rea (sf)	CN	Description						
	48,549	79	50-75% Grass cover, Fair, HSG C						
	48,549	100.00% Pervious Area							
 Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry,				

#### Subcatchment 19S: Post Dev. Basin 3



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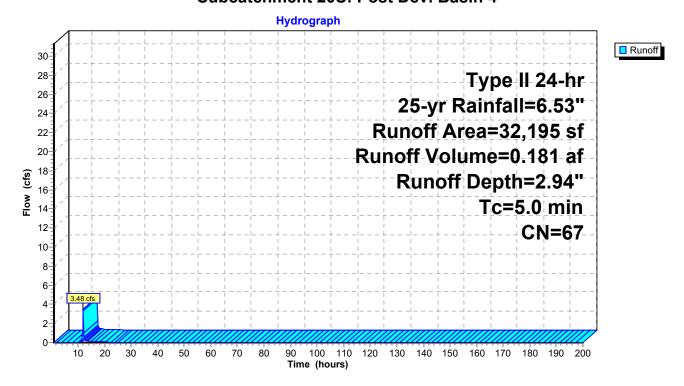
## Summary for Subcatchment 20S: Post Dev. Basin 4

Runoff = 3.48 cfs @ 11.96 hrs, Volume= 0.181 af, Depth= 2.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

Area (sf)	) CN	CN Description					
6,966	60	Woods, Fai	r, HSG B				
20,201	61	>75% Gras	s cover, Go	ood, HSG B			
5,028	98	Paved park	ing, HSG E	В			
32,195	67	Weighted A	verage				
27,167	7	84.38% Pe	rvious Area	a			
5,028	3	15.62% lmp	pervious Ar	rea			
Tc Lengt			Capacity	Description			
(min) (fee	t) (ft/	ft) (ft/sec)	(cfs)				
5.0				Direct Entry,			

## Subcatchment 20S: Post Dev. Basin 4



## Summary for Subcatchment 21S: Post Dev. Basin 5 to SCM

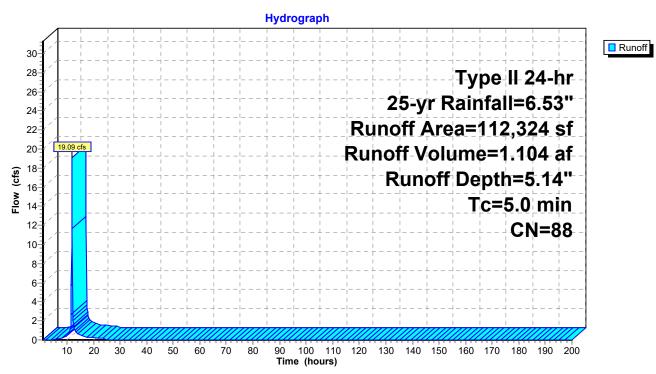
Runoff = 19.09 cfs @ 11.95 hrs, Volume= 1.104 af, Depth= 5.14"

Routed to Pond 3P: Wet Pond SCM 3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

Area (sf)	CN	Description	Description						
40,704	98	Roofs, HSG	B						
22,215	48	Brush, Goo	d, HSG B						
44,766	98	Paved park	ing, HSG B	В					
4,639	98	Water Surfa	ace, 0% imp	np, HSG B					
112,324	88	Weighted A	verage						
26,854		23.91% Per	vious Area	a					
85,470		76.09% Imp	pervious Ar	rea					
Tc Length	Slop	oe Velocity	Capacity	Description					
(min) (feet)	(ft/	ft) (ft/sec)	(cfs)						
5.0				Direct Entry.					

#### Subcatchment 21S: Post Dev. Basin 5 to SCM



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## Summary for Subcatchment 22S: Post Dev. Bypass 5

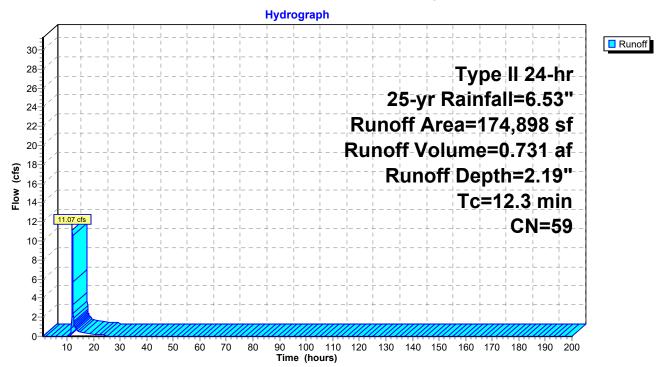
Runoff = 11.07 cfs @ 12.05 hrs, Volume= 0.731 af, Depth= 2.19"

Routed to Link 3L: POA 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

A	rea (sf)	CN	Description				
1	10,888	60	Woods, Fai	r, HSG B			
	51,602	48	Brush, Goo	d, HSG B			
	12,408	98	Paved park	ing, HSG B	3		
1	74,898	59	Weighted A	verage			
1	62,490		92.91% Per	vious Area	a a constant of the constant o		
	12,408		7.09% Impe	ervious Are	ea		
Tc	Length	Slope	e Velocity	Capacity	Description		
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)			
12.3					Direct Entry,		

## Subcatchment 22S: Post Dev. Bypass 5



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## Summary for Subcatchment 23S: Post Dev. Basin 6 to SCM

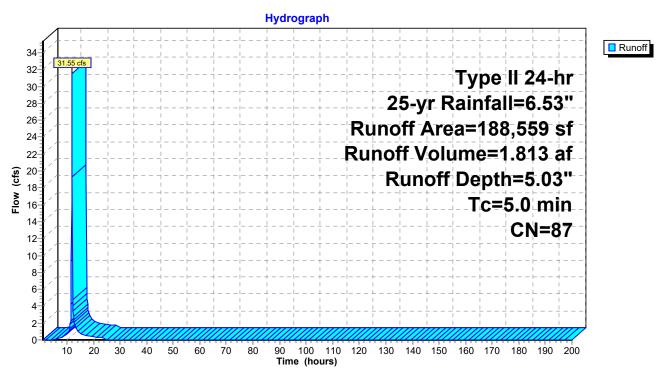
Runoff = 31.55 cfs @ 11.95 hrs, Volume= 1.813 af, Depth= 5.03"

Routed to Pond 4P: Wet Pond SCM 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

 Α	rea (sf)	CN	Description						
	54,466	98	Paved park	ing, HSG B	3				
	58,385	61	>75% Gras	s cover, Go	ood, HSG B				
	60,672	98	Roofs, HSG	βB					
	15,036	98	Water Surfa	ace, 0% imp	o, HSG B				
1	88,559	87	Weighted A	verage					
	73,421		38.94% Per	rvious Area					
1	15,138		61.06% Imp	pervious Ar	ea				
_									
	Length	Slope	,	Capacity	Description				
 (min)	(feet)	(ft/ft	(ft/sec)	(cfs)					
5.0					Direct Entry,				

#### Subcatchment 23S: Post Dev. Basin 6 to SCM



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## Summary for Subcatchment 24S: Post Dev. Bypass 6

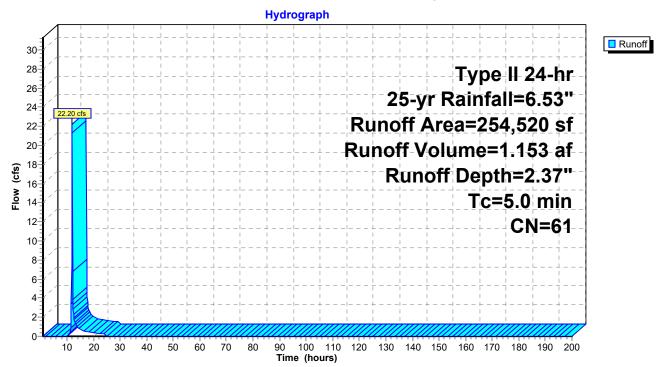
Runoff = 22.20 cfs @ 11.97 hrs, Volume= 1.153 af, Depth= 2.37"

Routed to Link 4L: POA 5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

Are	ea (sf)	CN	Description					
21	5,461	60	Woods, Fai	r, HSG B				
3	4,572	61	>75% Gras	s cover, Go	ood, HSG B			
	4,487	98	Paved park	ing, HSG E	В			
25	4,520	61	Weighted A	verage				
25	250,033 98.24% Pervious Area				a			
	4,487 1.76% Impervious Area			rvious Are	ea			
	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.0					Direct Entry,			

## Subcatchment 24S: Post Dev. Bypass 6



## Summary for Subcatchment 25S: Post Dev. Basin 7 to SCM

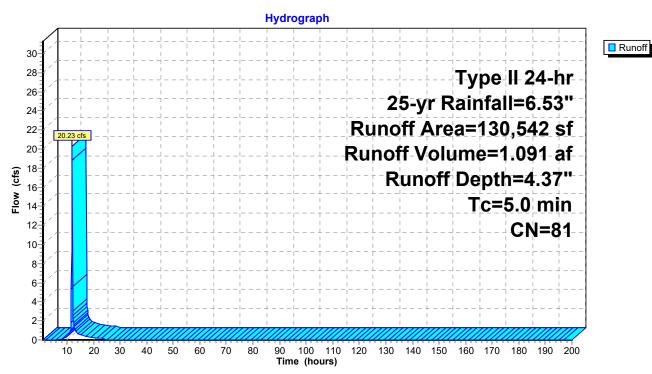
Runoff = 20.23 cfs @ 11.95 hrs, Volume= 1.091 af, Depth= 4.37"

Routed to Pond 5P: Wet Pond SCM 5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

Area (sf)	CN	Description					
50,624	98	Paved parking, HSG B					
44,621	48	Brush, Good, HSG B					
28,800	98	Roofs, HSG B					
6,497	98	Water Surface, 0% imp, HSG B					
130,542	81	Weighted Average					
51,118		39.16% Pervious Area					
79,424		60.84% Impervious Area					
Tc Length	n Slo <sub>l</sub>	pe Velocity Capacity Description					
(min) (feet)	) (ft/	(ft/ft) (ft/sec) (cfs)					
5.0		Direct Entry					

#### Subcatchment 25S: Post Dev. Basin 7 to SCM



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## Summary for Subcatchment 26S: Post Dev. Bypass 7

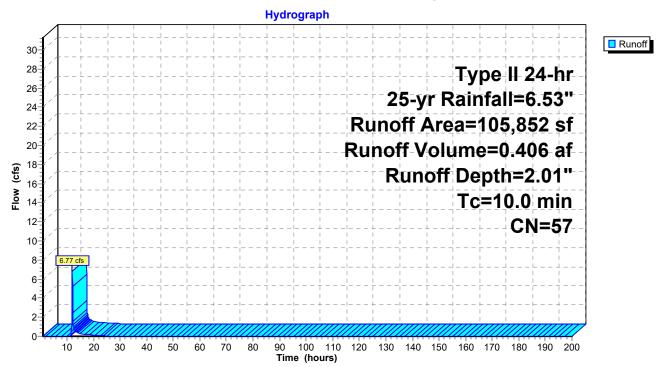
Runoff = 6.77 cfs @ 12.02 hrs, Volume= 0.406 af, Depth= 2.01"

Routed to Link 5L: POA 6

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

Area (s	sf) CN	Description	Description				
39,94	44 55	Woods, Go	od, HSG B	3			
52,48	36 48	Brush, Goo	d, HSG B				
13,42	22 98	Paved park	ing, HSG E	В			
105,8	52 57	Weighted A	verage				
92,43	30	87.32% Pe	rvious Area	a			
13,42	22	12.68% lm	pervious Ar	rea			
Tc Len	0		Capacity	·			
(min) (fe	et) (ft	/ft) (ft/sec)	(cfs)				
10.0				Direct Entry,			

## Subcatchment 26S: Post Dev. Bypass 7



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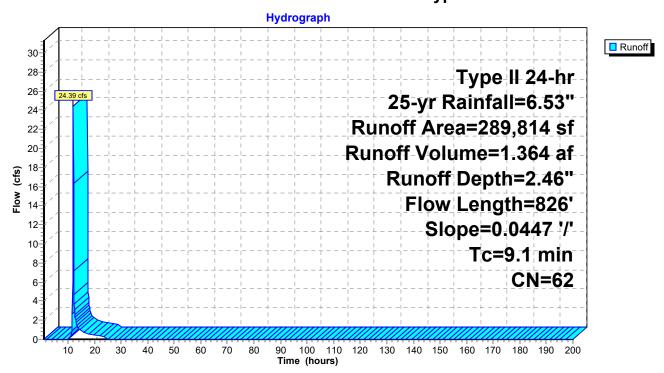
## Summary for Subcatchment 27S: Post Dev. Bypass 8

Runoff = 24.39 cfs @ 12.01 hrs, Volume= 1.364 af, Depth= 2.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 25-yr Rainfall=6.53"

A	rea (sf)	CN [	Description					
2	30,959	60 ۱	Voods, Fai	r, HSG B				
	53,439	69 5	50-75% Gra	ass cover, l	Fair, HSG B			
	5,416	98 F	Paved park	ing, HSG B	3			
2	89,814	62 \	Veighted A	verage				
2	284,398 98.13% Pervious Area				a a constant of the constant o			
	5,416 1.87% Impervious Area			ervious Are	ea			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
9.1	826	0.0447	1.51		Kirpich Method,			
					General overland flow k= 2.00			

#### Subcatchment 27S: Post Dev. Bypass 8



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## Summary for Pond 1P: Sand Filter -SCM 1

Inflow Area = 7.523 ac, 77.83% Impervious, Inflow Depth = 5.25" for 25-yr event

Inflow 56.50 cfs @ 11.95 hrs, Volume= 3.291 af

Outflow 28.94 cfs @ 12.08 hrs, Volume= 3.291 af, Atten= 49%, Lag= 7.9 min

3.291 af Primary 28.94 cfs @ 12.08 hrs, Volume=

Routed to Link 1L: POA 1

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Peak Elev= 531.55' @ 12.08 hrs Surf.Area= 6,825 sf Storage= 51,523 cf

Plug-Flow detention time= 90.7 min calculated for 3.290 af (100% of inflow)

Center-of-Mass det. time= 91.3 min (872.5 - 781.2)

Volume	Inve	rt Avail.Sto	rage Storage	Description			
#1	524.0	0' 68,25	50 cf Custom	Stage Data (Pri	smatic)Listed below (Recalc) x 65		
Elevation (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
524.00 109		105	0	0			
534.0	00	105	1,050	1,050			
Device	Routing	Invert	Outlet Device	s			
#1	Primary	524.00'	36.0" Round	l Culvert			
			L= 85.0' RCP, square edge headwall, Ke= 0.500				
			Inlet / Outlet Invert= 524.00' / 523.00' S= 0.0118 '/' Cc= 0.900				
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 7.07 sf				
#2	Device 1	524.00'	<b>8.0" Vert. Drawdown</b> C= 0.600 Limited to weir flow at low heads				
#3	Device 1	528.90'	60.0" W x 8.0	0.0" W x 8.0" H Vert. Main Orifice C= 0.600			
			Limited to we	ir flow at low head	ds		
#4 Device 1 533.00'		25.0' long Overflow 2 End Contraction(s)					

Primary OutFlow Max=28.42 cfs @ 12.08 hrs HW=531.46' (Free Discharge)

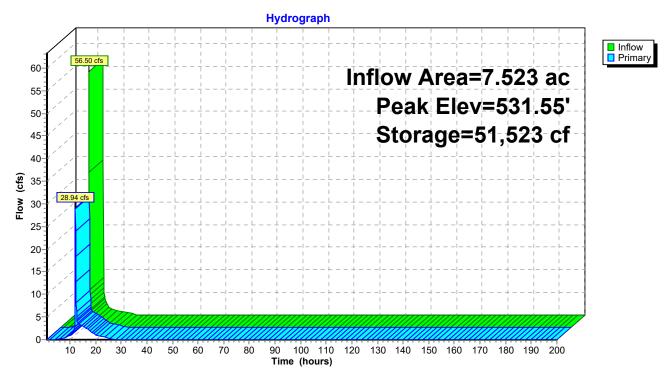
**-1=Culvert** (Passes 28.42 cfs of 83.09 cfs potential flow)

**2=Drawdown** (Orifice Controls 4.49 cfs @ 12.85 fps)

**-3=Main Orifice** (Orifice Controls 23.93 cfs @ 7.18 fps)

-4=Overflow (Controls 0.00 cfs)

Pond 1P: Sand Filter -SCM 1



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## **Summary for Pond 2P: Wet Pond SCM 2**

Inflow Area = 3.812 ac, 71.42% Impervious, Inflow Depth = 4.91" for 25-yr event

Inflow = 28.14 cfs @ 11.95 hrs, Volume= 1.561 af

Outflow = 5.86 cfs @ 12.15 hrs, Volume= 1.561 af, Atten= 79%, Lag= 12.1 min

Primary = 5.86 cfs @ 12.15 hrs, Volume= 1.561 af

Routed to Link 2L: POA 2

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

Starting Elev= 526.00' Surf.Area= 11,086 sf Storage= 32,033 cf

Peak Elev= 528.36' @ 12.15 hrs Surf.Area= 15,796 sf Storage= 63,678 cf (31,645 cf above start)

Plug-Flow detention time= 487.7 min calculated for 0.826 af (53% of inflow)

Center-of-Mass det. time= 177.3 min ( 967.9 - 790.6 )

Volume	Inve	ert Avail.Sto	orage Storage	Description				
#1	522.0	0' 92,4	29 cf Custom	Stage Data (Pi	rismatic)Listed below (Recalc)			
Elevation	on	Surf.Area	Inc.Store	Cum.Store				
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)				
522.0	00	5,067	0	0				
523.0	00	7,010	6,039	6,039				
524.0	00	7,727	7,369	13,407				
525.0	00	9,219	8,473	21,880				
526.0	00	11,086	10,153	32,033				
527.0		13,027	12,057	44,089				
528.0		15,043	14,035	58,124				
529.0		17,134	16,089	74,213				
530.0	00	19,299	18,217	92,429				
Device	Routing	Invert						
#1	Primary	526.00'						
				, ,	neadwall, Ke= 0.500			
					525.55' S= 0.0100 '/' Cc= 0.900			
				n= 0.010 Concrete pipe, straight & clean, Flow Area= 7.07 sf				
#2	Device 1	526.00'			600 Limited to weir flow at low heads			
#3	Device 1	527.40'			flow Orifice X 2.00 C= 0.600			
				r flow at low hea				
#4	Device 1	528.60'		Horiz. Overflow				
			Limited to wei	r flow at low hea	ads			

Primary OutFlow Max=5.84 cfs @ 12.15 hrs HW=528.35' (Free Discharge)

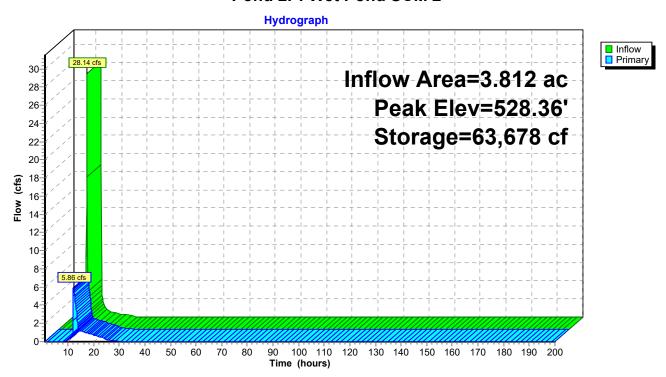
**-1=Culvert** (Passes 5.84 cfs of 27.39 cfs potential flow)

**2=Drawdown** (Orifice Controls 1.60 cfs @ 6.95 fps)

-3=Peakflow Orifice (Orifice Controls 4.24 cfs @ 4.49 fps)

**-4=Overflow** ( Controls 0.00 cfs)

Pond 2P: Wet Pond SCM 2



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## **Summary for Pond 3P: Wet Pond SCM 3**

Inflow Area = 2.579 ac, 76.09% Impervious, Inflow Depth = 5.14" for 25-yr event

Inflow 19.09 cfs @ 11.95 hrs, Volume= 1.104 af

8.22 cfs @ 12.11 hrs, Volume= Outflow 1.104 af, Atten= 57%, Lag= 9.4 min

Primary 8.22 cfs @ 12.11 hrs, Volume= 1.104 af

Routed to Link 3L: POA 4

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

Starting Elev= 534.00' Surf.Area= 3,130 sf Storage= 15,183 cf

Peak Elev= 537.18' @ 12.10 hrs Surf.Area= 5,841 sf Storage= 35,089 cf (19,906 cf above start)

Plug-Flow detention time= 394.0 min calculated for 0.755 af (68% of inflow)

Center-of-Mass det. time= 172.8 min (957.3 - 784.5)

Volume	Inver	t Avail.Sto	rage Storage [	Description	
#1	530.00	)' 52,86	62 cf Custom	Stage Data (Pi	rismatic)Listed below (Recalc)
<b>-</b> 14:.		N	la o Otana	0	
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(fee	t)	(sq-ft)	(cubic-feet)	(cubic-feet)	
530.0	0	2,212	0	0	
531.0	0	3,200	2,706	2,706	
532.0	0	4,144	3,672	6,378	
533.0	0	5,168	4,656	11,034	
534.0	0	3,130	4,149	15,183	
535.0	0	6,262	4,696	19,879	
536.0	0	8,640	7,451	27,330	
537.0	0	4,960	6,800	34,130	
538.0	0	9,920	7,440	41,570	
539.0		12,664	11,292	52,862	
Device	Routing	Invert	Outlet Devices	1	
#1	Primary	534.00'	30.0" Round	Culvert	
	-		L= 44.0' RCP	, square edge h	neadwall, Ke= 0.500
					533.56' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Cond	crete pipe, bend	ds & connections, Flow Area= 4.91 sf
#2	Device 1	534.00'		1 1 1	600 Limited to weir flow at low heads
#3	Device 1	536.30'			Flow X 2.00 C= 0.600
			Limited to weir	flow at low hea	ads
#4	Device 1	537.60'	48.0" x 48.0" l	Horiz. Overflov	<b>v</b> C= 0.600

Limited to weir flow at low heads

Primary OutFlow Max=8.16 cfs @ 12.11 hrs HW=537.16' (Free Discharge)

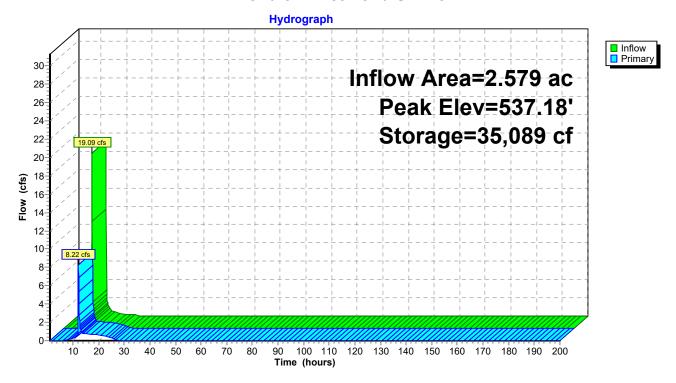
**-1=Culvert** (Passes 8.16 cfs of 31.61 cfs potential flow)

**2=Drawdown** (Orifice Controls 0.92 cfs @ 8.31 fps)

-3=Peak Flow (Orifice Controls 7.24 cfs @ 4.14 fps)

**-4=Overflow** (Controls 0.00 cfs)

#### Pond 3P: Wet Pond SCM 3



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## Summary for Pond 4P: Wet Pond SCM 4

Inflow Area = 4.329 ac, 61.06% Impervious, Inflow Depth = 5.03" for 25-yr event

Inflow 31.55 cfs @ 11.95 hrs, Volume= 1.813 af

Outflow 4.23 cfs @ 12.27 hrs, Volume= 1.488 af, Atten= 87%, Lag= 19.4 min

Primary 4.23 cfs @ 12.27 hrs, Volume= 1.488 af

Routed to Link 4L: POA 5

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Peak Elev= 526.41' @ 12.27 hrs Surf.Area= 12,316 sf Storage= 44,889 cf

Plug-Flow detention time= 539.3 min calculated for 1.488 af (82% of inflow)

Center-of-Mass det. time= 462.9 min (1,250.5 - 787.6)

Volume	Invert Ava	il.Storage	Storage	e Description	
#1	519.00'	67,235 cf	Custom	n Stage Data (Prismatic)Listed below (Recalc)	)
Elevation (feet)	Surf.Area (sq-ft)		Store c-feet)	Cum.Store (cubic-feet)	
519.00	1,472		0	0	
520.00	2,352		1,912	1,912	
521.00	3,406		2,879	4,791	
522.00	4,636		4,021	8,812	
523.00	6,046		5,341	14,153	
524.00	7,648		6,847	21,000	
525.00	9,474		8,561	29,561	
526.00	11,446	•	10,460	40,021	
527.00	13,570	•	12,508	52,529	
528.00	15,842	•	14,706	67,235	
Device Ro	outing Ir	nvert Outl	et Device	es	

Device	Routing	Invert	Outlet Devices
#1	Primary	523.00'	36.0" Round Culvert
			L= 45.0' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 523.00' / 521.50' S= 0.0333 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 7.07 sf
#2	Device 1	523.00'	<b>2.5" Vert. Drawdown</b> C= 0.600 Limited to weir flow at low heads
#3	Device 1	525.10'	<b>36.0" W x 3.0" H Vert. Peakflow</b> C= 0.600
			Limited to weir flow at low heads
#4	Device 1	526.50'	<b>48.0"</b> x <b>48.0"</b> Horiz. Overflow C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=4.22 cfs @ 12.27 hrs HW=526.41' (Free Discharge)

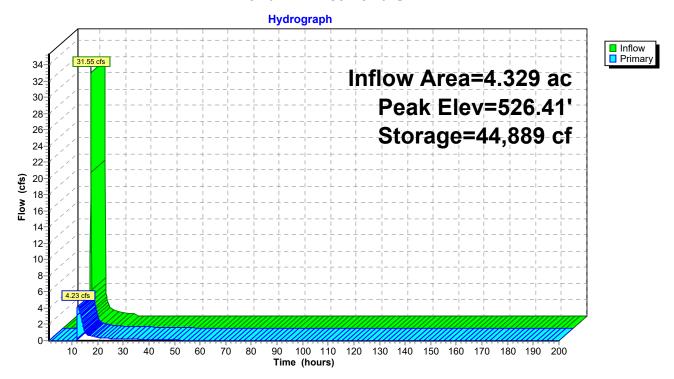
**-1=Culvert** (Passes 4.22 cfs of 47.01 cfs potential flow)

**2=Drawdown** (Orifice Controls 0.30 cfs @ 8.75 fps)

**-3=Peakflow** (Orifice Controls 3.93 cfs @ 5.23 fps)

-4=Overflow (Controls 0.00 cfs)

#### Pond 4P: Wet Pond SCM 4



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## **Summary for Pond 5P: Wet Pond SCM 5**

Inflow Area = 2.997 ac, 60.84% Impervious, Inflow Depth = 4.37" for 25-yr event

Inflow = 20.23 cfs @ 11.95 hrs, Volume= 1.091 af

Outflow = 8.33 cfs @ 12.10 hrs, Volume= 1.091 af, Atten= 59%, Lag= 9.1 min

Primary = 8.33 cfs @ 12.10 hrs, Volume= 1.091 af

Routed to Link 5L: POA 6

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

Starting Elev= 513.00' Surf.Area= 4,340 sf Storage= 13,131 cf

Peak Elev= 516.22' @ 12.10 hrs Surf.Area= 6,936 sf Storage= 32,230 cf (19,099 cf above start)

Plug-Flow detention time= 378.4 min calculated for 0.789 af (72% of inflow)

Center-of-Mass det. time= 180.8 min ( 985.0 - 804.2 )

Volume	Inve	rt Avail.Sto	rage Sto	orage Description				
#1	509.00	0' 45,4	75 cf <b>C</b> u	stom Stage Data (Prismatic)Listed below (Recalc)				
Clayatia	n (	Surf.Area	Inc.Sto	ora Cum Stora				
Elevatio								
(fee		(sq-ft)	(cubic-fe	<del></del>				
509.0		2,355		0 0				
510.0		2,789	2,5					
511.0	0	3,517	3,1	53 5,725				
512.0	0	3,477	3,4	97 9,222				
513.0	0	4,340	3,9	09 13,131				
514.0	0	5,752	5,0	46 18,177				
515.0	0	6,271	6,0	12 24,188				
516.0	0	6,813	6,5	42 30,730				
517.0	0	7,377	7,0	95 37,825				
518.0		7,922	7,6	·				
Device	Routing	Invert	Outlet D	evices				
#1	Primary	513.00'	36.0" R	ound Culvert				
		L=		RCP, square edge headwall, Ke= 0.500				
			Inlet / O	utlet Invert= 513.00' / 512.00' S= 0.0127 '/' Cc= 0.900				
				n= 0.013 Concrete pipe, bends & connections, Flow Area= 7.07 sf				
#2	Device 1	513.00'		t. Drawdown C= 0.600 Limited to weir flow at low heads				
#3	Device 1	515.10'		x 3.0" H Vert. Peakflow Orifice X 2.00 C= 0.600				
		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		to weir flow at low heads				
#4	Device 1	516.70'		<b>48.0" Horiz. Overflow</b> C= 0.600				
		5.5.7.6		to weir flow at low heads				

Primary OutFlow Max=8.29 cfs @ 12.10 hrs HW=516.21' (Free Discharge)

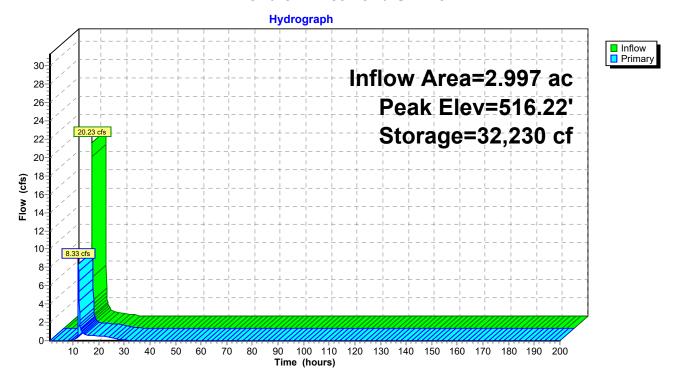
**-1=Culvert** (Passes 8.29 cfs of 44.49 cfs potential flow)

**2=Drawdown** (Orifice Controls 0.73 cfs @ 8.40 fps)

-3=Peakflow Orifice (Orifice Controls 7.56 cfs @ 4.77 fps)

-4=Overflow (Controls 0.00 cfs)

#### Pond 5P: Wet Pond SCM 5



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# Summary for Link 1L: POA 1

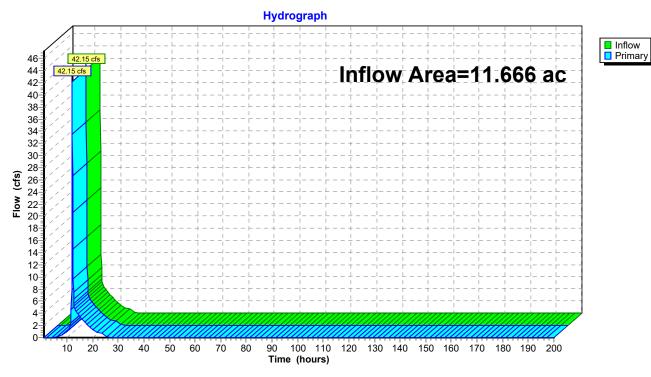
Inflow Area = 11.666 ac, 51.60% Impervious, Inflow Depth = 4.19" for 25-yr event

Inflow = 42.15 cfs @ 12.02 hrs, Volume= 4.077 af

Primary = 42.15 cfs @ 12.02 hrs, Volume= 4.077 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

## Link 1L: POA 1



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# **Summary for Link 2L: POA 2**

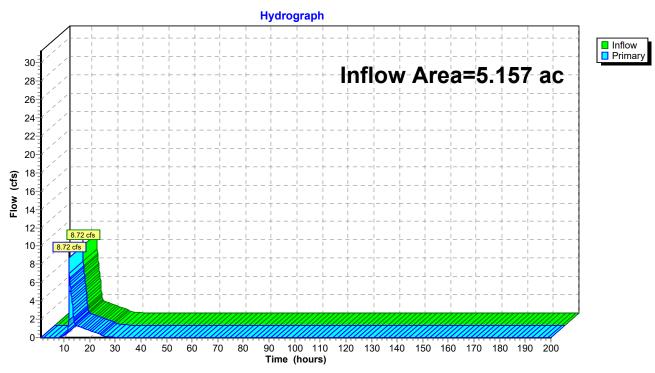
Inflow Area = 5.157 ac, 52.79% Impervious, Inflow Depth = 4.09" for 25-yr event

Inflow = 8.72 cfs @ 12.02 hrs, Volume= 1.757 af

Primary = 8.72 cfs @ 12.02 hrs, Volume= 1.757 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

## Link 2L: POA 2



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# **Summary for Link 3L: POA 4**

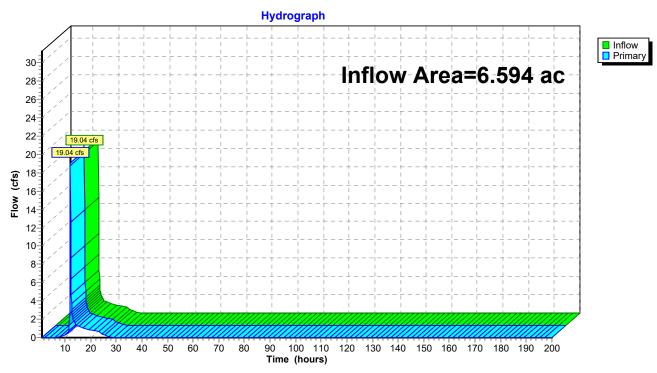
Inflow Area = 6.594 ac, 34.08% Impervious, Inflow Depth = 3.34" for 25-yr event

Inflow = 19.04 cfs @ 12.07 hrs, Volume= 1.835 af

Primary = 19.04 cfs @ 12.07 hrs, Volume= 1.835 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

## Link 3L: POA 4



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# Summary for Link 4L: POA 5

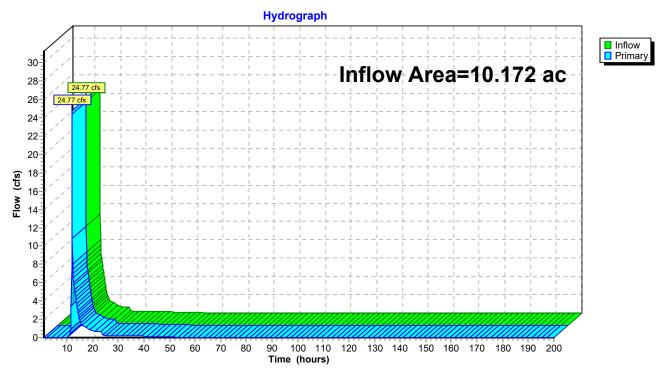
Inflow Area = 10.172 ac, 27.00% Impervious, Inflow Depth = 3.12" for 25-yr event

Inflow = 24.77 cfs @ 11.98 hrs, Volume= 2.641 af

Primary = 24.77 cfs @ 11.98 hrs, Volume= 2.641 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

## Link 4L: POA 5



# Summary for Link 5L: POA 6

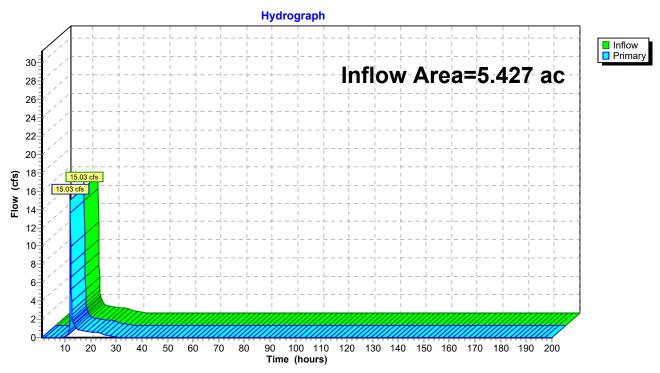
Inflow Area = 5.427 ac, 39.28% Impervious, Inflow Depth = 3.31" for 25-yr event

Inflow = 15.03 cfs @ 12.05 hrs, Volume= 1.498 af

Primary = 15.03 cfs @ 12.05 hrs, Volume= 1.498 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

## Link 5L: POA 6



Time span=1.00-200.00 hrs, dt=0.10 hrs, 1991 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: Pre Dev. Basin 1 Runoff Area=366,801 sf 0.00% Impervious Runoff Depth=5.66" Flow Length=560' Slope=0.0630 '/' Tc=5.9 min CN=79 Runoff=73.83 cfs 3.970 af Subcatchment2S: Pre Dev. Basin 2A Runoff Area=250,337 sf 0.00% Impervious Runoff Depth=5.66" Flow Length=803' Slope=0.0650 '/' Tc=7.7 min CN=79 Runoff=49.72 cfs 2.709 af Runoff Area=132,113 sf 0.00% Impervious Runoff Depth=3.45" Subcatchment3S: Pre Dev. Basin 2B Flow Length=577' Slope=0.0451 '/' Tc=6.9 min CN=60 Runoff=16.86 cfs 0.873 af Runoff Area=467,738 sf 32.11% Impervious Runoff Depth=6.49" Subcatchment4S: Pre Dev. Basin 2C Flow Length=753' Slope=0.0633'/' Tc=7.4 min CN=86 Runoff=103.18 cfs 5.804 af Runoff Area=52,001 sf 0.00% Impervious Runoff Depth=5.54" Subcatchment6S: Pre Dev. Basin 3 Flow Length=243' Slope=0.0170 '/' Tc=5.1 min CN=78 Runoff=10.20 cfs 0.551 af Subcatchment7S: Pre Dev. Basin 4 Runoff Area=130,388 sf 0.00% Impervious Runoff Depth=3.34" Flow Length=561' Slope=0.0221 '/' Tc=8.9 min CN=59 Runoff=15.12 cfs 0.833 af Subcatchment8S: Pre Dev. Basin 5 Runoff Area=305,128 sf 0.00% Impervious Runoff Depth=3.34" Flow Length=998' Slope=0.0301 '/' Tc=12.3 min CN=59 Runoff=30.39 cfs 1.950 af Subcatchment9S: Pre Dev. Basin 6 Runoff Area=441,055 sf 0.00% Impervious Runoff Depth=3.45" Flow Length=1,222' Slope=0.0441 '/' Tc=12.4 min CN=60 Runoff=45.38 cfs 2.914 af Subcatchment 10S: Pre Dev. Basin 7 Runoff Area=238,293 sf 0.00% Impervious Runoff Depth=3.12" Flow Length=977' Slope=0.0468 '/' Tc=10.2 min CN=57 Runoff=24.16 cfs 1.421 af Runoff Area=289,814 sf 1.87% Impervious Runoff Depth=3.68" Subcatchment11S: Pre Dev. Basin 8 Flow Length=826' Slope=0.0447'/' Tc=9.1 min CN=62 Runoff=36.89 cfs 2.041 af Subcatchment 12S: Post Basin 1 to SCM Runoff Area = 327,690 sf 77.83% Impervious Runoff Depth = 6.84" Tc=5.0 min CN=89 Runoff=72.49 cfs 4.290 af Subcatchment13S: Post Dev Bypass 1 Runoff Area=180,474 sf 3.97% Impervious Runoff Depth=3.45" Tc=5.0 min CN=60 Runoff=23.00 cfs 1.192 af Runoff Area=120,581 sf 0.00% Impervious Runoff Depth=5.66" Subcatchment14S: Post Dev. Bypass 2A Flow Length=421' Slope=0.0411 '/' Tc=5.6 min CN=79 Runoff=24.21 cfs 1.305 af Subcatchment15S: Post Dev. Basin 2B Runoff Area=166,052 sf 71.42% Impervious Runoff Depth=6.49" Tc=5.0 min CN=86 Runoff=35.51 cfs 2.061 af

Subcatchment18S: Post Dev Bypass 2B Runoff Area=58,575 sf 0.00% Impervious Runoff Depth=2.78"

Tc=5.0 min CN=54 Runoff=5.99 cfs 0.312 af

Subcatchment16S: Post Dev. Bypass 2C Runoff Area=460,928 sf 34.20% Impervious Runoff Depth=6.49"

Flow Length=753' Slope=0.0633'/' Tc=7.4 min CN=86 Runoff=101.68 cfs 5.720 af

<b>32044.0000 - CZ</b> Prepared by Thomas & Hutton  HydroCAD® 10.20-6a s/n 02108 © 2024 Hyd	Type II 24-hr 100-yr Rainfall=8.16" Printed 3/4/2025 roCAD Software Solutions LLC Page 140
Subcatchment19S: Post Dev. Basin 3	Runoff Area=48,549 sf 0.00% Impervious Runoff Depth=5.66" Tc=5.0 min CN=79 Runoff=9.67 cfs 0.525 af
Subcatchment20S: Post Dev. Basin 4	Runoff Area=32,195 sf 15.62% Impervious Runoff Depth=4.25" Tc=5.0 min CN=67 Runoff=5.01 cfs 0.262 af
Subcatchment21S: Post Dev. Basin 5 to	Runoff Area=112,324 sf 76.09% Impervious Runoff Depth=6.72" Tc=5.0 min CN=88 Runoff=24.59 cfs 1.445 af
Subcatchment22S: Post Dev. Bypass 5	Runoff Area=174,898 sf 7.09% Impervious Runoff Depth=3.34" Tc=12.3 min CN=59 Runoff=17.42 cfs 1.118 af
Subcatchment23S: Post Dev. Basin 6 to	Runoff Area=188,559 sf 61.06% Impervious Runoff Depth=6.61" Tc=5.0 min CN=87 Runoff=40.81 cfs 2.383 af
Subcatchment24S: Post Dev. Bypass 6	Runoff Area=254,520 sf 1.76% Impervious Runoff Depth=3.57" Tc=5.0 min CN=61 Runoff=33.49 cfs 1.737 af
Subcatchment25S: Post Dev. Basin 7 to	Runoff Area=130,542 sf 60.84% Impervious Runoff Depth=5.89" Tc=5.0 min CN=81 Runoff=26.83 cfs 1.472 af
Subcatchment26S: Post Dev. Bypass 7	Runoff Area=105,852 sf 12.68% Impervious Runoff Depth=3.12" Tc=10.0 min CN=57 Runoff=10.83 cfs 0.631 af
Subcatchment27S: Post Dev. Bypass 8 Flow Length=826'	Runoff Area=289,814 sf 1.87% Impervious Runoff Depth=3.68" Slope=0.0447 '/' Tc=9.1 min CN=62 Runoff=36.89 cfs 2.041 af
Pond 1P: Sand Filter -SCM 1	Peak Elev=533.15' Storage=62,453 cf Inflow=72.49 cfs 4.290 af Outflow=39.51 cfs 4.290 af
Pond 2P: Wet Pond SCM 2	Peak Elev=528.87' Storage=72,072 cf Inflow=35.51 cfs 2.061 af Outflow=14.55 cfs 2.061 af
Pond 3P: Wet Pond SCM 3	Peak Elev=537.75' Storage=39,211 cf Inflow=24.59 cfs 1.445 af Outflow=13.39 cfs 1.445 af
Pond 4P: Wet Pond SCM 4	Peak Elev=526.97' Storage=52,154 cf Inflow=40.81 cfs 2.383 af Outflow=22.12 cfs 2.058 af
Pond 5P: Wet Pond SCM 5	Peak Elev=516.90' Storage=37,058 cf Inflow=26.83 cfs 1.472 af

Link 1L: POA 1

Link 2L: POA 2

Link 3L: POA 4

Outflow=15.02 cfs 1.472 af

Inflow=56.96 cfs 5.483 af Primary=56.96 cfs 5.483 af

Inflow=16.28 cfs 2.373 af Primary=16.28 cfs 2.373 af

Inflow=30.22 cfs 2.563 af

Primary=30.22 cfs 2.563 af

**32044.0000 - CZ**Prepared by Thomas & Hutton

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Type II 24-hr 100-yr Rainfall=8.16"

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Link 4L: POA 5 Inflow=43.63 cfs 3.795 af

Primary=43.63 cfs 3.795 af

**Link 5L: POA 6**Inflow=23.82 cfs 2.103 af
Primary=23.82 cfs 2.103 af

Total Runoff Area = 122.250 ac Runoff Volume = 49.560 af Average Runoff Depth = 4.86" 80.94% Pervious = 98.954 ac 19.06% Impervious = 23.297 ac

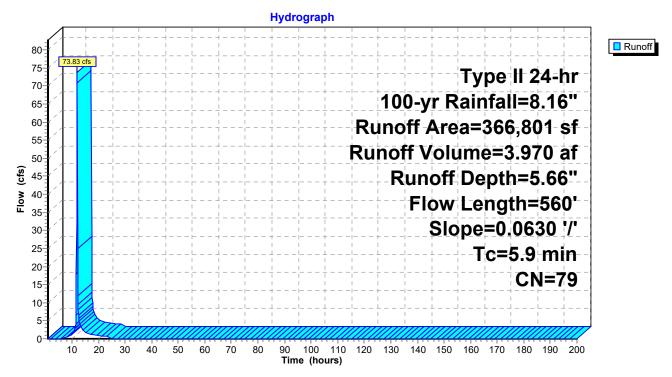
## Summary for Subcatchment 1S: Pre Dev. Basin 1

Runoff = 73.83 cfs @ 11.97 hrs, Volume= 3.970 af, Depth= 5.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

	rea (sf)	CN	Description		
;	342,042	79	Woods, Fai	r, HSG D	
	24,759	73	Brush, Goo	d, HSG D	
;	366,801	79	Weighted A	verage	
;	366,801	66,801 100.00% Pervious Area			a
Tc	Length	Slope	,	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.9	560	0.0630	1.58		Kirpich Method, Estimated Tc
					General overland flow k= 2.00

#### Subcatchment 1S: Pre Dev. Basin 1



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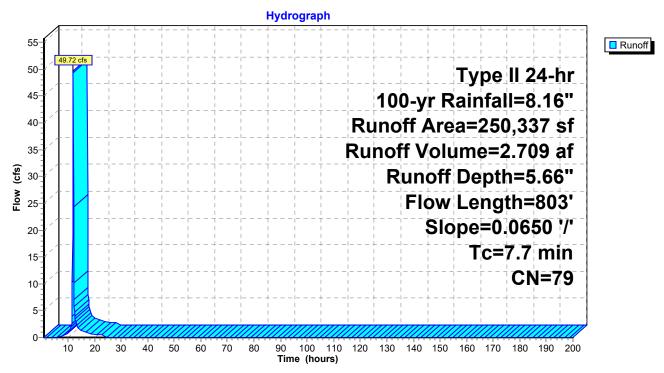
## Summary for Subcatchment 2S: Pre Dev. Basin 2A

Runoff = 49.72 cfs @ 11.99 hrs, Volume= 2.709 af, Depth= 5.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

_	Α	rea (sf)	CN	Description		
	2	250,337	79	Woods, Fai	r, HSG D	
	2	250,337		100.00% P	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	7.7	803	0.0650		(6.6)	Kirpich Method, Estimated Tc General overland flow k= 2.00

## Subcatchment 2S: Pre Dev. Basin 2A



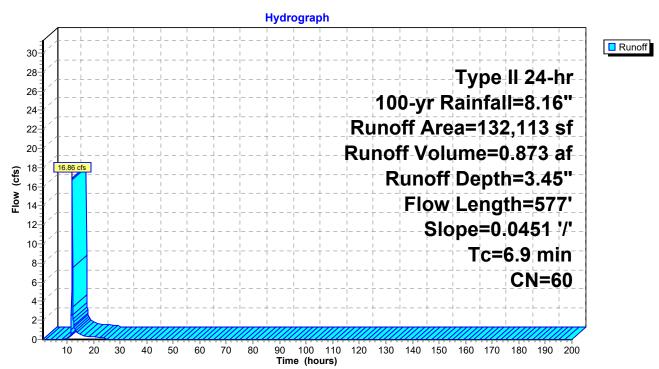
# Summary for Subcatchment 3S: Pre Dev. Basin 2B

Runoff = 16.86 cfs @ 11.99 hrs, Volume= 0.873 af, Depth= 3.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

 Α	rea (sf)	CN [	Description		
1	32,113	60 \	Noods, Fai	r, HSG B	
 1	32,113	,	100.00% P	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.9	577	0.0451	1.40		Kirpich Method, Estimated Tc General overland flow k= 2.00

#### Subcatchment 3S: Pre Dev. Basin 2B



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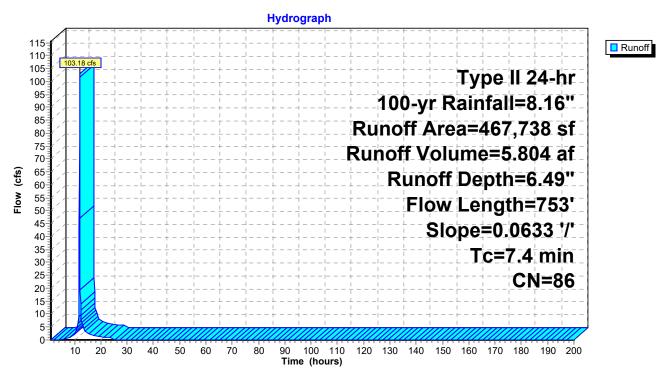
## Summary for Subcatchment 4S: Pre Dev. Basin 2C

Runoff = 103.18 cfs @ 11.98 hrs, Volume= 5.804 af, Depth= 6.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

Area (sf)		CN E	Description			
173,689		79 V	Woods, Fair, HSG D			
94,275		98 F	Paved parking, HSG D			
143,869		82 V	Woods/grass comb., Fair, HSG D			
55,905		98 F	Roofs, HSG D			
467,738		86 V	Weighted Average			
317,558		6	67.89% Pervious Area			
150,180		3	32.11% Impervious Area			
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
7.4	753	0.0633	1.69		Kirpich Method, Estimated Tc	
					General overland flow k= 2.00	

#### Subcatchment 4S: Pre Dev. Basin 2C



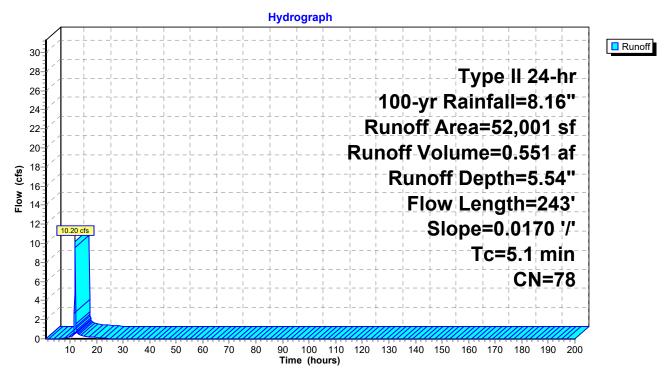
## Summary for Subcatchment 6S: Pre Dev. Basin 3

Runoff = 10.20 cfs @ 11.96 hrs, Volume= 0.551 af, Depth= 5.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

	Area (sf)	CN	Description				
	9,925	73	Woods, Fai	r, HSG C			
	42,076	79	50-75% Gra	ass cover, f	Fair, HSG C		
	52,001	78	Weighted Average				
	52,001		100.00% Pervious Area				
To	9	Slope	,	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
5.1	243	0.0170	0.79		Kirpich Method, Estimated Tc		
					General overland flow k= 2.00		

#### Subcatchment 6S: Pre Dev. Basin 3



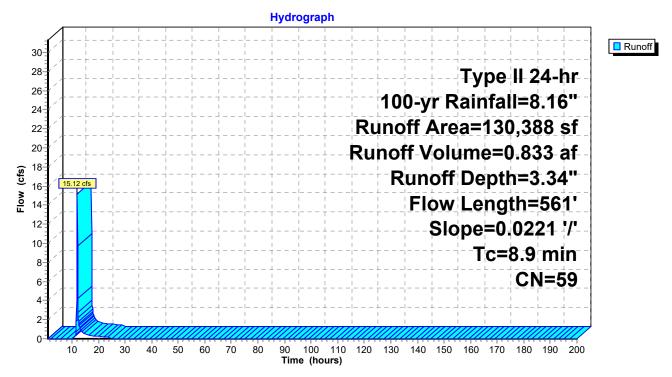
## Summary for Subcatchment 7S: Pre Dev. Basin 4

Runoff = 15.12 cfs @ 12.00 hrs, Volume= 0.833 af, Depth= 3.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

	Α	rea (sf)	CN	Description						
	1	04,102	60	Woods, Fai	r, HSG B					
_		26,286	56	Brush, Fair,	, HSG B					
	1	30,388	59	Weighted A	verage					
	1	30,388		100.00% Pe	ervious Are	a				
	Тс	Length	Slope	,	Capacity	Description				
_	(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)					
	8.9	561	0.022	1.06		Kirpich Method,				
						General overland flow	k= 2.00			

#### Subcatchment 7S: Pre Dev. Basin 4



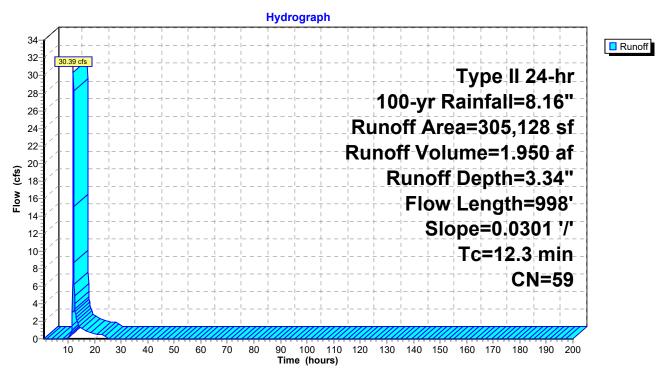
#### Summary for Subcatchment 8S: Pre Dev. Basin 5

Runoff = 30.39 cfs @ 12.05 hrs, Volume= 1.950 af, Depth= 3.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

A	rea (sf)	CN	Description		
2	267,919	60	Woods, Fai	r, HSG B	
	37,209	48	Brush, Goo	d, HSG B	
3	305,128	59	Weighted A	verage	
3	305,128		100.00% Pe	ervious Are	a
Tc	Length	Slope	,	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.3	998	0.0301	1.36		Kirpich Method, Estimated Tc
					General overland flow k= 2.00

#### Subcatchment 8S: Pre Dev. Basin 5



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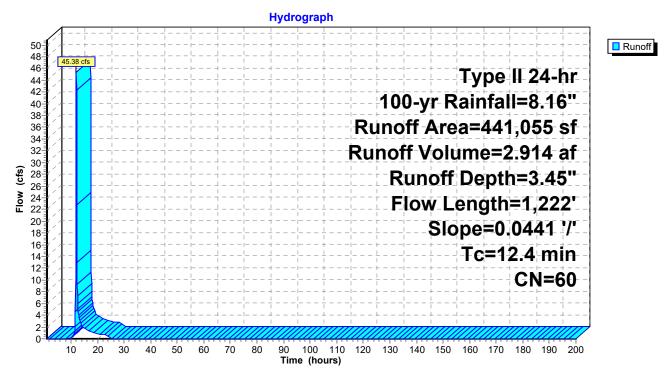
## Summary for Subcatchment 9S: Pre Dev. Basin 6

Runoff = 45.38 cfs @ 12.05 hrs, Volume= 2.914 af, Depth= 3.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

A	rea (sf)	CN	Description		
4	01,238	60	Woods, Fai	r, HSG B	
	39,817	56	Brush, Fair,	HSG B	
441,055 60 Weighted Average				verage	
4	441,055 100.00% Pervious Area			ervious Are	a
Tc	Length	Slope	,	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.4	1,222	0.0441	1.65		Kirpich Method, Estimated Tc
					General overland flow k= 2.00

#### Subcatchment 9S: Pre Dev. Basin 6



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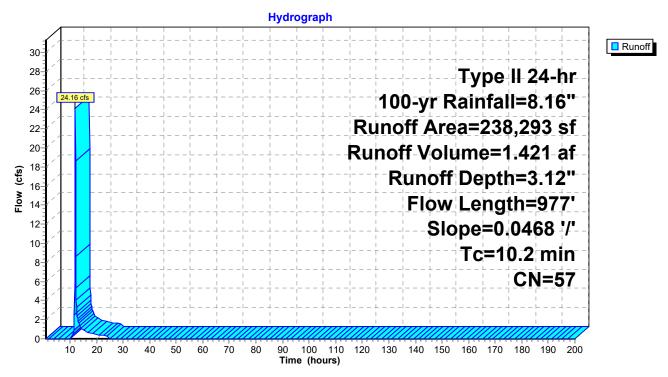
## Summary for Subcatchment 10S: Pre Dev. Basin 7

Runoff = 24.16 cfs @ 12.02 hrs, Volume= 1.421 af, Depth= 3.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

	Area (sf)	CN	Description						
	207,447	55	Woods, Good, HSG B						
	30,846	69	Pasture/gra	ssland/ran	ge, Fair, HSG B				
	238,293		Weighted A						
	238,293		100.00% P	ervious Are	ea				
To	9	Slope	,	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
10.2	977	0.0468	1.60		Kirpich Method, Estimated Tc				
					General overland flow k= 2.00				

#### Subcatchment 10S: Pre Dev. Basin 7



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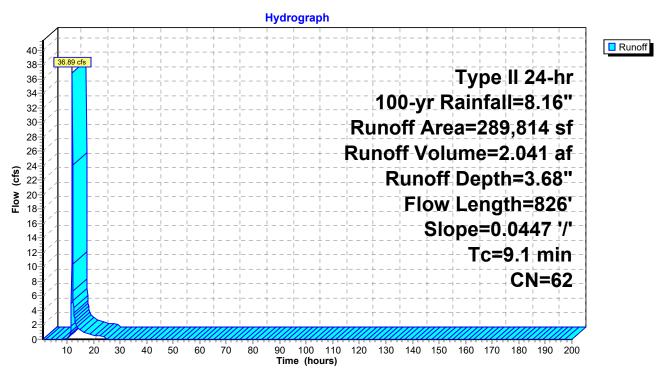
# Summary for Subcatchment 11S: Pre Dev. Basin 8

Runoff = 36.89 cfs @ 12.00 hrs, Volume= 2.041 af, Depth= 3.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

Area	(sf) CN	Description	Description						
230,9	959 60	Woods, Fa	ir, HSG B						
53,4	439 69	50-75% Gr	ass cover, l	Fair, HSG B					
5,4	416 98	Paved park	king, HSG E	3					
289,8	314 62	Weighted A	Average						
284,3	398	98.13% Pe	rvious Area	a					
5,4	116	1.87% Imp	ervious Are	ea					
	ngth Slo		Capacity	Description					
<u>(min)</u> (1	feet) (ft	/ft) (ft/sec)	(cfs)						
9.1	826 0.04	47 1.51		Kirpich Method,					
				General overland flow k= 2.00					

#### Subcatchment 11S: Pre Dev. Basin 8



## Summary for Subcatchment 12S: Post Basin 1 to SCM

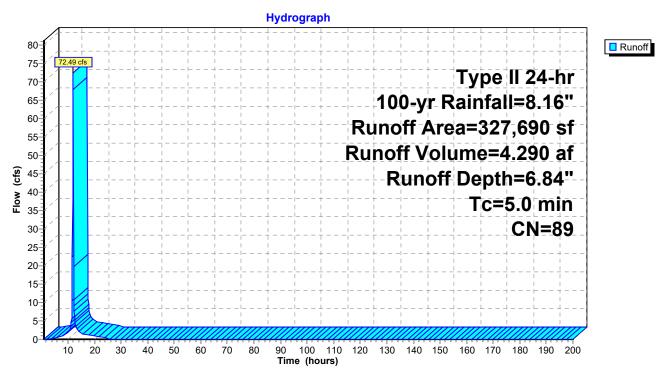
Runoff = 72.49 cfs @ 11.95 hrs, Volume= 4.290 af, Depth= 6.84"

Routed to Pond 1P: Sand Filter -SCM 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

Are	ea (sf)	CN	Description						
10	4,903	98	Paved park	ing, HSG A	4				
7	2,655	56	Brush, Fair,	HSG B					
15	0,132	98	Roofs, HSG	βB					
32	27,690	89	9 Weighted Average						
7	2,655		22.17% Per	vious Area	a a constant of the constant o				
25	55,035		77.83% lmp	ervious Ar	rea				
Tc	Length	Slope	<ul><li>Velocity</li></ul>	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.0					Direct Entry,				

#### Subcatchment 12S: Post Basin 1 to SCM



## Summary for Subcatchment 13S: Post Dev Bypass 1

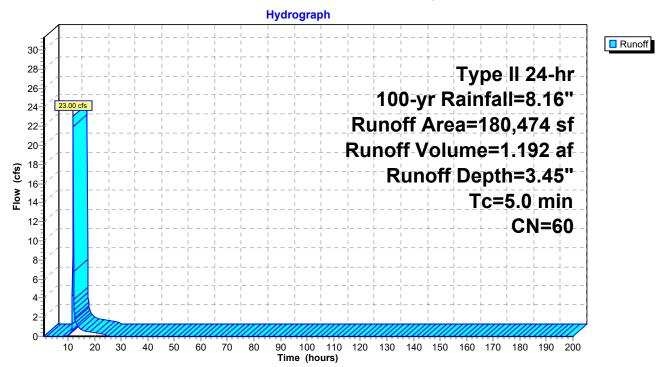
Runoff = 23.00 cfs @ 11.97 hrs, Volume= 1.192 af, Depth= 3.45"

Routed to Link 1L: POA 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

Are	ea (sf)	CN	Description					
12	4,058	60	Woods, Fai	r, HSG B				
4	9,243	56	Brush, Fair,	HSG B				
	7,173	98	Paved park	ing, HSG B	3			
18	0,474	60	Weighted Average					
17	3,301		96.03% Pei	vious Area	a			
•	7,173		3.97% Impe	ervious Area	ea			
	Length	Slope	•	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.0					Direct Entry,			

## **Subcatchment 13S: Post Dev Bypass 1**



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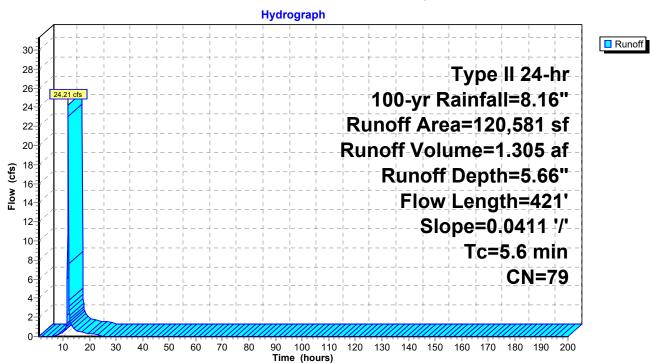
## Summary for Subcatchment 14S: Post Dev. Bypass 2A

Runoff = 24.21 cfs @ 11.96 hrs, Volume= 1.305 af, Depth= 5.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

A	rea (sf)	CN	Description		
1	20,581	79	Woods, Fai	r, HSG D	
1	20,581	100.00% Pervious Are			a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.6	421	0.0411	1.25		Kirpich Method, Estimated Tc General overland flow k= 2.00

# Subcatchment 14S: Post Dev. Bypass 2A



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## Summary for Subcatchment 15S: Post Dev. Basin 2B to SCM

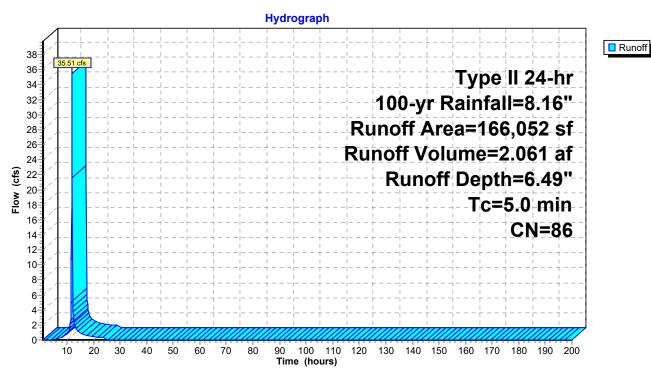
Runoff = 35.51 cfs @ 11.95 hrs, Volume= 2.061 af, Depth= 6.49"

Routed to Pond 2P: Wet Pond SCM 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

Area (sf	) CN	Description							
22,91	4 98	Paved parking, HSG B							
6,46	5 48	Brush, Good, HSG B							
95,673	3 98	Roofs, HSG B							
41,000	0 58	Woods/grass comb., Good, HSG B							
166,052	2 86	6 Weighted Average							
47,46	5	28.58% Pervious Area							
118,58	7	71.42% Impervious Area							
Tc Leng	th Slo	pe Velocity Capacity Description							
(min) (fee	et) (ft/	/ft) (ft/sec) (cfs)							
5.0		Direct Entry							

#### Subcatchment 15S: Post Dev. Basin 2B to SCM



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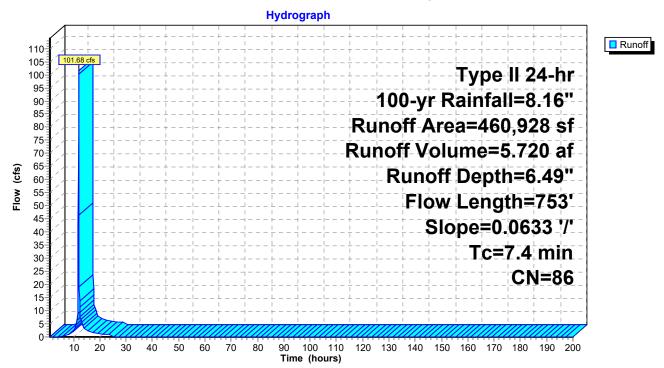
## Summary for Subcatchment 16S: Post Dev. Bypass 2C

Runoff = 101.68 cfs @ 11.98 hrs, Volume= 5.720 af, Depth= 6.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

A	rea (sf)	CN E	Description					
1	73,689	79 V	Voods, Fai	r, HSG D				
1	01,714	98 F	Paved park	ing, HSG D	)			
1	29,620	82 V	Voods/gras	s comb., F	Fair, HSG D			
	55,905 98 Roofs, HSG D							
4	60,928	86 V	Veighted A	verage				
3	303,309	6	5.80% Per	vious Area	1			
1	57,619	3	4.20% Imp	ervious Ar	rea			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
7.4	753	0.0633	1.69		Kirpich Method, Estimated Tc			
					General overland flow k= 2.00			

# Subcatchment 16S: Post Dev. Bypass 2C



# Summary for Subcatchment 18S: Post Dev Bypass 2B

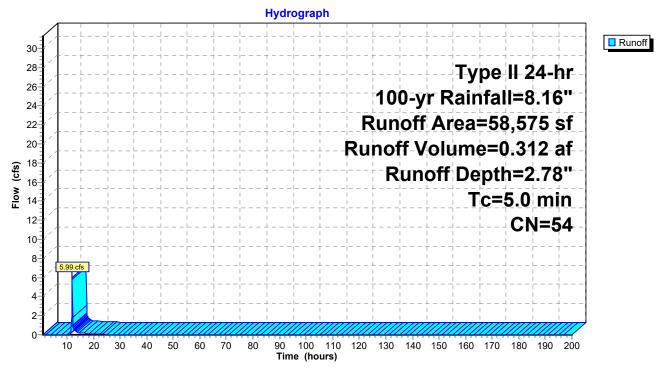
Runoff = 5.99 cfs @ 11.97 hrs, Volume= 0.312 af, Depth= 2.78"

Routed to Link 2L: POA 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

_	Α	rea (sf)	CN	Description						
_		54,125	55	Woods, Good, HSG B						
_		4,450	48	Brush, Good, HSG B						
_		58,575	54	54 Weighted Average						
		58,575		100.00% Pe	ervious Are	ea				
	Tc	Length	Slope	e Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	) (ft/sec)	(cfs)					
	5.0					Direct Entry				

# Subcatchment 18S: Post Dev Bypass 2B



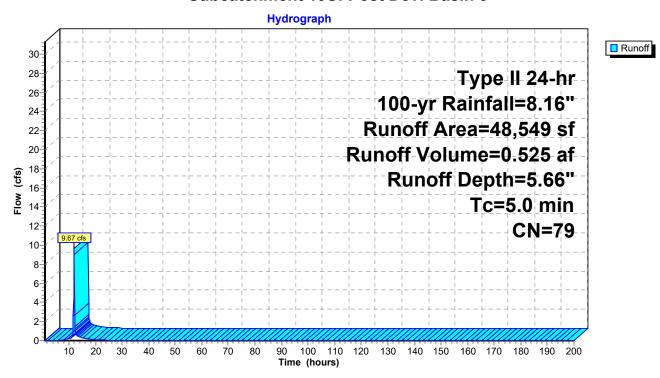
## Summary for Subcatchment 19S: Post Dev. Basin 3

Runoff = 9.67 cfs @ 11.95 hrs, Volume= 0.525 af, Depth= 5.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

A	rea (sf)	CN E	Description						
	48,549	79 5	50-75% Grass cover, Fair, HSG C						
	48,549	1	100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry,				

#### Subcatchment 19S: Post Dev. Basin 3



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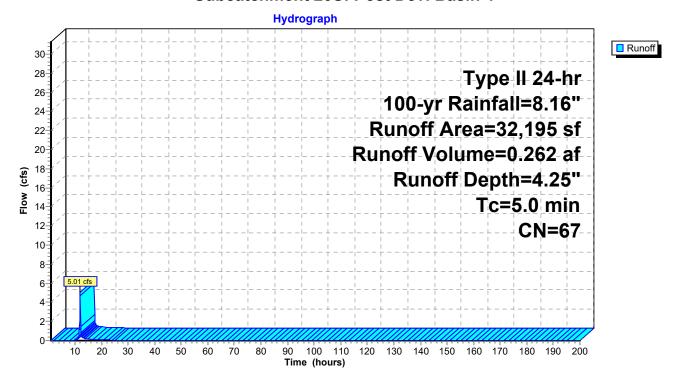
## Summary for Subcatchment 20S: Post Dev. Basin 4

Runoff = 5.01 cfs @ 11.96 hrs, Volume= 0.262 af, Depth= 4.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

Area (sf)	CN	Description						
6,966	60	Woods, Fair	r, HSG B					
20,201	61	>75% Grass	s cover, Go	Good, HSG B				
5,028	98	Paved parki	ing, HSG B	В				
32,195	67	7 Weighted Average						
27,167		84.38% Per	vious Area	a				
5,028		15.62% Imp	ervious Ar	urea				
Tc Lengt	h Slo <sub>l</sub>	pe Velocity	Capacity	/ Description				
(min) (feet	t) (ft/	ft) (ft/sec)	(cfs)					
5.0				Direct Entry.				

#### Subcatchment 20S: Post Dev. Basin 4



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## Summary for Subcatchment 21S: Post Dev. Basin 5 to SCM

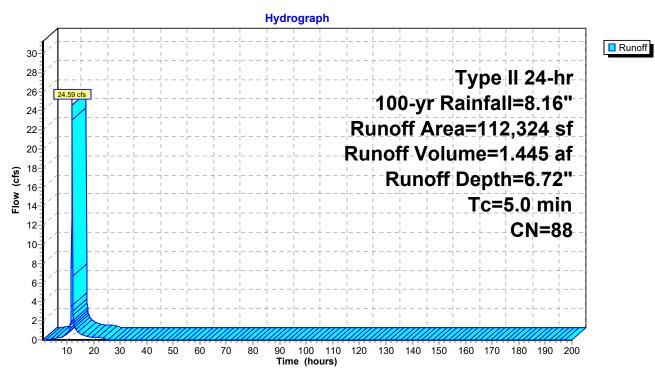
Runoff = 24.59 cfs @ 11.95 hrs, Volume= 1.445 af, Depth= 6.72"

Routed to Pond 3P: Wet Pond SCM 3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

Area (sf)	CN	Description						
40,704	98	Roofs, HSG	B					
22,215	48	Brush, Goo	d, HSG B					
44,766	98	Paved park	ing, HSG B	В				
4,639	98	Water Surfa	ace, 0% imp	np, HSG B				
112,324	88	Weighted A	Weighted Average					
26,854		23.91% Per	vious Area	a				
85,470		76.09% Imp	pervious Ar	rea				
Tc Length	Slop	oe Velocity	Capacity	Description				
(min) (feet)	(ft/	ft) (ft/sec)	(cfs)					
5.0				Direct Entry,				

#### Subcatchment 21S: Post Dev. Basin 5 to SCM



# Summary for Subcatchment 22S: Post Dev. Bypass 5

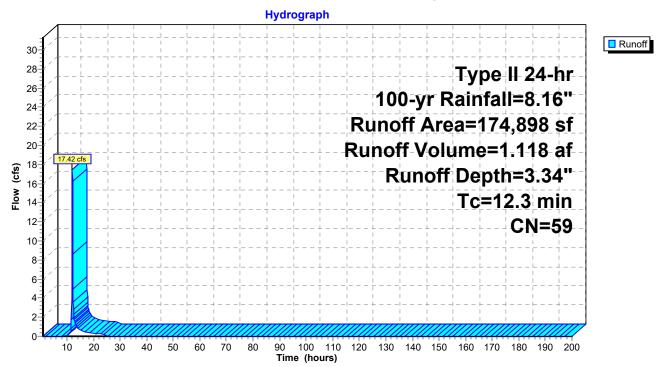
Runoff = 17.42 cfs @ 12.05 hrs, Volume= 1.118 af, Depth= 3.34"

Routed to Link 3L: POA 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

A	rea (sf)	CN	Description					
1	10,888	60	Woods, Fai	r, HSG B				
	51,602	48	Brush, Goo	d, HSG B				
	12,408	98	Paved park	ing, HSG B	3			
1	74,898	59	59 Weighted Average					
1	62,490		92.91% Per	vious Area	1			
	12,408		7.09% Impe	ervious Are	a			
Tc	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)				
12.3					Direct Entry,			

## Subcatchment 22S: Post Dev. Bypass 5



## Summary for Subcatchment 23S: Post Dev. Basin 6 to SCM

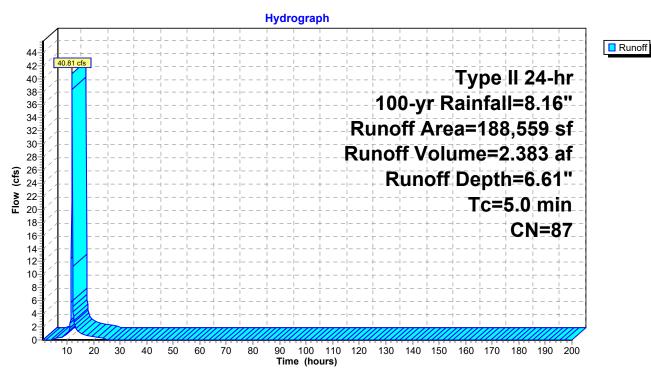
Runoff = 40.81 cfs @ 11.95 hrs, Volume= 2.383 af, Depth= 6.61"

Routed to Pond 4P: Wet Pond SCM 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

	Area (sf)	CN	Description							
,	54,466	98	Paved park	ing, HSG E						
	58,385	61	>75% Gras	s cover, Go	ood, HSG B					
	60,672	98	Roofs, HSG	βB						
	15,036	98	Water Surfa	ace, 0% imp	o, HSG B					
	188,559	87	87 Weighted Average							
	73,421		38.94% Per	vious Area						
	115,138		61.06% Imp	ervious Ar	ea					
-	c Length		•	Capacity	Description					
(mi	n) (feet)	(ft/f	t) (ft/sec)	(cfs)						
5	.0				Direct Entry,					

#### Subcatchment 23S: Post Dev. Basin 6 to SCM



## Summary for Subcatchment 24S: Post Dev. Bypass 6

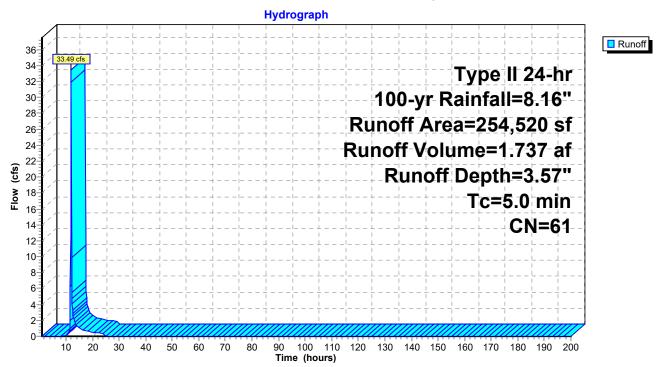
Runoff = 33.49 cfs @ 11.96 hrs, Volume= 1.737 af, Depth= 3.57"

Routed to Link 4L: POA 5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

Are	ea (sf)	CN	Description					
21	5,461	60	Woods, Fai	r, HSG B				
3	4,572	61	>75% Gras	s cover, Go	ood, HSG B			
	4,487	98	Paved park	ing, HSG E	В			
25	4,520	61	Weighted A	verage				
25	0,033		98.24% Per	vious Area	a			
	4,487		1.76% Impe	rvious Are	ea			
	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(t) (ft/sec) (cfs)					
5.0					Direct Entry,			

# Subcatchment 24S: Post Dev. Bypass 6



# Summary for Subcatchment 25S: Post Dev. Basin 7 to SCM

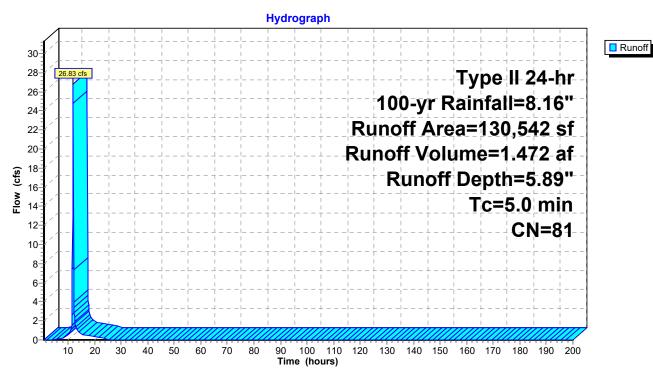
Runoff = 26.83 cfs @ 11.95 hrs, Volume= 1.472 af, Depth= 5.89"

Routed to Pond 5P: Wet Pond SCM 5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

Area (sf)	CN	Description						
50,624	98	Paved parking, HSG B						
44,621	48	Brush, Good, HSG B						
28,800	98	Roofs, HSG B						
6,497	98	Water Surface, 0% imp, HSG B						
130,542	81	Weighted Average						
51,118		39.16% Pervious Area						
79,424		60.84% Impervious Area						
Tc Length	n Slo <sub>l</sub>	pe Velocity Capacity Description						
(min) (feet)	) (ft/	/ft) (ft/sec) (cfs)						
5.0		Direct Entry						

#### Subcatchment 25S: Post Dev. Basin 7 to SCM



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#### Summary for Subcatchment 26S: Post Dev. Bypass 7

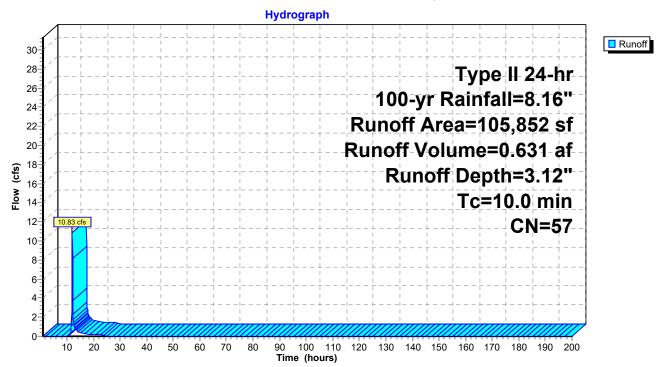
Runoff = 10.83 cfs @ 12.02 hrs, Volume= 0.631 af, Depth= 3.12"

Routed to Link 5L: POA 6

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

Ar	rea (sf)	CN	Description					
	39,944	55	Woods, Go	od, HSG B				
	52,486	48	Brush, Goo	d, HSG B				
	13,422	98	Paved park	ing, HSG E	3			
1	05,852	57	57 Weighted Average					
	92,430		87.32% Pervious Area					
	13,422		12.68% Imp	ervious Ar	rea			
Tc	Length	Slope	<ul><li>Velocity</li></ul>	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
10.0					Direct Entry,			

## Subcatchment 26S: Post Dev. Bypass 7



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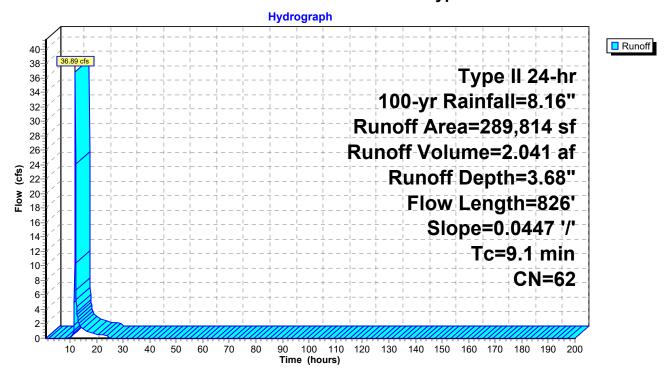
# Summary for Subcatchment 27S: Post Dev. Bypass 8

Runoff = 36.89 cfs @ 12.00 hrs, Volume= 2.041 af, Depth= 3.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Type II 24-hr 100-yr Rainfall=8.16"

A	rea (sf)	CN [	Description							
2	230,959	60 V	Woods, Fair, HSG B							
	53,439	69 5	50-75% Gra	ass cover, I	Fair, HSG B					
	5,416	98 F	Paved park	ing, HSG E	3					
	289,814	62 V	62 Weighted Average							
2	284,398	ç	8.13% Per	vious Area	a					
	5,416	1	.87% Impe	ervious Are	ea					
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
9.1	826	0.0447	1.51		Kirpich Method,					
					General overland flow k= 2.00					

#### Subcatchment 27S: Post Dev. Bypass 8



Prepared by Thomas & Hutton

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# Summary for Pond 1P: Sand Filter -SCM 1

Inflow Area = 7.523 ac, 77.83% Impervious, Inflow Depth = 6.84" for 100-yr event

Inflow = 72.49 cfs @ 11.95 hrs, Volume= 4.290 af

Outflow = 39.51 cfs @ 12.09 hrs, Volume= 4.290 af, Atten= 45%, Lag= 8.4 min

Primary = 39.51 cfs @ 12.09 hrs, Volume= 4.290 af

Routed to Link 1L: POA 1

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Peak Elev= 533.15' @ 12.07 hrs Surf.Area= 6,825 sf Storage= 62,453 cf

Plug-Flow detention time= 82.7 min calculated for 4.288 af (100% of inflow)

Center-of-Mass det. time= 83.3 min ( 857.4 - 774.1 )

Volume	Inve	rt Avail.Sto	rage Storage	Description			
#1	524.0	0' 68,25	50 cf Custom	n Stage Data (Pi	rismatic)Listed below (Recalc) x 65		
Elevatio (fee	t)	Surf.Area (sq-ft) 105	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
534.0	-	105	1,050	1,050			
Device	Routing	Invert	Outlet Device	es			
#1	Primary	524.00'	36.0" Round Culvert L= 85.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 524.00' / 523.00' S= 0.0118 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 7.07 sf				
#2	Device 1	524.00'	8.0" Vert. Dr	awdown C= 0.	600 Limited to weir flow at low heads		
#3	Device 1	528.90'		<b>0" H Vert. Main</b> eir flow at low hea	Orifice C= 0.600		
#4	Device 1	533.00'		verflow 2 End 0			

Primary OutFlow Max=37.42 cfs @ 12.09 hrs HW=533.06' (Free Discharge)

**-1=Culvert** (Passes 37.42 cfs of 93.56 cfs potential flow)

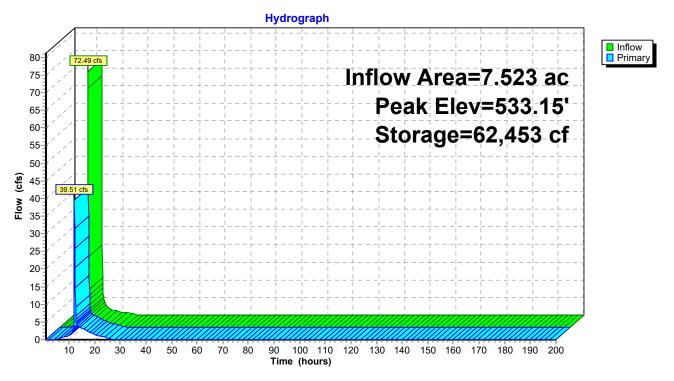
**2=Drawdown** (Orifice Controls 4.96 cfs @ 14.22 fps)

**—3=Main Orifice** (Orifice Controls 31.37 cfs @ 9.41 fps)

**-4=Overflow** (Weir Controls 1.08 cfs @ 0.77 fps)

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Pond 1P: Sand Filter -SCM 1



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## **Summary for Pond 2P: Wet Pond SCM 2**

Inflow Area = 3.812 ac, 71.42% Impervious, Inflow Depth = 6.49" for 100-yr event

Inflow = 35.51 cfs @ 11.95 hrs, Volume= 2.061 af

Outflow = 14.55 cfs @ 12.12 hrs, Volume= 2.061 af, Atten= 59%, Lag= 10.0 min

Primary = 14.55 cfs @ 12.12 hrs, Volume= 2.061 af

Routed to Link 2L: POA 2

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

Starting Elev= 526.00' Surf.Area= 11,086 sf Storage= 32,033 cf

Peak Elev= 528.87' @ 12.12 hrs Surf.Area= 16,871 sf Storage= 72,072 cf (40,040 cf above start)

Plug-Flow detention time= 387.1 min calculated for 1.325 af (64% of inflow)

Center-of-Mass det. time= 158.5 min ( 941.4 - 782.9 )

Volume	Inve	rt Avail.Sto	rage Storage	e Description	
#1	522.0	0' 92,42	29 cf Custon	n Stage Data (Pri	smatic)Listed below (Recalc)
Classatia		Court Anna	In a Ctava	Cura Stara	
Elevation (fee		Surf.Area	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
		(sq-ft)			
522.0		5,067	0	0	
523.0	00	7,010	6,039	6,039	
524.0	00	7,727	7,369	13,407	
525.0	00	9,219	8,473	21,880	
526.0	00	11,086	10,153	32,033	
527.0	00	13,027	12,057	44,089	
528.0		15,043	14,035	58,124	
529.0		17,134	16,089	74,213	
530.0		19,299	18,217	92,429	
000.		.0,200	.0,2	02, .20	
Device	Routing	Invert	Outlet Device	es	
#1	Primary	526.00'	36.0" Roun	d Culvert	
	, <b>,</b>				eadwall, Ke= 0.500
					25.55' S= 0.0100 '/' Cc= 0.900
					ht & clean, Flow Area= 7.07 sf
#2	Davisa 1	E06 00!			00 Limited to weir flow at low heads
#2	Device 1	526.00'			
#3	Device 1	527.40'			ow Orifice X 2.00 C= 0.600
				eir flow at low head	
#4	Device 1	528.60'	48.0" x 48.0'	" Horiz. Overflow	C= 0.600
			Limited to we	eir flow at low head	ls

Primary OutFlow Max=13.71 cfs @ 12.12 hrs HW=528.85' (Free Discharge)

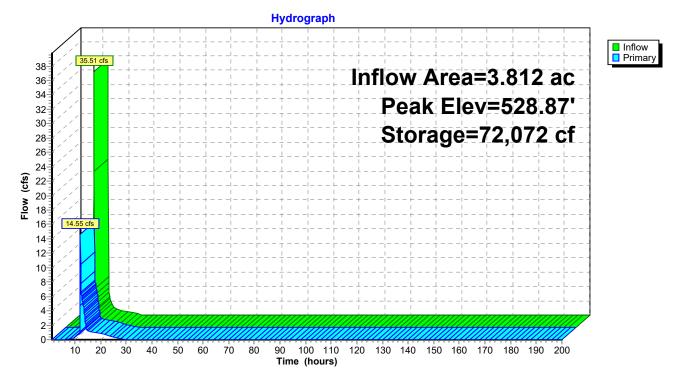
**1=Culvert** (Passes 13.71 cfs of 36.29 cfs potential flow)

**2=Drawdown** (Orifice Controls 1.78 cfs @ 7.74 fps)

-3=Peakflow Orifice (Orifice Controls 5.32 cfs @ 5.63 fps)

**-4=Overflow** (Weir Controls 6.61 cfs @ 1.64 fps)

#### Pond 2P: Wet Pond SCM 2



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## Summary for Pond 3P: Wet Pond SCM 3

Inflow Area = 2.579 ac, 76.09% Impervious, Inflow Depth = 6.72" for 100-yr event

Inflow = 24.59 cfs @ 11.95 hrs, Volume= 1.445 af

Outflow = 13.39 cfs @ 12.10 hrs, Volume= 1.445 af, Atten= 46%, Lag= 8.9 min

Primary = 13.39 cfs @ 12.10 hrs, Volume= 1.445 af

Routed to Link 3L: POA 4

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

Starting Elev= 534.00' Surf.Area= 3,130 sf Storage= 15,183 cf

Peak Elev= 537.75' @ 12.08 hrs Surf.Area= 8,660 sf Storage= 39,211 cf (24,028 cf above start)

Plug-Flow detention time= 328.0 min calculated for 1.096 af (76% of inflow)

Center-of-Mass det. time= 154.3 min (931.5 - 777.2)

Volume	Inv	ert Avail	.Storage	Storage	e Description	
#1	530.0	00' 5	52,862 cf	Custon	n Stage Data (P	rismatic)Listed below (Recalc)
<b>-</b> 14:		O	1	01	0	
Elevation		Surf.Area		Store	Cum.Store	
(fee	et)	(sq-ft)	(cubi	c-feet)	(cubic-feet)	
530.0	00	2,212		0	0	
531.0	00	3,200		2,706	2,706	
532.0	00	4,144		3,672	6,378	
533.0	00	5,168		4,656	11,034	
534.0	00	3,130		4,149	15,183	
535.0	00	6,262		4,696	19,879	
536.0	00	8,640		7,451	27,330	
537.0	00	4,960		6,800	34,130	
538.0	00	9,920		7,440	41,570	
539.0	00	12,664	•	11,292	52,862	
Device	Routing	lny	ert Outl	et Device	26	
#1	Primary	534.			d Culvert	
					, I	headwall, Ke= 0.500
						533.56' S= 0.0100'/' Cc= 0.900
						ds & connections, Flow Area= 4.91 sf
#2	Device 1					600 Limited to weir flow at low heads
#3	Device 1	536.		_		Flow X 2.00 C= 0.600
					eir flow at low hea	
#4	Device 1	537.	60' <b>48.0</b>	" x 48.0'	" Horiz. Overflo	<b>w</b> C= 0.600
			Limi	ted to we	eir flow at low hea	ads

Primary OutFlow Max=13.17 cfs @ 12.10 hrs HW=537.73' (Free Discharge)

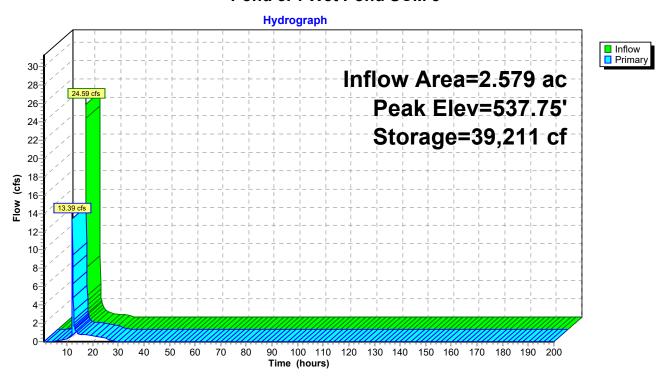
**-1=Culvert** (Passes 13.17 cfs of 36.89 cfs potential flow)

-2=Drawdown (Orifice Controls 1.00 cfs @ 9.07 fps)

-3=Peak Flow (Orifice Controls 9.63 cfs @ 5.50 fps)

**-4=Overflow** (Weir Controls 2.54 cfs @ 1.19 fps)

Pond 3P: Wet Pond SCM 3



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## Summary for Pond 4P: Wet Pond SCM 4

Inflow Area = 4.329 ac, 61.06% Impervious, Inflow Depth = 6.61" for 100-yr event

Inflow 40.81 cfs @ 11.95 hrs, Volume= 2.383 af

Outflow 22.12 cfs @ 12.10 hrs, Volume= 2.058 af, Atten= 46%, Lag= 9.1 min

2.058 af Primary 22.12 cfs @ 12.10 hrs, Volume=

Routed to Link 4L: POA 5

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs Peak Elev= 526.97' @ 12.10 hrs Surf.Area= 13,511 sf Storage= 52,154 cf

Plug-Flow detention time= 414.5 min calculated for 2.057 af (86% of inflow)

Center-of-Mass det. time= 353.6 min (1,133.7 - 780.1)

Volume	Inve	ert Avail.St	orage Storage	Storage Description		
#1	519.0	00' 67,2	235 cf Custon	n Stage Data (Pı	rismatic)Listed below (Recalc)	
Elevatio (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
519.0		1,472	0	0		
520.0	0	2,352	1,912	1,912		
521.0	0	3,406	2,879	4,791		
522.0	0	4,636	4,021	8,812		
523.0	0	6,046	5,341	14,153		
524.0	0	7,648	6,847	21,000		
525.0	0	9,474	8,561	29,561		
526.0	0	11,446	10,460	40,021		
527.0	0	13,570	12,508	52,529		
528.0	0	15,842	14,706	67,235		
Device	Routing	Invert	Outlet Device	es		
#1	Primary	523.00	36.0" Roun			

Device	Routing	invert	Outlet Devices
#1	Primary	523.00'	36.0" Round Culvert
	-		L= 45.0' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 523.00' / 521.50' S= 0.0333 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 7.07 sf
#2	Device 1	523.00'	<b>2.5" Vert. Drawdown</b> C= 0.600 Limited to weir flow at low heads
#3	Device 1	525.10'	<b>36.0" W x 3.0" H Vert. Peakflow</b> C= 0.600
			Limited to weir flow at low heads
#4	Device 1	526.50'	<b>48.0" x 48.0" Horiz. Overflow</b> C= 0.600
			Limited to weir flow at low heads

**Primary OutFlow** Max=21.98 cfs @ 12.10 hrs HW=526.97' (Free Discharge)

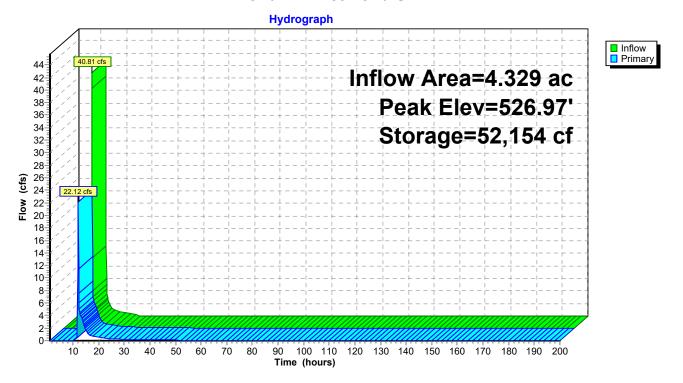
**-1=Culvert** (Passes 21.98 cfs of 53.50 cfs potential flow)

**2=Drawdown** (Orifice Controls 0.32 cfs @ 9.47 fps)

**-3=Peakflow** (Orifice Controls 4.77 cfs @ 6.36 fps)

**-4=Overflow** (Weir Controls 16.89 cfs @ 2.24 fps)

#### Pond 4P: Wet Pond SCM 4



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#### **Summary for Pond 5P: Wet Pond SCM 5**

Inflow Area = 2.997 ac, 60.84% Impervious, Inflow Depth = 5.89" for 100-yr event

Inflow = 26.83 cfs @ 11.95 hrs, Volume= 1.472 af

Outflow = 15.02 cfs @ 12.10 hrs, Volume= 1.472 af, Atten= 44%, Lag= 8.7 min

Primary = 15.02 cfs @ 12.10 hrs, Volume= 1.472 af

Routed to Link 5L: POA 6

Routing by Stor-Ind method, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

Starting Elev= 513.00' Surf.Area= 4,340 sf Storage= 13,131 cf

Peak Elev= 516.90' @ 12.08 hrs Surf.Area= 7,318 sf Storage= 37,058 cf (23,928 cf above start)

Plug-Flow detention time= 306.8 min calculated for 1.170 af (80% of inflow)

Center-of-Mass det. time= 156.6 min ( 952.3 - 795.7 )

Volume	Inve	ert Avail.Sto	rage Storage	Storage Description			
#1	509.00' 45,475 cf <b>Cust</b>		75 cf Custom	m Stage Data (Prismatic)Listed below (Recalc)			
- ·		0 ( )	. 01	0 01			
Elevation		Surf.Area	Inc.Store	Cum.Store			
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)			
509.0	00	2,355	0	0			
510.0	00	2,789	2,572	2,572			
511.00		3,517	3,153	5,725			
512.00		3,477	3,497	9,222			
513.0	00	4,340	3,909	13,131			
514.0	00	5,752	5,046	18,177			
515.0	00	6,271	6,012	24,188			
516.0	00	6,813	6,542	30,730			
517.0	00	7,377	7,095	37,825			
518.0	00	7,922	7,650	45,475			
Device	Routing	Invert	Outlet Device	es			
#1	Primary	513.00'	36.0" Round Culvert				
			L= 79.0' RCP, square edge headwall, Ke= 0.500				
			Inlet / Outlet I	Invert= 513.00' /	512.00' S= 0.0127 '/' Cc= 0.900		
n= 0.013 Concrete pipe, bends & connections, Flo					ds & connections, Flow Area= 7.07 sf		
#2	Device 1	513.00'					
#3	Device 1	515.10'					
			Limited to we	ir flow at low hea	ads		
#4	Device 1	516.70'	48.0" x 48.0"	' Horiz. Overflo	<b>w</b> C= 0.600		
			Limited to we	ir flow at low hea	ads		

Primary OutFlow Max=14.69 cfs @ 12.10 hrs HW=516.88' (Free Discharge)

**-1=Culvert** (Passes 14.69 cfs of 52.53 cfs potential flow)

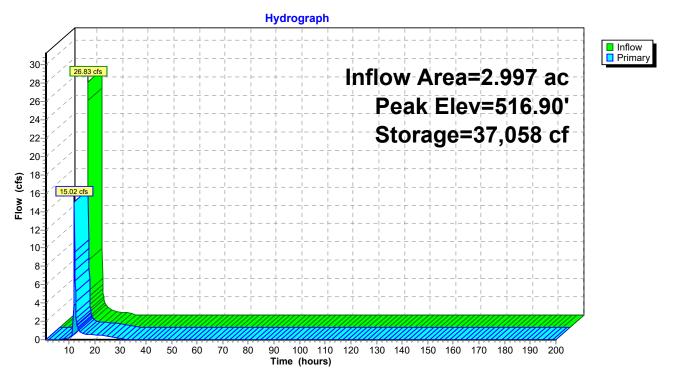
-2=Drawdown (Orifice Controls 0.81 cfs @ 9.28 fps)

-3=Peakflow Orifice (Orifice Controls 9.81 cfs @ 6.20 fps)

**-4=Overflow** (Weir Controls 4.07 cfs @ 1.40 fps)

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#### Pond 5P: Wet Pond SCM 5



# Summary for Link 1L: POA 1

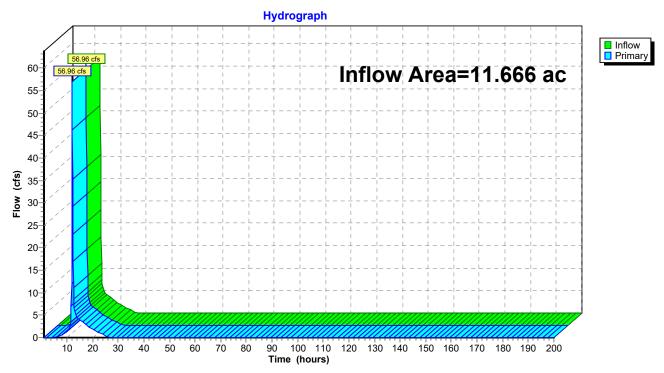
Inflow Area = 11.666 ac, 51.60% Impervious, Inflow Depth = 5.64" for 100-yr event

Inflow = 56.96 cfs @ 12.01 hrs, Volume= 5.483 af

Primary = 56.96 cfs @ 12.01 hrs, Volume= 5.483 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

#### Link 1L: POA 1



# **Summary for Link 2L: POA 2**

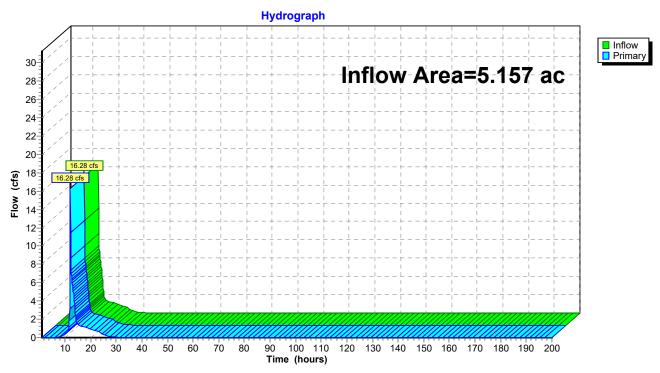
Inflow Area = 5.157 ac, 52.79% Impervious, Inflow Depth = 5.52" for 100-yr event

Inflow = 16.28 cfs @ 12.09 hrs, Volume= 2.373 af

Primary = 16.28 cfs @ 12.09 hrs, Volume= 2.373 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

#### Link 2L: POA 2



# Summary for Link 3L: POA 4

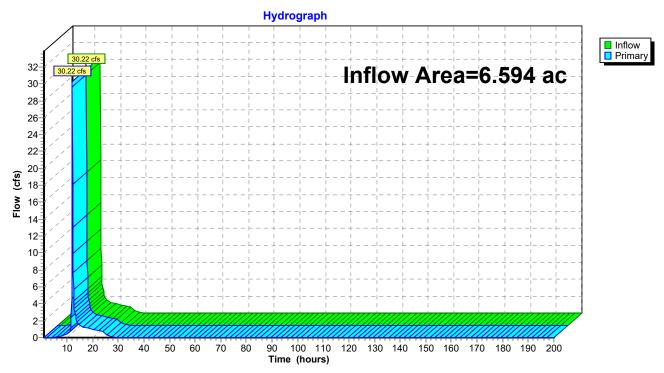
Inflow Area = 6.594 ac, 34.08% Impervious, Inflow Depth = 4.66" for 100-yr event

Inflow = 30.22 cfs @ 12.07 hrs, Volume= 2.563 af

Primary = 30.22 cfs @ 12.07 hrs, Volume= 2.563 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

#### Link 3L: POA 4



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# Summary for Link 4L: POA 5

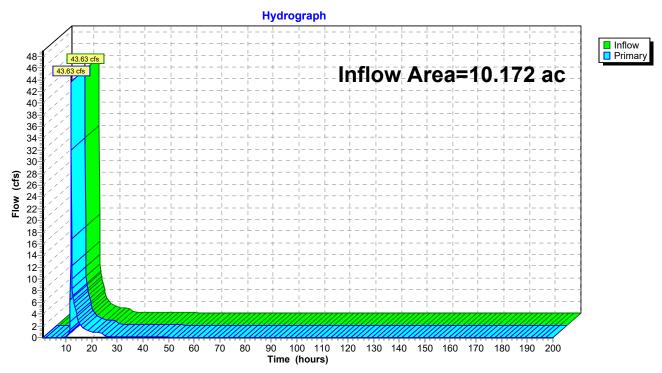
Inflow Area = 10.172 ac, 27.00% Impervious, Inflow Depth = 4.48" for 100-yr event

Inflow = 43.63 cfs @ 12.00 hrs, Volume= 3.795 af

Primary = 43.63 cfs @ 12.00 hrs, Volume= 3.795 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

#### Link 4L: POA 5



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# **Summary for Link 5L: POA 6**

Inflow Area = 5.427 ac, 39.28% Impervious, Inflow Depth = 4.65" for 100-yr event

Inflow = 23.82 cfs @ 12.07 hrs, Volume= 2.103 af

Primary = 23.82 cfs @ 12.07 hrs, Volume= 2.103 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-200.00 hrs, dt= 0.10 hrs

#### Link 5L: POA 6

