DEVELOPMENT AGREEMENT

BY AND BETWEEN THE UNIVERSITY OF NORTH CAROLINA AT CHAPEL HILL, THE BOARD OF TRUSTEES OF THE ENDOWMENT FUND OF THE UNIVERSITY OF NORTH CAROLINA AT CHAPEL HILL, AND THE TOWN OF CHAPEL HILL

AND

THE TOWN OF CHAPEL HILL, NORTH CAROLINA

Effective Date: JUNE 27, 2018

Orange County Parcel Identifier Numbers: 978-924-7373 978-903-3163

The Municipal Services Center - Campus Development Agreement

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STATE OF NORTH CAROLINA

COUNTY OF ORANGE

DEVELOPMENT AGREEMENT

This Development Agreement (hereinafter the "Agreement") is made and entered into as of the 27th day of June, 2018 by and among The Town of Chapel Hill, a municipal corporation organized and existing under the laws of the State of North Carolina, and The University of North Carolina at Chapel Hill and the Board of Trustees of the Endowment Fund of The University of North Carolina at Chapel Hill, (collectively the "Developers") and the Town of Chapel Hill (the Municipality).

ARTICLE 1. STATUTORY FRAMEWORK

- 1.1 G.S. 160A-400.20(b) and 160A-400.22 expressly authorize local governments and agencies to enter into development agreements with developers pursuant to the procedures and requirements of G.S. 160A-400.20 through 160A-400.32, which procedures and requirements include approval of the development agreement by the governing body of the local government by ordinance after a duly noticed public hearing.
- 1.2 In addition to any force of law conferred upon this Agreement by North Carolina law related to local governments, the terms of this Agreement are also contractual in nature, are a significant inducement and consideration to enter into this Agreement, and may be enforced as contractual terms.

ARTICLE 2. DEFINITIONS

In the construction of this Agreement and its incorporated EXHIBITs, the following capitalized terms shall have the respective meanings set forth below wherever they appear in this Agreement. Except as otherwise provided in this Agreement, terms used in the relevant portions of the North Carolina General Statutes or the Town of Chapel Hill Land Use Management Ordinance ("LUMO") shall have the same meanings as employed in those statutes and ordinances.

- 2.1 Agreement. The word "Agreement" shall mean this Development Agreement.
- 2.2 *Annual Report*. The words "Annual Report" shall mean the annual report required by the Town pursuant to Section 5.25 of this Agreement.
- 2.3 *Council*. The word "Council" or "Town Council," shall mean the legislative body of the Town of Chapel Hill, which together with the mayor of the Town shall constitute the governing body of the town.
- 2.4 Development or Develop. The words "Development" or "Develop" shall mean any activity on the Property involving, requiring, or consisting of: (i) the construction of a new building, and the construction or installation of other structures, roads, greenways or paved trails, sidewalks, parking lots, or utility infrastructures; (ii) the clearing or alteration of land as an adjunct of such construction; or (iii) the expansion of an existing building.
- 2.5 *Developer or Developers*. The words "Developer" or "Developer" shall mean the Town of Chapel Hill, the University of North Carolina at Chapel Hill, and the Endowment of the University of North

- Carolina at Chapel Hill acting in the role of a developer and/or property owner.
- 2.6 Effective Date. The word "Effective Date" shall mean the effective date of this Agreement, which is June 27, 2018.
- 2.7 *LUMO*. The term "LUMO" shall mean the Land Use Management Ordinance of the Town of Chapel Hill in effect as of the Effective Date.
- 2.8 *Manager*. The words "Manager" or "Town Manager" shall mean the Town Manager of the Town of Chapel Hill or his/her designee and anyone acting pursuant to authority vested by state statute or local ordinance to issue local permits for Development on behalf of the Town of Chapel Hill.
- 2.9 *Municipality*. "Municipality" shall mean and refer to the Town of Chapel Hill, a municipal corporation organized under the laws of the State of North Carolina when acting in its capacity as the governing municipality for this Development Agreement, site plan approval, special use permits, building permits or other required approvals under the laws, rules and regulations adopted by the Town of Chapel Hill or other governmental authority.
- 2.10 *Nontechnical and technical words*. Words and phrases shall be construed according to the common and approved usage of the language, but technical words and phrases and such others as may have acquired a peculiar and appropriate meaning in law shall be construed and understood according to such meaning.
- 2.11 *Party*. The word "Party" shall mean one or more of the "Parties" as may be understood based on the specific context of the reference.
- 2.12 *Parties*. The word "Parties" shall mean the Town of Chapel Hill, the University of North Carolina at Chapel Hill, and the University and the Municipality.
- 2.13 *Periodic Review*. The term "Periodic Review" shall have the meaning ascribed in Section 4.12 of this Agreement.
- 2.14 *Property*. The term "Property" shall mean that certain parcel of land, located on Estes Drive as depicted in EXHIBIT A, attached hereto. The Property also includes any public rights-of-way that may be located within the boundaries of EXHIBIT A.
- 2.15 Registry. The term "Registry" shall mean the Orange County Register of Deeds.
- 2.16 Site Development Permit, or SDP. The words "Site Development Permit" shall mean the permit issued by the Town Manager authorizing Development or portions thereof in accordance with this Agreement. A Site Development Permit required by this Agreement shall be in lieu of any Zoning Compliance Permit that might otherwise be required by Section 4.9 of the LUMO.
- 2.17 State. The term "State" shall mean the State of North Carolina.
- 2.18 *Town*. The word "Town" shall mean and refer to the Town of Chapel Hill, a North Carolina municipal corporation or its successor, solely in its capacity as a *participant* and not as the governing municipality.
- 2.19 Town Attorney. The term "Town Attorney" shall mean the attorney for the Municipality.

- 2.20 *Town Regulations*. The term "Town Regulations" shall have the meaning as described in Section 4.4.
- 2.21 *University*. The words "the University" shall mean the University of North Carolina at Chapel Hill and The Board of Trustees of the Endowment Fund of The University of North Carolina at Chapel Hill.
- 2.22 Zoning Compliance Permit. The term "Zoning Compliance Permit" or "ZCP" shall mean an administrative permit issued by the town manager authorizing the recipient to make use of existing buildings in accord with the requirements of the Land Use Management Ordinance as of the effective date of this Agreement.

ARTICLE 3. RECITALS

- 3.1 The University of North Carolina at Chapel Hill through the State of North Carolina, and The Board of Trustees of the Endowment Fund of The University of North Carolina at Chapel Hill are the owners of that certain property situated on the southern side of Estes Drive as shown on EXHIBIT A and described on EXHIBIT G. The Property is located within the corporate and regulatory limits of the Town of Chapel Hill.
- 3.2 On September 27, 2017 the Town Council directed the Town Manager and Town Attorney to negotiate a development agreement for the redevelopment of the *Property*.
- 3.3 On May 10, 2017 the Town created a web page on the Town of Chapel Hill website to provide information about the proposal.
- 3.4 On March 6, 2017 and September 7, 2017, the Town hosted public information meetings to learn about the proposal.
- 3.5 On November 30 and December 13 of 2017, and January 8, February 5, February 15, March 1, and April 12, 2018, the Town hosted public workshops to receive public input regarding specific topics about the project. The topics for these meetings were developed with interested community members following the September 7, 2017 public information meeting.
- 3.6 Residents of the Elkin Hills neighborhood participated in community meetings and developed a document consisting of Guiding Principles for the Development Agreement. The principles have served as design drivers for the site layout, and have been broadly incorporated into the program for development. Specific incorporation of portions of the Guiding Principles are in the following sections of this Agreement: 5.4, 5.5, 5.7, 5.8, 5.10, 5.17, 5.18, and 5.22.
- 3.7 An impact analysis on the value of contiguous properties was performed by a NC State-Certified General Real Estate Appraiser. This report was provided to the Town and interested residents. The report is provided as an Exhibit to the Agreement.
- 3.8 The following advisory boards reviewed and provided their feedback on the development agreement:
 - The Community Design Commission and the Transportation and Connectivity Advisory Board both met on March 27, 2018; the Environmental Stewardship Advisory Board met on April 10, 2018; and the Planning Commission met on April 17, 2018.
 - The Parks, Greenways, and Recreation Commission reviewed and provided feedback on April 17, 2018, and the Stormwater Management Utility Advisory Board reviewed and provided feedback on April 24, 2018.
- 3.9 The purpose of this Agreement is to facilitate the Development of the Property in a way that best realizes the public benefits to the Town of Chapel Hill and the University of North Carolina at Chapel Hill. The Development of the Property requires a major investment by the first builder, presumed to be the Town, in facilities, substantial front-end investment in infrastructure improvements, and substantial commitments of the resources to achieve the benefits of the Development for the Town, and the University. The Town will be unable to make and realize the benefits from such commitments without the assurances as provided by this Agreement.

- 3.10 The general benefits to be received by the Town of Chapel Hill from the implementation of the Development include, without limitation:
 - a) The opportunity to build a municipal services center in a desirable location for serving the needs of the entire community,
 - b) Minimization of adverse off-site impacts, and incorporation of sustainability principles in the design and implementation of the Development of the Property;
 - c) Provision of a plan for addressing the transportation needs of the Development of the Property, including commitments of sidewalks, a Greenway connection, and road improvements; and
 - d) Assurance that the long term Development of the Property will be undertaken in a manner that is compatible with the interests of the Town and the University.
- 3.11 The general benefits to be received by the University from the implementation of the Development include without limitation:
 - a) Obtaining sufficient certainty, timeliness, and predictability in the Town's development review and approval process to justify the substantial up- front capital investment by the Town on University-owned property;
 - b) Integration of site plans, and uses, architecture, site engineering, landscape architecture, and mitigation measures over the entire Property;
 - c) Flexibility and alternatives to Town ordinances, standards, policies and guidelines to achieve the Development; and
 - d) Participation from the Town to achieve the public benefits necessary for the Development including stormwater management infrastructure.
- 3.12 In exchange for providing these benefits mutual benefits, both parties receive the assurance that they may proceed with the Development of the Property in accordance with any and all existing Town of Chapel Hill development regulations and conditions of approval of the Municipality as they exist on the Effective Date, subject to the terms, conditions, and exceptions contained herein and subject to periodic potential amendments to this Agreement made in accordance with this Agreement.
- 3.13 After careful review and deliberation, the Parties have determined that the latitude afforded the proposed Development of the Property and the certainty, timeliness, and predictability regarding Town development approval afforded by this Agreement provide important benefits and Development justifies the provision of the mitigation measures specified in Article 5 of this Agreement, which the Parties freely and with full knowledge and consent agree to provide.
- 3.14 The terms and conditions of this Agreement have undergone review by the Town's staff and the Town Council and have been found to be fair, just, and reasonable. After careful review and deliberation, the Town Council has determined and concluded that the Agreement meets the goals and needs of the Town and the University and complies with all statutory requirements.
- 3.15 The Town of Chapel Hill, by electing to enter into this Agreement in accordance with statutory procedures, acknowledges that the obligations of the Town of Chapel Hill shall survive beyond the term or terms of the present Town Council and that such action will serve to bind the Town of Chapel Hill and future councils to the obligations thereby undertaken. By approving this Agreement, the Municipality has elected to exercise certain governmental powers at the time of entering into this Agreement rather than deferring its actions to some undetermined future date.
- 3.16 Based on the foregoing considerations, the Parties desire to enter into this Agreement for the

purposes of:

- a) agreeing upon the plan, design, and density of Development on the Property and the types of uses thereon, and promoting sustainability, reflecting consideration of economic issues;
- b) coordinating the construction and provision of infrastructure that will serve the above-described Development of the Property and the community at large;
- c) confirming the dedication and/or provision of the public amenities described herein; and
- d) providing assurances to the Parties that they may proceed with the Development of the Property in accordance with the terms of this Agreement without encountering future changes in ordinances, regulations or policies that would affect their ability to Develop the Property under the terms of this Agreement;
- e) providing certainty that the Parties can obtain permits necessary for the Development pursuant to a non-discretionary, predictable and expeditious process.
- 3.17 Pursuant to G.S. 160A-400.24, the Town Council conducted a public hearing on May 9, 2018 to consider the approval of this Agreement. The notice of public hearing specified, among other things, the location of the Property subject to this Agreement, and the development uses proposed on the Property. The initial draft of this Agreement was posted on the Town website on March 28, 2018 and was available for public inspection at Town offices and online at that time. Updates to the draft of this Agreement were posted for the May 9, 2018 public hearing.
- 3.18 On June 27, 2018 the Town Council considered and approved this Agreement and authorized the Town's execution of the same as a Party and as the Municipality. The approval of this Agreement constitutes a legislative act of the Municipality's Town Council.

ARTICLE 4. TERMS

NOW, THEREFORE, based upon the terms and conditions set forth herein and in consideration of the mutual promises and assurances provided herein, the Parties do hereby agree as follows:

- 4.1. <u>Recitals.</u> The Parties agree the foregoing Recitals in Article 3 are true and correct and are incorporated herein by reference.
- 4.2. <u>Property Subject to Agreement.</u> The real property subject to this Agreement includes all of the Property.
- 4.3. Zoning of the Property. On June 27, 2018, the Town Council rezoned the Property to University-1 (U-1) which is located within the Town's planning and zoning jurisdiction.
- 4.4. Term of Agreement: The term of this Agreement shall commence upon the Effective Date, and it shall expire twenty (20) years thereafter unless sooner terminated by the mutual consent of the parties hereto or their successors in interest or is otherwise terminated pursuant to the terms of this Agreement. This twenty- (20-) year term has been established as the maximum lease length currently permitted by the University. The Town finds that a term of twenty (20) years is reasonably necessary to assure the Town of the realization of the public benefits from the Project. Expiration of the twenty- (20-) year term shall not terminate mutually agreed to obligations and commitments included within this Agreement that are expressly specified to extend beyond the term of the Agreement. The term of this Agreement may be extended at any time by mutual agreement between the Parties of this agreement.

- 4.5. Law in Effect at Time of the Agreement Governs the Development; Vested Rights. Except as provided in G.S. 160A-400.26 and G.S. 160A-400.29(b), the Municipality may not apply subsequently adopted ordinances or development policies to the Property during the term of this Agreement without the written consent of the Parties. The consent of Parcel Owners shall not be required unless the Parcel owned by the Parcel Owner is directly and adversely affected by such subsequently adopted ordinances or development policies, in which case the affected Parcel Owner must also provide written consent under this Section. Accordingly, during the term of this Agreement the Parties shall have a vested right to Develop the Property in accordance with the terms of Article 5 of this Agreement, the terms of the LUMO, and any applicable laws and regulations, all of the foregoing as they exist as of the Effective Date. This Agreement does not abrogate any rights that may vest pursuant to statutory or common law or otherwise in the absence of this Agreement. The Participants agree, intend, and understand that the obligations imposed by this Agreement are only such as are consistent with state and federal law. In accordance with G.S. 160A-400.26(c), in the event state or federal law is changed after the Effective Date and the change prevents or precludes compliance with one or more provisions of this Agreement, the Town may modify the affected provisions with the consent of the Parties, upon a finding that the change in state or federal law has a fundamental effect on the Agreement, by ordinance after notice and a hearing and upon such modification the Town shall record the modification in the Registry. If the Parties fail to consent, this Agreement shall be terminated (with any Parties retaining any rights with respect to any Site Development Permit obtained prior to termination) and said termination will be recorded in the Registry.
- 4.6 <u>Development of the Property Special Uses.</u> In the event a proposed Development does not comply with this Agreement and applicable Town Regulations and is not considered a Minor Modification (as hereinafter defined) to this Agreement, the applicant, in the applicant's sole discretion, may:
 - a) apply to the Town Council for an amendment to this Agreement as a Major Modification (as hereinafter defined), or
 - b) apply for a special use permit under Section 4.5 of the LUMO to permit the proposed Development.

4.7 Local Development Permits.

- a) In accordance with G.S. 160A-400.25(a)(6), the local development permits approved or needed to be approved for the Development shall include the following: (1) Individual Site Development Permits; (2) Building and other applicable construction permits; and (3) Engineering Construction Permits, Street Cut Permits, Street Closure Permits for work in public rights of way. Any such approvals and permits shall be consistent with the requirements of Article 5 of this Agreement. The failure of this Agreement to address a particular permit, condition, term or restriction does not relieve the Parties of the necessity of complying with such permitting requirements, conditions, terms or restrictions, except as may be limited or otherwise provided in this Agreement.
- b) Application may be made for more than one type of local permit at a time, and such permit applications shall be reviewed concurrently by the Town to the extent possible. Additionally, the Municipality shall exercise reasonable diligence to expedite the processing of the required permit and approval applications for the Development of the Property. The Parties shall in a timely manner provide the Town with all documents, applications, plans, and other information necessary for the Municipality to carry out its obligations hereunder.

4.8 <u>Development of the Property – Site Development Permit.</u>

a) Permit Required. A Site Development Permit shall be required prior to the commencement of any Development or construction work related to Development except as otherwise indicated in this Agreement. The Site Development Permit required by this section shall be in lieu of any Zoning Compliance Permit that might otherwise be required by Section 4.9 of the LUMO. The Parties and/or Parcel Owners intending to undertake Development of the Property (the "Applicant") shall apply for a Site Development Permit by filing the application provided for in EXHIBIT L.

b) Schedule of Review:

- i. Prior to submitting an application for a Site Development Permit, the Party shall meet with the Municipality to review the proposed Development.
- ii. Upon submittal of a Site Development Permit, Town projects shall be scheduled for Community Design Commission review, and University projects shall be scheduled for Design Review Committee review. The Community Design Commission shall have the option to review and provide comment on Town projects prior to a Site Development Permit approval.
- iii. Within five (5) working days following submission of an application for a Site Development Permit, the Municipality shall provide the applicant with a completeness determination.
- iv. The Municipality shall review the application for compliance with this Agreement and the applicable Town regulations that are not expressly superseded by this Agreement.
- v. Within twenty (20) working days following the date on which an application is deemed complete, the Town shall provide written comments to the Party regarding whether the submitted application is in compliance with this Agreement and applicable Town regulations or whether the submitted application requires revision.
- vi. If the application is not in compliance, the Party shall submit a revised application to the Town. Upon submitting a revised application, the Town shall process the revised application in the manner described in Section 4.8, including the twenty- (20-) day time frame set forth in subsection 5) and such process shall be repeated until the application is in compliance with this Agreement and applicable Town regulations. Throughout this review period, the Party and the Municipality will strive to review and respond to information in an expeditious manner and provide updates to one another every ten (10) working days to ensure open communication and accountability.
- vii. The Town Manager shall approve the application upon finding it complies with and does not violate any term of this Agreement and the applicable Town regulations, and shall deny the application upon finding it does not comply with the terms of this Agreement and the applicable Town regulations.
- viii. If the application is approved, the Town shall issue the Applicant a Compliance with Development Agreement Approval Letter.
- ix. If the application is denied, the Town Manager shall specify the grounds for finding that it is inconsistent or in violation and refer the Party to the special use permit process described in Section 4.5 of the LUMO. Alternatively, the applicant may modify the Site Development Permit application or apply for Major Modification to this Agreement.
- x. Notwithstanding anything to the contrary, a one-time change in floor area of one thousand (1,000) square feet or fewer to Development previously approved with a SDP shall not require modification or approval of another SDP. Such changes shall be reported

to the Municipality in the Annual Report.

- c) Construction Management Plan. Each Site Development Permit application shall include a construction management plan. The construction management plan shall, at a minimum:
 - i. Show how construction vehicle traffic will be managed and where the construction vehicle routes will be located.
 - ii. Identify any impacts to bicycle, pedestrian, or Transit routes and facilities and indicate how they will be mitigated.
 - iii. Show parking areas for on-site construction workers including plans to prohibit parking in residential neighborhoods.
 - iv. Identify construction staging and material storage areas.
 - v. Identify construction trailers and other associated temporary construction management structures.
 - vi. Indicate how the project construction will comply with the Town's Noise Ordinance.
 - vii. Propose times and days when construction and noise from the Development are permitted.
 - viii. Indicate that the construction management plan for a project will provide a phone number for noise notifications during the construction period. The Party will contact adjoining property owners directly prior to construction and will post a sign on-site and in the neighborhood, in public right-of-way or an alternate, visible location agreeable to adjoining property owners, stating that noise issues can be reported by calling the posted phone number.
 - ix. Submit written confirmation that the Party has provided information to contractors and subcontractors regarding noise mitigation requirements for Development for contractor and subcontractor review and compliance with same.
 - x. If initial site clearing is proposed during the Spring season, the developer shall consider methods to reduce the impact to nesting birds.
- 4.9 Amendment and Modification. The terms of this Agreement may be amended or modified by the mutual consent of the Parties, A modification of this Agreement that is considered a Major Modification (as hereinafter defined) of the terms of this Agreement shall follow the same procedures as required by North Carolina law for the adoption of a development agreement. Any Party may propose a Major Modification requiring amendment or a Minor Modification (as hereinafter defined) to this Agreement. Upon receipt of a proposed modification, the Town Manager shall consider the following criteria in making the determination as to whether a proposed modification is a Major Modification or a Minor Modification to this Agreement. The following changes (a) (g) will be considered Major Modifications under the Agreement:
 - a) A substantial change in the boundaries of Property subject to this Agreement as defined as:
 - i. any single proposed increase or decrease in the area of land subject to this Agreement of more than five (5) percent, or
 - ii. a cumulative increase of ten (10) percent or more in the land area subject to this Agreement.
 - b) An increase in the amount of total square footage of greater than five (5) percent
 - c) An increase in maximum building stories

- d) Any decrease in the proposed stormwater management standards
- e) Any encroachment into the 100 foot buffer other than the uses contemplated in this Agreement.
- f) Locating a building that is designated to be located internally within the site to the south or east of the internal loop road.
- g) Construction of a fire station or other uses not included on Exhibit F
- h) Any increase in the number of vehicular parking spaces.

All other amendments or modifications to this Agreement are considered Minor Modifications and therefore would not require the procedures for adoption of a development agreement. All Minor Modifications require notice to Town Council and shall be subject to review and approval by the Town Manager. Such approval shall be memorialized by letter from the Town Manager and acknowledged by the Parties and shall be maintained on file by the Town Clerk if applicable.

Notwithstanding the above, some proposed changes to this Agreement that do not meet the threshold to constitute a Major Modification may in the judgment of the Town Manager, because of size, perimeter location or transportation impacts, merit public review. In the event the Town Manager makes such a determination, the Town Manager may submit a proposed Minor Modification as notification to Town Council to allow an opportunity for Council review. Unless the Parties agree otherwise, such a review shall not extend the time period allowed for a decision by the Town Manager on the Minor Modification or convert the change from a Minor Modification into a Major Modification.

All proposed adjustments to this Agreement shall be publicly posted in such a manner that citizens of Chapel Hill will have the opportunity to express any concerns to the Town Council and/or the Town Manager.

- i) The Town Manager shall determine whether a proposed adjustment to this Agreement is a Major Modification or a Minor Modification within fifteen- (15-) working days of receipt of a proposed adjustment and shall promptly notify the Town Council and applicant of that determination. If the proposed action is determined to be a Major Modification, the Town Manager shall require the filing of an application for approval of the amendment, following procedures outlined in Section 1.6 of this Agreement.
- j) Minor Modifications to this Agreement may be approved by the Town Manager as long as such changes continue to be in substantial compliance with the approving action of the Town Council and all other applicable requirements and result in a configuration of buildings/development that is generally consistent with this Agreement. The Town Manager shall not have the authority to approve changes that constitute a Major Modification to this Agreement.
- 4.10 <u>Recordation/Binding Effect.</u> Within fourteen (14) days after the Town of Chapel Hill and University enter into this Agreement, the Municipality shall record this Agreement in the Registry. The burdens of this Agreement shall be binding upon, and the benefits of this Agreement shall inure to the Town and its successors in interest, to the Parties and their successors and assigns in title (as defined in Section 4.23). All of the provisions of this Agreement shall be enforceable during the term as equitable servitudes and constitute covenants running with the land pursuant to applicable law.
- 4.11 <u>Annual Report.</u> The Parties or their designee shall on an annual basis for five consecutive years or until construction is completed submit a written report to the Town Manager on the Development undertaken pursuant to this Agreement in the previous year (the "Annual Report"). The Annual Report shall set forth all individual Site Development Permits issued, infrastructure installed, the

status of participation by the Parties in the provision of or financing of public infrastructure for the Development, dedications and acquisitions of infrastructure by the Parties and the projected schedule for Development of the Property in the forthcoming year. The Annual Report shall include all of the information required pursuant to Section 5.25 of the Agreement and shall be provided at the times specified by that Section. The Annual Report shall also include a report demonstrating good faith compliance by the Parties with the terms of this Agreement. Upon receipt of the Annual Report, the Town Manager shall undertake the Periodic Review as set forth in Section 4.12 of this Agreement. For so long as the Town of Chapel Hill is the sole occupant of the Property, the Town of Chapel Hill agrees to prepare and submit the Annual Report, subject to the review and approval of the University prior to submission.

4.12 Periodic Compliance Review and Enforcement.

- a) Periodic Review. Pursuant to G.S. 160A-400.27, the Town Manager shall conduct a periodic compliance review (the "Periodic Review") at least every twelve (12) months for five consecutive years or until construction is completed, at which time the Parties shall be required to demonstrate good faith compliance with the terms of this Agreement. The Town Manager shall promptly report the results of this review to the Town Council. The Town Manager will use the Annual Report as the basis for preparing the Periodic Review.
- b) <u>Periods of Inactivity</u> If there has been no significant activity on the Property for twelve months, the Parties may submit a statement to that effect in lieu of the Annual Report.
- c) <u>Material Breach.</u> If, as a result of the Periodic Review, the Town Council finds and determines that any Party has committed a material breach of the terms or conditions of the Agreement (the "Breaching Owner"), the Town Manager shall serve notice in writing to the Breaching Party within fifteen (15) days after the Periodic Review setting forth with reasonable particularity the nature of the breach and the evidence supporting the finding and determination.
- d) Right to Cure. After Notice has been served, the Breaching Party shall have a reasonable time in which to cure the material breach (the "Breaching Owner's Cure"), but in no event shall the Breaching Party have less than thirty (30) days for the Breaching Party's Cure. Notwithstanding the foregoing, the Breaching Party, as applicable, shall be afforded an additional reasonable period of time to cure the breach if cure is commenced within the time period and thereafter diligently pursued but cannot be completed within said time frame.
- e) Termination or Modification by the Town. If the Breaching Party fails to cure the material breach under Section 4.12(c), then the Town Council may elect to terminate or modify the Agreement after complying with mediation requirements as stated in Section 4.13. Any notice of termination or modification or finding of breach by the Town may be appealed to the Town Board of Adjustment in the manner provided by G.S. 160A-388(b1). Thereafter, the Breaching Party may pursue any other rights and remedies available at law or in equity. If after mediation the Town Council elects to unilaterally modify the Agreement, the Parties may elect for the Agreement to be terminated rather than accede to the Agreement with the modifications made by the Town Council by giving written notice to the Town within sixty (60) days after the proposed modification. In such an event, Section 4.27(a) and Section 4.27(b) shall apply and the Party has the right to file a termination in the Registry. Failure of the Town to conduct this Periodic Review shall not constitute a waiver by the Town of its rights to otherwise enforce the provisions of this Agreement, nor shall the Parties have or assert any defense to such enforcement by reason of such failure to conduct a Periodic Review.
- 4.13 <u>Mediation</u>. In the event any Developer believes another Developer is in default or is in material breach, the Developers shall make a good faith effort to negotiate and informally resolve the issues in dispute prior to terminating this Agreement. In the event of an impasse between the Participants in reaching any mutual agreement mandated by this Agreement, the Developers shall make good

faith efforts to negotiate and informally resolve the issue in dispute (the "Claim"). If the Developers do not resolve the Claim through negotiation within 30 days of the date of the notice of default, the Developers agree to submit the claim to mediation pursuant to the following process:

- a) The non-defaulting Developer (the "Claimant") shall have thirty (30) additional days within which to submit the Claim to mediation under the auspices of any dispute resolution center or other such independent agency providing similar services upon which the Participants may mutually agree.
- b) If Claimant does not submit the claim to mediation within thirty (30) days after notice of default, Claimant shall be deemed to have waived the claim, and the defaulting Developer (the "Respondent") shall be released and discharged from any and all liability to Claimant on account of such claim; provided, nothing herein shall release or discharge Respondent from any liability to Persons not a participant to the foregoing proceedings.
- c) If the Developers do not settle the Claim within thirty (30) days after submission of the matter to the mediation process, or within such time as determined reasonable or appropriate by the mediator, the mediator shall issue a notice of termination of the mediation proceedings. Such notice shall set forth when and where the Developers met, that the Participants are at an impasse, and the date that mediation was terminated.
- d) If settlement does not occur and mediation is terminated, the Developers may pursue any and all actions at law and equity permitted under this Agreement subject to the right to notice and cure as provided in Section 4.12.
- 4.14 <u>Development Timing and Moratoria</u>. The Parties may, but shall not be obligated to, develop the Property pursuant to this Agreement. If any Party so elects to develop a portion of the Property, such Participant shall carry out the Development of such portion of the Property in such order and sequence as the Parties determine in its discretion, provided such does not violate an express provision of this Agreement. Phasing of the Development shall be based on sound engineering to ensure functional and safe street circulation and utility systems at all times. Phasing shall be based in such a way that all infrastructure improvements to be constructed within the Property will be provided when or before they are necessary for that phase of the Development. Absent an imminent threat to public health or safety, neither the right to develop nor the timing of Development shall be affected by a moratorium or suspension of development rights adopted by the Town except to the extent imposed by this Agreement or by supervening federal or state law, order, rule or regulation.
- 4.15 <u>Default.</u> Apart from the Periodic Review process set forth in Section 4.12 of this Agreement, the failure of the Parties to comply with the terms of this Agreement shall constitute a default, entitling the non-defaulting Developer or Developers to pursue such remedies as allowed under applicable law against the defaulting party, after following mediation requirements in Section 4.13 above, provided, however, that no termination of this Agreement may be declared by the Town of Chapel Hill or the University absent its according to the Parties, the notice and opportunity to cure set out in Section 4.12 of this Agreement.
- 4.16 Force Majeure. In addition to specific provisions of this Agreement, neither the University nor the Town of Chapel Hill shall be responsible for any default, delay or failure to perform if such default, delay or failure to perform is due to causes beyond such Participant's reasonable control, including, but not limited to, strikes, lockouts, actions or inactions of governmental authorities (excluding Participating Parties to this Agreement), epidemics, wars, embargoes, fires, hurricanes, adverse weather, acts of God, interference duly caused by any other Participant, or the default of a common carrier. In the event of a default, delay or failure to perform due to causes beyond such Party's reasonable control or due to interference by another Party, any date or times by which the Parties are otherwise scheduled to perform, if any, shall be extended automatically for a period of time equal in

duration to the time lost by reason of the cause beyond the reasonable control of such Participant. If written notice of such delay is given to the other Party after the commencement of such delay, an extension of time for such cause shall be deemed granted and will be agreed to by the Parties in writing for the period of the enforced delay, or longer as may be mutually agreed.

- 4.17 <u>Disclaimer of Joint Venture</u>, <u>Partnership and Agency</u>. This Agreement shall not be interpreted or construed to create an association, joint venture, or partnership between the University, the Endowment and the Municipality, or to impose any partnership obligation or liability upon such participating Parties. Neither the University nor the Town shall have any right, power or authority to enter into any agreement or undertaking for, or act on behalf of, or to act as or be an agent or representative of, or to otherwise bind, the other Party as provided in this Agreement.
- 4.18 No Third Party Beneficiaries. The Agreement is not intended to and does not confer any right or benefit on any third party that is not a signatory to this Agreement.
- 4.19 <u>Legal Actions</u>. In addition to any other rights or remedies, and subject to the mediation requirements in Section 4.13, and further subject to the notice and right to cure provisions in Section 4.12, any Party may institute legal action against a defaulting Party or a defaulting Parcel Owner to cure, correct, or remedy any default or breach, to specifically enforce any covenants or agreements set forth in the Agreement or to enjoin any threatened or attempted violation of the Agreement, or to obtain any remedies consistent with the purposes of the Agreement. Legal actions shall be instituted in the Superior Court of the County of Orange, State of North Carolina, and the Participating Parties hereto submit to the personal jurisdiction of such court without application of any conflicts of law's provisions of any jurisdiction. In the event that it becomes necessary for a Participant to pursue a civil action against a defaulting Participant, the prevailing party in such action shall be entitled to recover its reasonable attorney fees and costs from the defaulting party if such fees are awarded by a court of competent jurisdiction.
- 4.20 Notices. Unless specifically provided otherwise by this Agreement, any notice, demand, request, consent, approval or communication which a signatory Party is required to or may give to another signatory Party hereunder shall be in writing and shall be delivered or addressed to the other at the address below set forth or to such other address as such Party may from time to time direct by written notice given in the manner herein prescribed. Such notice or communication shall be deemed to have been given or made when communicated by personal delivery or by independent courier service or by facsimile or by email with a hard copy to be delivered by independent courier service by the next business day. The Parties shall make reasonable inquiry to determine whether the names or titles of the persons listed in this Agreement should be substituted with the name of the listed person's successor. Notice to a Parcel Owner shall be to the address designated in the deed conveying the Parcel to the Parcel Owner.

All notices, demands, requests, consents, approvals or communications to the Town of Chapel Hill shall be addressed to:

Planning and Development Services Town of Chapel Hill 405 Martin Luther King, Jr. Blvd. Chapel Hill, NC 27514 919-969-5066 (phone) planning@townofchapelhill.org (email)

All notices, demands, requests, consents, approvals or communications to the Parties shall be addressed to:

Property Office University of North Carolina at Chapel Hill 103 Airport Drive, CB #1060 Chapel Hill, NC 27599-1060 919-966-3297 (fax) property@unc.edu (email)

Town Manager Town of Chapel Hill 405 Martin Luther King, Jr. Blvd. Chapel Hill, NC 27514 919-969-2063 (fax) manager@townofchapelhill.org (email)

- 4.21 Entire Agreement. This Agreement sets forth and incorporates by reference all of the agreements, conditions and understandings between the Parties relative to the Property and supersedes all previous agreements. There are no promises, agreements, conditions or understandings, oral or written, expressed or implied, among the Parties relative to the matters addressed herein other than as set forth or as referred to in this Agreement or as contained in the LUMO as of the Effective Date.
- 4.22 <u>Construction</u>. The Parties agree that each Party and its counsel have reviewed and revised this Agreement and that any rule of construction to the effect that ambiguities are to be resolved against the drafting Party shall not apply in the interpretation of this Agreement or any amendments or exhibits hereto.
- 4.23 <u>Assignment.</u> After notice to the Town and to the Municipality, the University may assign its respective rights and responsibilities hereunder to subsequent land owners of all or any portion of the relevant parcels of land, provided that no assignment as to a portion of the relevant parcel of land will relieve the assigning Party of responsibility with respect to the remaining portion of the relevant parcel of land owned by the assigning Party without the written consent of the Town.
- 4.24 Governing Law. This Agreement shall be governed by the laws of the State of North Carolina.
- 4.25 <u>Counterparts</u>. This Agreement may be executed in several counterparts, each of which shall be deemed an original and such counterparts shall constitute one and the same instrument.
- 4.26 <u>Agreement to Cooperate.</u> In the event of any legal action instituted by a third party or other governmental entity or official challenging the validity of any provision of this Agreement, the Participating Parties hereby agree to cooperate in defending such action; provided, however, each Participant shall retain the right to pursue its own independent legal defense.
- 4.27 <u>Termination</u>. This Agreement shall terminate within 20 years of the recording of this agreement, unless both parties agree to an extension; or a specific termination made by operation of the provisions of this Agreement; or by agreement of the Parties. Any Termination other than by expiration of the term shall be recorded in the Registry. Termination of this Agreement shall not affect any of the following:
 - a) any requirements to comply with the applicable terms and conditions of the LUMO, Site Development Permits, approval and acceptance of infrastructure improvements, and any applicable permits;

- b) rights under this Agreement with respect to which a Site Development Permit has been issued by the Town Manager prior to the termination or expiration but for which construction is not completed.
- c) In the event consideration is given to (i) suspension of this Agreement or (ii) termination of this Agreement, the Parties agree that each will identify appropriate representatives to meet and participate in good faith negotiations and mediation as provided in Section 4.13, aimed at resolving the issues prompting that consideration.
- 4.28 No Obligation to Develop. It is understood that the Town's and University's development of the Project depends upon a number of factors including, but not limited to, the availability of financing, state appropriations, receipt funds, general economic factors, and academic and programmatic needs. Nothing in this Agreement shall be construed as requiring the Town or University to develop the subject property and any failure to develop the subject property shall not be deemed a default by the Town of University of its obligations set forth in this agreement.
- 4.29 <u>No Deemed Waiver</u>. Failure of a Party to exercise any right under this Agreement shall not be deemed a waiver of any such right and shall not affect the right of such Party to exercise at some future time said right or any other right it may have hereunder.
- 4.30 <u>Severability</u>. If any term or provision herein shall be judicially determined to be void or of no effect, such determination shall not affect the validity of the remaining terms and provisions. The participating Parties agree that if any provision of this Agreement is judicially determined to be invalid because it is inconsistent with a provision of state or federal law, this Agreement shall be amended to the extent necessary to make it consistent with state or federal law and the balance of the Agreement shall remain in full force and effect. Participating parties, having voluntarily agreed to be contractually bound, for themselves and their successors and assigns, accept all of the terms of the Agreement and confirm their belief that the terms are consistent with applicable law as of the Effective Date.
- 4.31 <u>Authority</u>. Each Party represents that it has undertaken all actions necessary for approval of this Agreement, and that the person signing this Agreement has the authority to bind the Parties on behalf of whom they have executed this Agreement.
- 4.32 <u>Transparency and Public Involvement.</u> It is the intent of the participating Parties that all aspects of the implementation of this Agreement shall be carried out in an open, transparent fashion with opportunities for effective and meaningful public involvement. The Town and the University shall take reasonable steps to make information about all aspects of the implementation of this Agreement (including required studies, analysis, plans, reports, and applications which are public record) fully available for public review with the exception of any attorney-client privileged information, financing documentation, market analysis, internal financial documentation, reports, pro formas, returns or other personal information.

The Developers shall take efforts to form a committee consisting of interested residents, Town, and University staff to allow for future dialogue before and during construction efforts. Adjacent residents shall be notified of upcoming construction, in accordance with 5.22, and the Town and the University shall take reasonable steps to form a committee to discuss construction issues during periods of construction. The matters of such a committee may be continued during operations of the site.

4.33 Estoppel. Each of the participating Parties agree, from time to time, within twenty (20) days after

request of the other Participant, to deliver to the requesting Participant or such Participant's designee, an estoppel certificate stating that this Agreement is in full force and effect, the unexpired term of this Agreement, and whether or not, to such Participant's knowledge, there are any existing defaults or matters which, with the passage of time, would become defaults under this Agreement. It is understood and agreed that the Parties' obligation to furnish such estoppel certificates in a timely fashion is a material inducement for the other's execution of this Agreement. In no event may any Party request from the other an estoppel certificate more than one (1) time in any twelve month period other than in connection with a bona fide sale or financing of the Property or portion thereof, any interests in, or any assets of, any Party.

ARTICLE 5: SPECIFIC STANDARDS AND MITIGATION MEASURES

The Parties do hereby agree to the specific standards and mitigation measures set forth in this Section.

5.1 Scale of Development and Uses Permitted.

a) The development of the Property shall not exceed the limits in Table 1 of this Article and shall be generally consistent with the layout in EXHIBIT A. A design is generally consistent if it does not significantly change the appearance, function, or impact of the project to the detriment of neighboring properties or the community.

Table 1: Scale of Development

Land Area	Total Floor Area	Maximum Building Height	Maximum Number of Parking Spaces
20.55 acres	200,000 s.f.	4 stories	615 spaces

- b) Land uses within the University-1 (U-1) zoning district and as further modified in EXHIBIT F are permitted.
- c) The Town may build and/or occupy a maximum of 100,000 s.f. of the allowable 200,000 s.f. of building area on the Property.
- d) The property is designed such that buildings and infrastructure are located towards Estes Drive and away from the Elkin Hills neighborhood.

5.2 <u>Timing of Development</u>

- a) Development of individual buildings is anticipated to occur in separate phases through the Term of the agreement.
- b) A Site Development Permit is required for each building.
- c) The following land uses and activities that do not involve the construction or use of a building shall be undertaken in a manner substantially consistent with the terms of this Agreement. and the Town and University shall include a report of all such activities undertaken in the previous year in the Annual Report required by this Agreement:
 - i. Maintenance and improvements to existing utility lines;
 - ii. Stormwater lines and improvements;
 - iii. Stormwater structures;
 - iv. Public utility lines traversing site (Duke Energy, PSNC, UNC, etc.);
 - v. Utility structures without a building (e.g., transformers);
 - vi. Alterations to utility equipment (generator, other mechanical components);
 - vii. Site storage and construction staging areas;
 - viii. Stream restoration projects;
 - ix. Greenways and other trails;
 - x. Public art.

5.3 Phasing

- a) First Phase: The Municipal Services Center is anticipated to be the first phase of development as generally illustrated in EXHIBIT B. Land disturbance shall be limited to the extent possible to allow earthwork where necessary for development of the building and supporting infrastructure in EXHIBIT C. The first phase shall include construction of the Municipal Services Center with ancillary infrastructure serving the building and the following improvements:
 - i. Construction of an internal access road with two points of access on Estes Drive.

- ii. Stormwater Control Measures, as specified in the Stormwater Management Report, to treat stormwater for the entire Project Area.
- iii. Vegetative planting to provide screening and buffering of the first phase of development, and planting of additional buffer areas that would not be negatively impacted by future phases of construction.
- iv. Vehicular and bicycle parking to serve the Municipal Services Center.
- v. Documentation of SEPA compliance (Environmental Assessment and either Finding of No Significant Impact or Environmental Impact Study) is required for the entire plan of development.
- b) Subsequent Phases: Every building constructed after the Municipal Services Center is expected to provide its own screening, infrastructure, utilities, and parking.
 - i. Each phase of subsequent development constructed by either the University or the Town shall provide infrastructure associated with building development and vegetative planting to screen the proposed use from residential properties located to the south and east.

5.4 Open Space and Natural Areas

- a) A minimum 100 foot buffer along the southern and eastern boundary of the property shall remain undisturbed except for the minimum disturbance required to install a sanitary sewer connection to Hartig Street and walking paths as generally shown on conceptual site plan. The following may be permitted within the buffer area:
 - i. Planting of new vegetation on the interior side of the buffer.
 - ii. Walking trails
 - iii. Forest education or other passive use of the wooded area that does not cause audial, visual, or other nuisance.
- b) A minimum 50 foot buffer shall be provided surrounding all intermittent streams which may be encroached only to allow road and infrastructure crossings. Encroachments shall be designed and constructed to have the minimum necessary impact on the buffer.

5.5 Stormwater Management

- a) Stormwater management shall be integrated into the site, building, and landscape design, with stormwater management strategies coordinated and applied in a comprehensive manner across the entire Project. The intent of this section is to improve stormwater conditions after development.
- b) Stormwater Control Measures calculated to treat the maximum impervious surface of the entire Property shall be submitted with the initial Site Development Permit application.
- c) EXHIBIT A shows the general location of impervious surfaces in the Development.
- d) Prior to issuance of the first Site Development Permit, the Applicant must submit a Stormwater Management Plan for the project which clearly identifies the stormwater impacts associated with the proposed development and clearly documents how those stormwater impacts are proposed to be mitigated by the improvements proposed in the application. Stormwater Management Plans shall clearly demonstrate compliance with the design criteria specified in this Agreement, applicable NCDEQ permit requirement, and applicable University, Federal, and State rules.
- e) Each Site Development Permit application shall provide a tabulation of the proposed and total impervious surface to demonstrate compliance with the impervious surface permitted by the Stormwater Management Plan.
- f) Peak Discharge Rate Limits: The post-development stormwater runoff peak discharge rate shall be controlled such that the post-development runoff peak discharge rate at all locations where stormwater runoff exits i n the Project (or at other points designated in the conceptual plan), shall not exceed the stormwater runoff peak discharge rate for the local 1-year (2.88 inches), 2-year

- (3.60 inches), and 25-year (6.41 inches), and 50-year (6.86 inches) 24-hour duration storm events.
- g) Two- (2-) Year Volume Control: At each location where the stormwater exits the Project, the difference in the runoff volume generated by 2- year (3.60 inches) frequency, 24-hour duration storm event in the post-development conditions and runoff volume generated by the same storm event in the pre-development conditions shall be managed on-site and released over a period of 2 days to 5 days.
- h) Eighty-five percent (85%) Total Suspended Solids (TSS) Removal: All post-development stormwater runoff resulting from the first one inch of precipitation shall be treated to remove 85% of total suspended solids of all new impervious surfaces resulting from development of the entire Property. Stormwater treatment facilities will be designed according to the North Carolina Department of Environmental Quality (NCDEQ) Stormwater Best Management Practices Manual as modified by the Town of Chapel Hill; the Town of Chapel Hill Design Manual, as amended; and any future written design guidance approved by both the Town and NCDEQ.
- i) Upon completion of each Site Development Permit, the Municipality shall receive:
 - i. The maintenance access must be shown and/or described in the Stormwater Plans. A copy of the Operations and Maintenance Plan, signed by the owner and for the stormwater management facility(ies). The Operations and Maintenance Plan must include a description of the device or structure, an inspections checklist, and operating and maintenance procedures. The plan should identify contact information, who will perform the inspections, frequency of inspections, inspections and maintenance logs, any specific equipment needs or certifications (e.g., confined space certification), action levels or thresholds (e.g., remove sediment after depth exceeds one foot), and disposal methods.
 - ii. Certified as-built plans, signed and sealed by a North Carolina-registered Professional Land Surveyor, showing building footprints, driveways, all other impervious surfaces, stormwater drainage/conveyance piping, and stormwater management structures. The asbuilt plans should be in DXF binary format using State plane coordinates and NAVD 88.
 - iii. Certification, signed and sealed by a North Carolina-registered Professional Engineer, that the stormwater management facility(ies) was/were constructed in accordance with the approved plans and specifications.
- j) Proposed covered parking/underground parking areas will require oil/water separator in the parking deck to collect the runoff prior to discharge into the sanitary sewer. The top level of parking areas will drain to the proposed stormwater control measures.
- k) The sumps of hydraulic elevators, if proposed, shall be connected to the sanitary sewer.
- The development shall meet the Jordan Lake Rules in place at the time of approval of the Development Agreement. Per Jordan Watershed Stormwater Management for Development, Nitrogen and Phosphorus exported from the Property shall not exceed 2.2 pounds/acre/year and 0.82 pounds/acre/year respectively.
- m) That Stormwater runoff from the site shall be reused whenever possible.
- n) Stormwater control measures shall not be permitted within the 100 foot vegetated buffer.
- o) Once project funding is allocated, the Town shall initiate an evaluation of downstream stormwater conditions and opportunities for improvement. The evaluation will include documentation of pre-construction stormwater conditions, and downstream stormwater performance shall be monitored following construction of the Municipal Services Center.

5.6 Stream Buffers

a) Development shall be consistent with the Resource Conservation District regulations of the Town applicable as of the date of this Agreement. All activities designated as permitted uses or special

- uses in Table 3.6.3-2 of the LUMO that are consistent with this Agreement shall be designed in compliance with this Section.
- b) The Town or University may perform and use a stream determination for future development provided that the location of buildings and infrastructure is consistent with EXHIBIT A.
- c) Development shall comply with Sections 404 and 401 of the Clean Water Act and with the Jordan Buffer Rules. As the landowner of record, the University shall review and provide comments to the Town on all 404, 401 and Buffer Authorization permits prior to their submittal to the applicable regulatory agencies.

5.7 Tree Canopy & Landscaping:

- a) A minimum of 40 percent tree canopy shall be maintained.
- b) Native plant species shall be used by the Town in all plantings. Prior to the issuance of the first Site Development Permit, the Project shall consult with Town staff, the North Carolina Botanical Garden, and other resources for appropriate species.
- c) Invasive species shall not be allowed.
- d) The Project shall be planned to support a low maintenance, low water use, low fertilizer use landscape with 'best suited' plant materials.
- e) Landscape Protection plans and Planting Plans for individual Site Development Permit applications shall identify existing and proposed tree canopy mix and percent coverage, and landscape materials. If irrigation is proposed, controls shall utilize smart technology to conserve water. Until such time as the University occupies the Property, the Town shall contact UNC Grounds to consult on all landscape design, protection, and planting plans prior to implementation.
- f) Continuous tree protection fencing shall be shown at the limits of disturbance on the demolition and grading plans. Fencing must be installed and inspected prior to any site work or clearing.
- g) A foundation buffer strip, as described in LUMO 5.9.5.c.2, may be provided in variable width.

5.8 Compatibility and Buffers:

- a) Development shall strive to be compatible with existing adjacent development and will be buffered per LUMO 5.4. Development near existing neighborhoods shall respect buffers, height limits, lighting, drainage, noise impacts.
- b) A planting mix or density is not prescribed for the southern or eastern buffer. Existing vegetation will be undisturbed but supplemental plantings may be placed adjacent to the buffer to improve screening as practical.
- c) A landscape buffer as described in the Town Design Manual is not required along Estes Drive.
- d) Noise and Lighting impacts will be regulated per Sections 5.17 and 5.18 in this agreement.

5.9 <u>Transportation: General Improvements, Traffic Impact Study, Specific Improvements, Transit, Parking, Streets, Sidewalks, Bikeways, and Greenways</u>

The general location of transportation-related improvements is shown in EXHIBITS E and K.

a) General Provisions:

- i. EXHIBIT E provides a plan for vehicle, bicycle, pedestrian, and bus amenities on the site. The exact location of each improvement shall be illustrated in each Site Development Permit, and be consistent with the locations shown on EXHIBIT E.
- ii. The internal access road with two accesses on Estes Drive is designed to contain development in the northern portion of the site and be located away from residential properties to the south and east. Final design of the road shall reflect this consideration.

iii. The location of bicycle, pedestrian, and bus amenities should be coordinated with the Carolina North Development Agreement (including planning for the campus to campus connector), and Town master plans for bikes and greenways.

b) Traffic Impact Study:

- i. The Municipality's consultant performed a Traffic Impact Study for the first phase of development, the Municipal Services Center. Impacts associated with that building were studied and determined for this agreement and are listed in 5.9.c and 5.9.d. The Traffic Impact Study is provided as EXHIBIT K.
- ii. An internal access road with two accesses on Estes Drive shall be constructed with the Municipal Services Center Phase as shown on (EXHIBIT E).
- iii. A Traffic Impact Study will be performed for each subsequent Site Development Permit that results in an increase of daily trip generation of greater than 500 trips, or 250 vehicles per day, whichever is less.
- iv. The applicant for a future Site Development Permit shall pay the cost of a required Transportation Impact Study and any necessary improvements within the site or right of way.

c) Necessary Improvements - Due to Site Impact

Construction of the Municipal Services Center shall include these specific improvements:

- i. Widen Estes Drive along the length of site frontage to provide a consistent three-lane cross-section with exclusive westbound left-turn lanes into the site at the two proposed site driveway intersections. This improvement improves operations for stop-controlled movements at both intersections and improves overall safety by removing the left-turn movements from the through traffic streams along the Estes Drive.
- ii. Provide a bus stop and amenities for transit riders along the frontage of the proposed Municipal Services Campus.

d) Necessary Improvements – Regardless of Site Impact

i. The applicant shall widen Estes Drive to provide exclusive westbound left-turn lanes into the site at the two proposed site driveway intersections. These (dedicated turn lanes) improve operations for stop-controlled movements at both intersections and improves overall safety by removing the left-turn movements from the through traffic streams along the Estes Drive corridor.

e) General Coordination for Estes Drive Corridor:

- i. The Town shall construct the improvements related to the project and coordinate with future NCDOT, Town, or University-funded projects to widen Estes Drive between the proposed site frontage and the NC Highway 86 (Martin Luther King Jr. Blvd.) intersection to a consistent three-lane cross-section. These improvements shall be compatible with the design of the NCDOT pedestrian and bicycle improvement project along the Estes Drive corridor.
- ii. Estes Drive shall be improved along the property frontage to provide a separated bike lane and a multi-use or pedestrian facility, subject to review by the Town Manager.
- iii. The applicant shall coordinate with NCDOT to install a pedestrian crossing across Estes Drive between the Property and the UNC Park & Ride. The pedestrian crossing shall utilize Rectangular Rapid Flash Beacons (RRFB), illumination, or other enhanced features to improve pedestrian safety.

- f) Roadway Improvements for future Site Development Permits:
 - i. Each Site Development Permit will include information about the phasing of the appropriate connections and infrastructure to support occupancy of each building.
 - ii. When Carolina North is developed, the Town shall coordinate with NCDOT to evaluate installation of a pedestrian crossing across Estes Drive at the eastern driveway.

g) Vehicular Parking:

- i. A maximum of 615 parking spaces shall be permitted on the Property. An increase shall constitute a major modification.
- ii. A maximum vehicular parking ratio of 3.5 spaces per 1,000 square feet of building area shall be permitted, excluding supplemental parking for police vehicles.
- iii. Individual Site Development Permit applications shall demonstrate an adequate amount of vehicle parking for the proposed use, including handicapped and visitor spaces.
- iv. The applicant shall provide the minimum required handicapped parking spaces and design all handicapped parking spaces, ramps, and crosswalks, and associated infrastructure according to Americans with Disabilities Act standards, North Carolina Building Code, American National Standards Institute (ANSI) Code, and Town standard.
- v. Prior to issuance of a Certificate of Occupancy, the Applicant shall construct the proposed surface parking lot to Town standard for dimensions and pavement design and the Applicant shall construct the proposed parking deck to National Parking Association standards or to another nationally accepted standard that may be approved by the Town Manager.
- vi. A shared parking model may be provided with each Site Development Permit application.
- vii. A minimum of twenty (20) percent of the parking spaces approved under each Site Development Permit shall be built with conduit to allow for future electric vehicle charging.
 - a) At least 1 percent, or a minimum of five parking spaces, shall be DC Fast Charge capable.

h) Bicycle Parking:

- i. The Town shall comply with LUMO bicycle parking requirements for a *public use facility*, and shall provide a mixture of Class I and Class II bicycle parking per the Engineering Design Manual.
- ii. Bicycle parking will be located close to building entrances to allow riders easy ingress and egress. If sheltered bicycle parking is provided within the parking garage it shall be located near the building entrance.
- iii. The University will comply with University standards for required amounts, locations, and types of bicycle parking.
- iv. Prior to the issuance of a Site Development Permit, the Applicant shall provide the Town with an agreed upon bicycle parking plan for the Project that addresses the quantity and locations of bicycle parking for the site.

i) Transit

i. A bus shelter on Estes Drive shall be constructed with the first Site Development Permit. The shelter shall contain solar lighting, and next bus information. A bus pull-off shall not

be required.

ii. Sidewalks shall connect the bus shelter to the Municipal Services Center.

j) Transportation Management Plan

i. A Transportation Management Plan (TMP) shall be submitted or updated with each Site Development Permit application. Each building is encouraged to have its own TMP Champion. The TMP shall contain the checklist items as of the Effective Date, and an annual report shall be submitted to the Town.

k) Relationship to Carolina North improvements

i. During each Site Development Permit review, the Town will consider whether any improvements have been constructed in satisfaction of the Carolina North Development Agreement and whether they offset the need for separate improvements on this site. A traffic impact study considering current conditions and expected growth at the time of Site Development Permit application shall be the basis for this evaluation.

1) Greenway:

- i. The applicant shall construct a ten foot minimum width multimodal greenway connection to Hartig Street. The greenway shall consist of a non-paved surface.
- ii. A greenway shall be coordinated with the Carolina North Development Agreement campus to campus connecter and constructed along the property frontage and to Hartig Street in general conformance with the location shown on the general site layout plan.
- iii. The site greenway shall be constructed with the first phase of development.
- iv. The site greenway shall be constructed to American with Disabilities Act (ADA) standards.
- v. All pedestrian, bicycle, and greenway facilities within the Property shall be designed and constructed to meet Town standards unless otherwise approved by the Town Manager.

m) Sidewalks:

- i. Sidewalks shall be provided within the Project to provide connectivity between buildings, parking lots, the greenway connection, pedestrian facilities along the street frontage, transit stop, and other site amenities.
- ii. Sidewalks shall be six feet in width, and this width may be reduced to five feet to preserve or plant adjacent trees.

5.10 Energy:

- a) Town buildings, including the Municipal Services Center, shall achieve LEED Silver certification, at a minimum.
- b) The Developers will include energy-efficient building practices and sustainable building practices in the design and construction of individual buildings and the Project as a whole.
- c) An Energy Management Plan shall be submitted with each Site Development Permit. The plan shall:
 - i. consider utilizing sustainable energy, currently defined as solar, wind, biofuels, and hydroelectric power;
 - ii. consider purchase of carbon offset credits and green power production through coordination with the NC Green Power program;
 - iii. provide for 20 percent more efficiency than the latest applicable standard for ASHRAE (e.g., 90.1) that ensures indoor air quality and adequate access to natural lighting, and

- allows for the proposed utilization of sustainable energy in the project;
- iv. include on-going energy management practices, and
- v. require that the property owner reports to the Town of Chapel Hill both the expected energy consumption as part of a model prior to issuance of a Site Development Permit and the actual energy consumption as compared to sealed engineering calculations at the time construction is completed.
- d) The goals of the Environmental Stewardship Advisory Board Development Review (provided as EXHIBIT N) shall be considered with each Site Development Permit application.
- e) Graywater use for reclamation and irrigation is encouraged.
- f) All building roofs shall be designed as to be "solar ready" to accommodate future photovoltaic systems unless use of another green building feature such as a green roof, presents a conflict.
- g) For a Site Development Permit that includes a parking structure, the applicant will explore the feasibility of applications to reduce the heat island and increase albedo (reflected sunlight) of the parking garage.

5.11 <u>Design Standards</u>

- a) Prior to Town Staff approval of a Site Development Permit, the applicant shall present final building elevations to the Community Design Commission for review and comment.
- b) The final location and design of buildings shall be reviewed by UNC Chapel Hill staff and be subject to all necessary University approvals required for University or State-owned land.
- c) Should the applicant choose to install public art, the application will present the public art to the Cultural Arts Commission and the appropriate University entity for their review and comment.

5.12 Utilities

- a) A sanitary sewer connection along with the dedication of a sewer easement shall be permitted to Hartig Street.
- b) The University may provide utilities to University-owned buildings on the Property.
- c) The Town will be responsible for securing utilities for Town-owned buildings on the Property.

5.13 Emergency Services

- a) The Town shall be responsible for its own police, fire, and emergency medical services until the University elects to exercise its option to occupy its allocated portion of the Property. If the University occupies a portion of the Property, the Town and University shall arrange for emergency services under the generally applicable arrangements for jurisdiction and operations on University-owned property. In general, for University-owned and occupied property, the University provides police services, the Town provides fire services, and Orange County provides most emergency medical services. This Agreement shall not be deemed to restrict changes in these jurisdictional arrangements as may from time to time be mutually agreed to by the service providers.
- b) The site plan as shown in EXHIBIT A includes the location for a future fire station. Review and approval of any additional conditions associated with that future use shall require a major modification of the Development Agreement at the time a station is needed.

5.14 Historic and Cultural Features

a) An archaeological study was performed and no significant features were found. No further archaeological study is required with Site Development Permit submittals.

5.15 Construction Waste Management

a) Construction waste shall be managed in accordance with Town standards and with the Orange County Regulated Materials Ordinance.

5.16 Solid Waste Management.

- a) Construction waste shall be managed in accordance with Town standards and with the Orange County Regulated Materials Ordinance.
- b) Each individual Site Development Permit shall provide clear direction on who will provide solid waste service for each building and each building will be designed for private waste collection.
- c) Prior to the issuance of each Site Development Permit, the Applicant will provide a Will-Serve letter for solid waste and recycling collection.

5.17 Noise/Communication During Construction

- a) The Town's Noise Ordinance shall apply to the Development during construction and occupancy of the Property.
- b) Buildings shall be designed to minimize noise impacts on adjacent properties from equipment, infrastructure, vehicles, and daily operations on the Property.
- c) Noise during construction:
 - vi. As provided in Section 4.8, prior to issuance of a Site Development Permit the Applicant shall include a construction management plan that provides the following noise-mitigation information at a minimum:
 - a) Indicate how the project construction will comply with the Town's Noise
 - Ordinance; and
 - b) Provide a phone number for noise notifications during the construction period.
- d) A construction sign to facilitate communication during the project and containing a phone number to report noise issues shall be located in the Elkin Hills neighborhood. The sign shall be located in a visible location within the right of way, such as a median. The exact location of the sign shall be determined through consultation between interested neighbors and Town staff.
- e) The Applicant will communicate directly with the neighborhood through electronic media, mailings, or face-to-face meeting to provide construction contact information for the duration of the construction of any work associated with a Site Development Permit.
- f) Complaints regarding compliance with the noise ordinance during construction shall be handled by the Town for Town projects, and by the University for University projects.

5.18 Lighting

- a) The project shall provide sensitive lighting design that does not intrude on adjacent properties.
- b) Each individual Site Development Permit application shall describe all proposed lighting and demonstrate no increase in lighting foot-candle levels at the adjacent property line through submittal of a photometric plan.
- c) Site lighting shall be energy efficient and appropriate for the program requirements and times of use.
- d) All Town lighting standards in effect as of the Effective Date of this Agreement shall apply to the Project, including the Municipal Services Center. Future buildings shall meet or exceed those same standards.
- e) New lighting within the Development shall comply with the following:
 - All lighting, including that used in and around buildings, parking areas, walkways, roadways, and signs, shall be designed to minimize spillover light onto property adjacent to the Property.
 - ii. All lighting shall be designed to prevent glare that could impair vision and/or otherwise deteriorate normally accepted qualities and uses of property adjacent to the Property.
 - iii. Outdoor lighting shall be mounted at heights no greater than fifteen (15) feet for non-cutoff

lights and no greater than thirty-five (35) feet for most cutoff lights.

- f) Photocell or motion sensor-activated, or other energy efficient light fixtures shall be utilized across the site except as counter-indicated for police or emergency services.
- g) Light Emitting Diode (LED) bulbs are preferred for both interior and exterior lighting.

5.19 Signs

a) Signs shall be permitted in accordance with LUMO 5.14.

5.20 Sedimentation:

- a) Erosion and Sediment Control: Construction at the Property shall comply with applicable soil and erosion control regulations of North Carolina Department of Environmental Quality.
- b) The Town shall comply with all applicable University, State, and local soil and erosion control regulations, and shall be responsible for implementing erosion and sedimentation control measures for all land disturbing projects.
- c) The Town agrees to comply with the University's Erosion and Sedimentation Control Permit Application Process for development located on University Property (see attached process chart). As landowner of record, the University must approve of the Erosion and Sedimentation Control Plan prior to its submittal to the State for review.
- d) Discharge of dewatered water from excavated areas and erosion control basins will require approval of Stormwater Management Division of the water quality prior to pumping into storm sewer pipes.
- e) The Town shall provide erosion control measures for the 25-year storm during construction.

5.21 Construction Management Plan:

- a) The Developer shall provide a Construction Management Plan prior to issuance of a Site Development Permit. The plan shall provide for construction employee parking, construction vehicle access and routes, the location of any construction office, materials staging areas, and hours of work.
- b) The Developer shall provide a Traffic Control Plan prior to issuance of a Site Development Permit. The plan shall include, at a minimum, detour and signage for pedestrian, bicycle, and vehicular traffic during construction.

5.22 <u>Public Notification and Participation</u>

- a) The Developer shall conduct a meeting with opportunity for citizen participation and attendance by a Town staff representative; for the purpose of discussing possible design features of new facilities prior to submission of each individual site development permit application. The University shall distribute notice of the meeting to all property owners within 1,000 feet of the Property.
- b) This notice shall specify the date, time, location and purpose of the meeting. This notice shall be mailed or otherwise distributed no later than one month prior to the meeting. Minutes of the meeting and the meeting attendance list shall be made publicly available before presentation of individual site development permit applications to the Town for approval. The public meeting shall provide an opportunity to discuss the presence of outdoor activities, if any are held, and whether mitigations are needed.
- c) Town and University staff shall be identified to serve as a liaison to interested residents before and during construction efforts.
- d) Prior to submittal of the first Site Development Permit, the Town shall contact adjacent residents to form a committee consisting of residents and Town staff to allow dialogue before and during construction efforts.

5.23 Duty to Comply with Regulations and Mitigate Environmental Releases

- a) The Town will immediately clean up any environmental releases on University-owned property that result from the Town's operations.
- b) In event of a clean-up, the Town will comply with all Federal and State regulations for cleanup, environmental investigation and remediation of all chemical releases to the environment.
- c) The Town will be responsible for all costs associated with clean up, investigation and remediation of chemical releases from Town operations.
- d) All clean up, investigative and remediation documentation and site work on University-owned land shall be reviewed by University staff.
- e) The University will be responsible for immediately addressing any environmental releases that result from their operations on the property and shall comply with all Federal and State regulations.
- f) The Town shall not place any deed restrictions on the University's property.
- g) Chemicals (including fuel) shall not be stored in underground storage tanks on the Property.
- h) The Town is responsible for any fines and penalties associated with Town operations on the University's property.

5.24 Fiscal Impact

- a) Prior to submission of a University-initiated Site Development Permit, the two agencies shall draft a mutually acceptable Memorandum of Understanding or similar guidance for the following:
 - i. Joint and/or individual responsibilities for landscape maintenance
 - ii. Joint and/or individual responsibilities for infrastructure maintenance including but not limited to sidewalks, roadways, stormwater management facilities, parking areas, bicycle facilities, and greenways.
 - iii. A system for tracking costs for future-shared infrastructure (such as parking, roadways and stormwater facilities) so that the cost of full infrastructure for the Project can be shared in a pro-rata manner as each party builds structures and infrastructure on the Property.
 - a) Shared infrastructure shall include the following:
 - i. all stormwater management facilities
 - ii. the drive aisle that serves each of the buildings
 - iii. sidewalks along the drive aisle that serves each of the buildings
 - iv. greenway
 - v. sanitary sewer connection to Hartig
 - vi. transit shelter
 - vii. cost of Estes Drive improvements
 - viii. lighting
- b) The costs for each of the above improvements shall be itemized and documented by the Town following the first Site Development Permit. Alternately, the above topics may be addressed in a lease document between the Town and the University.
- c) Subject to the terms and conditions of this Agreement, the Town of Chapel Hill shall be wholly responsible for all property and infrastructure maintenance (and associated costs) while it is the sole occupant of the site.
- d) The Town of Chapel Hill shall be responsible for property and infrastructure maintenance while it is the sole user of the site.

5.25 Annual Report

a) The Applicants shall submit to the Town Manager an Annual Report that includes the information required by this Agreement and that provides all necessary information for the Town Manager to assess their good faith compliance with the terms of this Agreement. This report shall form the

- basis for the Town Manager's periodic review of the Agreement as required by G.S. 160A-400.27(a). This required report is generally referred to as the "Annual Report."
- b) The initial Annual Report shall be filed on or before September 1, 2019 and shall report on activities from July 1, 2018 through June 30, 2019. Subsequent reports shall be filed on or before September 1 each year and shall report on activities in the preceding fiscal year (the preceding reporting period).
 - i. The Annual Report shall include the specified items set forth in this Agreement and listed in this Section. The failure to include in this Section an item expressly required to be included by other Sections of this Agreement shall not relieve the Applicants of the responsibility to include that item in the Annual Report. The report may include such other items as deemed relevant by the Applicant. The Town Manager may also request inclusion of other specific information or provide for its inclusion in the following year's Annual Report if such requested information reasonably relates to Development.

EXHIBIT A







TOWN OF CHAPEL HILLMUNICIPAL SERVICES BUILDING
06.27.2018







TOWN OF CHAPEL HILLMUNICIPAL SERVICES BUILDING
06.27.2018









TOWN OF CHAPEL HILLMUNICIPAL SERVICES BUILDING
06.27.2018









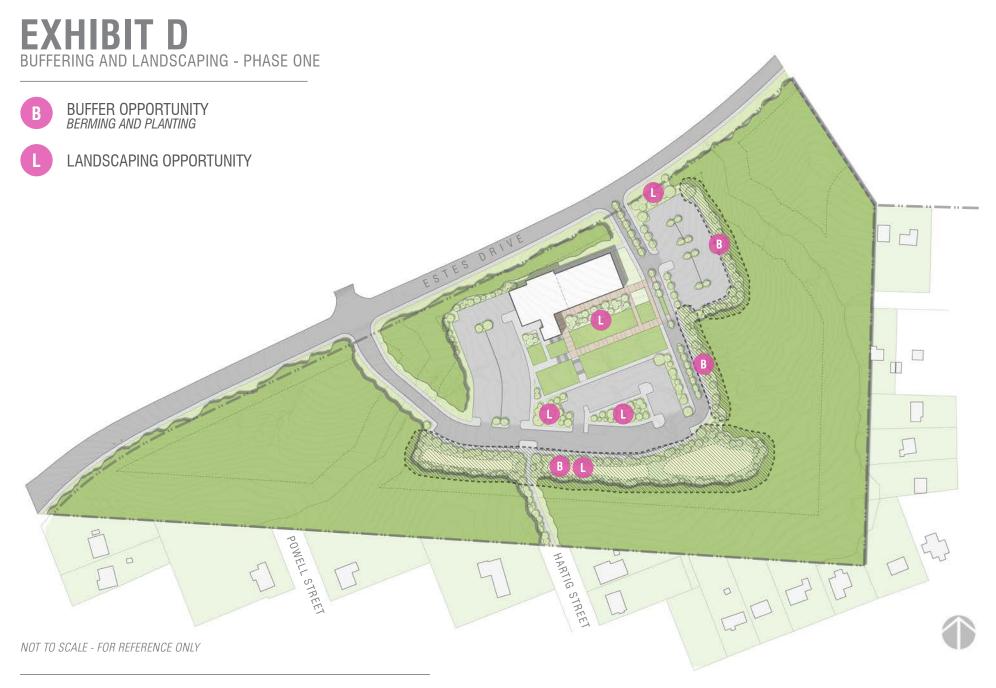






EXHIBIT E SIDEWALKS AND PATHS - PHASE ONE







Exhibit F: Permitted Land Uses

Permitted Use	Permitted, Accessory
Accessory use customarily incidental to a permitted principal or special use	А
Automated teller machines (ATM) (Walkup)	Р
Clinic	P, A
College or University	Р
Essential services	P, A
Public cultural facility	P, A
Public use facility	P, A
Radio, television or wireless transmitting and/or receiving antenna, accessory	А
Research Activities, Light	Р
Temporary portable building: Construction-related (See also Article 6)	А

Exhibit G - Legal Description

Legal Description: Former Dixon Property

Beginning at an iron pipe located in the southern right of way of Estes Drive Extension (S.R. 1780) running thence S 11° 23′ 00″ E 348.61 feet to an iron pipe, thence N 76° 30′ 50″ W 161.71 feet to an iron pipe; thence N 76° 30′ 50″ W 86.10 feet to an iron pipe located in the eastern right of way of Powell Street; thence N 76° 51′ 00″ W 66.45 feet to an iron pipe located in the western right of way of Powell Street; thence N 76° 29′ 00″ W 247.69 feet to an iron pipe; thence N 76° 29′ 00″ W 216.68 feet to an iron pipe located in the southern right of way of Estes Drive Extension (S.R. 1780); thence with the southern right of way of Estes Drive Extension (S.R. 1780) in a general northeasterly direction along a 557.97 foot radius curve to the right, said curve having a chord bearing and distance of N 62° 17′ 17″ E 248.62 feet, to an iron pipe in the southern right of way of Estes Drive Extension (S.R. 1780); thence continuing with the southern right of way of Estes Drive Extension (S.R. 1780) N 75° 09′ 40″ E 321.76 to an iron pipe, the point and place of beginning, containing 2.42 acres, more or less.

Legal Description: Facilities Parcel Division

Beginning at an iron pipe located in the southern right of way of Estes Drive Extension (S.R. 1780) running thence S 21° 31′ 42″ E 346.01 feet to an iron pipe; thence S 86°46′ 22″ E 1109.76 feet to a point; thence N 00° 41′ 32″ E 744.68 feet to a point; thence N 33° 28′ 00″ W 319.16 feet to a point in the southern right of way of Estes Drive Extension (S.R. 1780); thence with the southern right of way of Estes Drive Extension (S.R. 1780) in a general northwesterly direction along a 2019.02 foot radius curve to the right, said curve having a chord bearing and distance of S 53° 57′ 42″ W 586.01 feet, to a point in the southern right of way of Estes Drive Extension (S.R. 1780); thence continuing with the southern right of way of Estes Drive Extension S 64° 46′ 47″ W 685.46 feet to an iron pipe, the place and point of beginning, containing 18.13 acres, more or less.

ZONING ATLAS AMENDMENT APPLICATION



TOWN OF CHAPEL HILL Planning Department

405 Martin Luther King Jr. Blvd phone (919) 969-5066 fax (919) 969-2014 www.townofchapelhill.org

Parcel Iden	ntifier Number (PIN	9789-24-7373, 9789-03	-3163	Date:	March 8, 2018
Section A:	: Project Inform	ation			
Project Na	ıme:	Town and University Joint Servi	ces Center		
Property A	Address:	Unaddressed		Zip Code:	27514
Use Group	os (A, B, and/or C):	В	Existing Zoning District:	01-2	
Project Do	Project Description: A rezoning from OI-2 to U-1 to facilitate a Development Agreement that will allow for the Town				
Project be	scription.	Municipal Services Center and f	uture University Uses.		
			386		
Section B:	Applicant, Owr	ner, and/or Contract Purcha	ser Information		
Applic	ant Information	(to whom correspondence will	l be mailed)		
Name:	Aaron Frank, Tov	vn of Chapel Hill Planning & Deve	lopment Services		
Address:	405 Martin Luthe	er King Jr. Blvd			
City:	Chapel Hill	State:	NC	Zip	p Code: 27514
Phone:	919-969-5059	Email:	afrank@townofcha	pelhill.org	
supplie Signature:	ed within this app	ant hereby certifies that, to the dication is true and accurate.	-	dge and b	S/15/18
	wner	aser Information (9789-24-73)	ontract Purchaser		
Name:	The University of	North Carolina at Chapel Hill			
Address:	300 South Buildin	ng - CB #1000			
City:	Chapel Hill	State:	North Carolina	Zip	Code: 27599-1000
Phone:	919-962-3795	Email:	annaw@fac.unc.ed	1	A supportation of the support of the
	ed within this app	ant hereby certifies that, to the lication is true and accurate.	e best of their knowle		elief, all information



ZONING ATLAS AMENDMENT APPLICATION SUBMITTAL REQUIREMENTS

TOWN OF CHAPEL HILL Planning Department

Owner/Contract Purchaser Information (9789-03-3163):

	Owner	□ Co	ntract Purchaser		
Name:	Trustees, University of North Car	rolina at Chapel Hill	Endowment Fund		
Address	300 South Building – CB #1000				
City:	Chapel Hill	State:	North Carolina	Zip Code:	27599-1000
Phone:	919-962-3795	Email:	jpruitt@unc.edu		
The undersigned applicant hereby certifies that, to the best of their knowledge and belief, all information supplied within this application is true and accurate. Signature: Date: 3-15-2018 The following must accompany your application. Failure to do so will result in your application being considered incomplete. For assistance with this application, please contact the Chapel Hill Planning Department (Planning) at (919) 969-5066 or at planning@townofchapelhill.org. For detailed information, please refer to the Description of Detailed Information handout.					
N/A	Application fee (refer to fee schedu	le)		Amount Pa	aid \$ N/A
Yes Pre-Application Meeting – with appropriate staff					
Yes Digital Files – provide digital files of all plans and documents					
Yes	Mailing list of owners of property w	vithin 1,000 foot pe	erimeter of subject prop	erty (see GIS noti	fication tool)
N/A	Mailing fee for above mailing list			Amount Pa	aid \$
Yes	Written Narrative describing the pr	oposal			
Yes Statement of Justification					
Yes	Digital photos of site and surroundi	ng properties			
Yes	Legal description of property to be	rezoned			
Yes	Phasing Plan (if applicable) indication	ng phasing bounda	ries and phasing notes		
Yes	Reduced Site Plan Set (reduced to 8	.5" × 11")			

Plan Sets (10 copies to be submitted no larger than 24" x 36")

Plans should be legible and clearly drawn. All plan set sheets should include the following:

- Project Name
- Legend
- Labels

- North Arrow (North oriented toward top of page)
- Property Boundaries with bearing and distances
- Scale (Engineering), denoted graphically and numerically
- Setbacks
- Streams, RCD Boundary, Jordan Riparian Buffer Boundary, Floodplain, and Wetlands Boundary, where applicable

Area Map

- a) Project name, applicant, contact information, location, PIN, & legend
- b) Dedicated open space, parks, greenways
- c) Overlay Districts, if applicable
- d) Property lines, zoning district boundaries, land uses, project names of site and surrounding properties, significant buildings, corporate limit lines
- e) 1,000 foot notification boundary

Town University Joint Services Center Rezoning Application

Written Narrative

March 14, 2018

Orange County PINs 9789-24-7373, 9789-03-3163, ~20.55 acres

Introduction & Background

The Town has identified the need for a new Municipal Service Center facility and initiated an effort to identify potential sites that would meet the functional, operational, and location needs for several Town departments including the police, parks and recreation administration, fire department administration, health and wellness clinic, ombuds, and training. The Town also recognized this as an opportunity to facilitate increased collaboration, shared training, operations, and public services for Town operations.

The Town tested the capacity of a University-owned site on Estes Drive, recognizing that a coordinated effort could meet long-term interests for both the Town and the University. The site-test shows that there is a significant opportunity to develop a plan for the property that respects the site constraints, the neighboring properties, and complements the planned Carolina North campus. Co-location of certain functions could enhance operations, facilitate shared responses, and encourage joint training and planning for public safety or other uses.

In 2017, Town Council authorized the Town Manager to proceed with a development agreement for the site, and separately in 2017 the University Board of Trustees agreed to consider a lease that would set the groundwork for the Town and University to move forward with a project.

The "property" consists of two unaddressed parcels 9789-24-7373, 9789-03-3163, owned by the University of North Carolina and the University of North Carolina Endowment Fund, respectively. The project area contains approximately 20.55 acres. Of the 9789-24-7373 parcel, the project only encompasses the portion of this parcel that is currently zoned Office Institutional-2.

Property Rezoning & Development Agreement

The property is proposed to be rezoned from Office/Institutional-2 (OI-2) to University-1 (U-1) to allow for a maximum of 200,000 square feet, up to 50% of which may be occupied by the Town. The Town proposes to develop a portion of the property first with the future Town Municipal Services Center in an approximately ~72,000 square foot building. A 99 year lease is proposed and will accommodate the long-term timeline for future development of additional University and Town buildings.

The rezoning is proposed from Office/Institutional-2 to University-1 to facilitate the companion development agreement. A development agreement is an instrument suited to permit long-range implementation of a development program, shared infrastructure between the University and the Town, and a customized set of development standards. The University-1 (U-1) district is designed as a carrying district for a development agreement authorizing public uses such as civic and university uses, which are being proposed. The baseline permitted land uses within a U-1 zoning district are those within the Office/Institutional-4 district, a district designed primarily for University uses. The proposed permitted land uses with this application are refined further from this list to reflect compatibility with neighboring properties and the programmatic needs of the Town and the University. Notably, certain

land uses that are permitted within U-1 but generally regarded as more intense, such as *General Business*, *Service Stations*, will be prohibited by this Development Agreement.

A conceptual site plan accompanies the rezoning application and provides the anticipated general building and infrastructure layout of the site. This site plan was designed with input from adjacent property owners and University representatives. Site specific standards regarding site design and performance standards are proposed in the development agreement that reflect these conversations. Notably, vehicular access is prohibited to the south through the Elkin Hills neighborhood, buildings have been pushed towards Estes Drive, and a 100'+ vegetated buffer is proposed adjacent to the neighborhood to provide a harmonious transition to the adjacent neighborhood.

Draft development agreement standards are provided as an attachment to this application and will be refined further with continued discussion with community members, advisory boards, Town staff, and University staff. Six focused community meetings were held with community members prior to review by advisory boards and commissions.

Project Description

The University-1 zoning district intends for public or private development notably for university and civic uses. While the first user will be the Municipal Services Center the order of following development phases remains to be determined. However, the Municipal Services Project aims to limit the development envelope only to earthwork necessary for the Municipal Services Center and associated infrastructure. The infrastructure included in the first phase of development will include two vehicular accesses (as required for police access), parking only as needed for the Municipal Services Center (to limit development footprint), and stormwater management facilities that will meet the stormwater quantity and quality requirements for the anticipated full build-out by the Town and the University.. Stormwater management is anticipated through bio-retention basins, located at the southern portions of the site. These facilities are planned at depressed areas of the site to accommodate natural drainage patterns to the extent possible within the existing development footprint.

The project is designed with a centralized plaza between the primary buildings of the site, and a drive aisle will encircle the buildings leading to the two vehicular accesses; one of which is planned to match with the UNC park & ride access. Buildings have been brought towards Estes Drive in order to allow for greater buffering from the Elkin Hills neighborhood, and buildings and parking structures are proposed to be constructed into the hillside to limit relative height and land disturbance required for construction.

A 100'+ buffer of existing vegetation is proposed and a buffer is provided adjacent to the intermittent streams located on-site. Sanitary sewer will be connected from the south via Hartig Street, and a break in the buffer is required for this utility connection. A greenway is proposed to connect Justice Street to the Municipal Services Center for a non-vehicular connection as proposed in the Mobility & Connectivity Plan.

Three intermittent streams are located on the property and no development is planned within the 50' RCD except for the internal drive aisle, which is designed to cross the RCD perpendicularly to minimize disturbance.

Rezoning Statement of Justification

Town University Joint Services Center

March 21, 2018

Introduction

The Town and the University of North Carolina are seeking a rezoning from Office-Institutional-2 (OI-2) to University-1 (U-1) to facilitate the development of Town and University services on University-owned land. A rezoning to U-1 is necessary in order to accommodate the Development Agreement proposed between the Town and University. A Development Agreement is the appropriate zoning instrument to facilitate a long-term development partnership between the Town and University. The Town is pursuing a 99-year lease and build-out is anticipated within this period; a Special Use Permit would not allow for a long-term build-out during this timeframe. A total of 200,000 square feet of floor area, of which up to 50% may be occupied by the Town, is proposed across multiple buildings, and the Town anticipates constructing the Municipal Services Center in the immediate future to address Town needs. The remaining balance of square footage will be constructed by the University and the Town within the lease period although a timeline has not been established.

The project exists on two parcels, one of which contains the existing Giles Horney Building and other University services along Airport Drive. A zoning change is only requested for the OI-2-zoned portion of the property.

Compatibility with the Comprehensive Plan

This statement of justification demonstrates how the project aligns with goals of the Comprehensive Plan.

A Place for Everyone

The Municipal Services Center and surrounding grounds will be publicly accessible and the site will feature a pedestrian plaza, trails, and open space that may be used as amenities by the public. A design goal for the project is to create a pleasurable environment both for employees who work at the Municipal Services Center as well as for members of the public who visit the site. Ideally this property will be an amenity for nearby residents as well. The site will be accessible by multiple modes of transportation utilizing a proposed bus stop on Estes Drive, a greenway connection to Justice Street, and bicycle lanes on Estes Drive.

- Family-friendly, accessible exterior and interior places throughout the town for a variety of active uses (PFE.1)
- A creative place to live, work, and play because of Chapel Hill's arts and culture (PFE.2)
- A community of high civic engagement and participation (PFE.5)

Getting Around

The project is sited and designed in a way to promote multi-modal transportation opportunities for employees and visitors of the site. The users of the first phase of project development will be Town

employees, who are encouraged by the Town to use a variety of commuting options. The site is situated to capitalize on multi-modal transportation opportunities in the following ways:

<u>Transit:</u> The project site is located on Estes Drive and directly served by the NU and HS transit routes, and is less than half a mile from Martin Luther King Jr. Blvd, served by NS, G, T, and A routes. A bus stop is planned for on Estes Drive.

<u>Bicycle:</u> The property will be accessed by bicycle through bicycle lanes that may be provided on Estes Drive Extension, and bicycle parking shall be provided to meet Town standards. A Transportation Management Plan shall be submitted and reviewed on an annual basis to meet the Town's satisfaction.

<u>Pedestrian:</u> A greenway is proposed into the site via Justice Street and will provide connectivity and recreational opportunities for users of the municipal services center as well as residents from the surrounding neighborhood. A greenway may also be constructed along Estes Drive to provide future pedestrian connectivity along this roadway.

Vehicular parking shall be built to serve the phases as they are constructed in order to minimize the development envelope.

- A well-conceived and planned, carefully thought-out, integrated, and balanced transportation system that recognizes the importance of automobiles, but encourages and facilitates the growth and use of other means of transportation such as bicycle, pedestrian, and public transportation options (GA.1)
- A connected community that links neighborhoods, businesses, and schools through the provision of greenways, sidewalks, bike facilities, and public transportation (GA.2)
- Create a comprehensive transportation system that provides everybody safe and reasonable access to all the community offers (GA.5)
- A transportation system that accommodates transportation needs and demands while mitigating congestion and promoting air quality, sustainability, and energy conservation (GA.6)

Nurturing Our Community

This project is being designed in an environmentally conscious manner and LEED certification is being pursued. The building siting and orientation is designed to optimize southern building exposure for winter solar gain, and the buildings will feature natural daylighting to minimize energy usage. The building is designed to optimize collaborative and shared spaces for the multiple users of the building, both creating an innovative work environment as well resulting in a more efficient project footprint, resulting in economic and carbon reductions. The general siting of the Municipal Services Center in this location is necessary for police to provide fast response time, as well as to locate other Town services in a centralized location within the Town.

Stormwater management may be provided in bio-retention basins and will meet or exceed the minimum Town stormwater management requirements. Stormwater rate management is anticipated to exceed the 25-year, 24-hour storm event and the stormwater volume management may exceed the 2-year, 24-hour storm event. The final conditions and type of stormwater control measure will be determined when final engineering is performed. The project aims to provide maximum stormwater volume and rate control using bio-retention

basins located within the proposed development footprint. The first one inch of precipitation shall be treated to remove 85% of total suspended solids of all new impervious surfaces resulting from development anticipated at full build-out by the Town and the University, and the project shall meet Jordan Watershed Stormwater requirements for Nitrogen and Phosphorous removal. The Jordan Watershed Stormwater requirements for Nitrogen and Phosphorus are required as this project is located on State owned property and remains subject to these requirements.

A 50' RCD buffer is provided surrounding each of the intermittent streams of the site, and the driveway crossing has been designed to cross the buffer area in a perpendicular manner to reduce encroachment.

Attention has been given to the impact of stormwater runoff, light, noise pollution, and traffic to the adjacent neighborhood. The project will be designed with glare control light fixtures, and use of timed or photocell lights is being evaluated. The location of buildings has been designed close to Estes Drive in response to community feedback about visual impact of buildings to the surrounding properties.

Vehicular traffic is prohibited from the Elkin Hills neighborhood to prevent any traffic impacts to this residential area. A Transportation Impact Analysis has been performed and improvements to Estes Drive including a dedicated center turn lane will be provided.

- Maintain and improve air quality and water quality, and manage stormwater to heal local waterways and conserve biological ecosystems within the town boundaries and the Extra Territorial Jurisdiction (NOC.2)
- Reduce the carbon footprint of all Town-owned or managed services and properties; require that all new development meets standards; and support residents in minimizing their personal footprints (NOC.7)
- Protect neighborhoods from the impact of development such as stormwater runoff, light and noise pollution, and traffic (NOC.8)

Good Places, New Spaces

This project is a result of coordination between the University (property owner) and the Town to utilize the property that fits the needs of both the users, visitors of the property, as well as adjacent property owners. Environmental protection is proposed by consciously limiting the development envelope and maintaining a vegetative buffer along the periphery of the site. This project will serve an integral role in the Town's growth by offering new space for critical Town services, and also serving as an open gathering space accessible to the community.

The proposed use aligns with the future land use map and the form and density is proposed in such a manner to meet Town needs while maintaining compatibility with adjacent properties.

- A joint Town/University development strategy that aligns initiatives for transportation, housing, environmental protection, and entrepreneurial programs (GPNS.4)
- Open and accessible common spaces for community gathering, cultural uses, and community development (GPNS.7)
- Future land use, form, and density that strengthen the community, social equity, economic prosperity, and natural environment (GPNS.8)

Town and Gown Collaboration

This project is a collaborative effort between the Town and University to meet respective organizational interests while respecting the land and adjacent properties. Prior to filing of this rezoning application the Town project team has met several times with community members, University stakeholders, and reported project progress to Town Council to receive feedback.

• The University, the UNC Health Care System, and the Town will coordinate closely to manage development in ways that respect history, traditions, and the environment while fostering revitalization and innovation (TGC.5)

Municipal Services Center Photographs

View of site from Justice Street



View of site from Hartig Street



View of site from Powell Street



View from northeast



View from Southeast



View from Park & Ride



9.78E+09 GOFORTH I PO BOX 20 RALEIGH NC	2.76E+08
9.78E+09 POWELL AS 604 IRONN CHAPEL HILNC	27516
9.78E+09 DAVIS LESL 100 EASTRI CHAPEL HILNC	2.75E+08
9.78E+09 GARRETT N 108 HARDV CHAPEL HILNC	27516
9.78E+09 THACKER C 109 HARDV CHAPEL HILNC	27516
9.78E+09 FARLEY WII 105 HARDV CHAPEL HII NC	27516
9.78E+09 ADALSTEIN 104 EASTRI CHAPEL HII NC	2.75E+08
9.78E+09 HARDY RAL 616 IRONN CHAPEL HILNC	27516
9.78E+09 HEWETT C/ 104 BURLW CHAPEL HILNC	2.75E+08
9.78E+09 SHYAM KAI 101 EASTRI CHAPEL HILNC	27516
9.78E+09 WILLIAMS 102 BURLW CHAPEL HILNC	27517
9.78E+09 BAKER TAN 101 NORTH CHAPEL HII NC	2.75E+08
9.78E+09 GAERTNER 103 EASTRI CHAPEL HII NC	27516
9.78E+09 CHEN TSUN 105 EASTRI CHAPEL HII NC	2.75E+08
9.78E+09 ORANGE U 1220 MAR1CHAPEL HII NC	27514
9.78E+09 POON CHI 102 NORTH CHAPEL HII NC	27514
9.78E+09 BROWN M, 102 MARIG CHAPEL HILNC	2.75E+08
9.78E+09 WRIGHT ST 103 MARIG CHAPEL HILNC	27516
9.78E+09 KUCERA JO 104 NORTH CHAPEL HILNC	27516
9.78E+09 DENT GEOF 106 NORTH CHAPEL HILNC	2.75E+08
9.78E+09 SUBRAMAN 108 NORTH CHAPEL HILNC	2.731+08
9.78E+09 WHITEHEA 104 MARIG CHAPEL HILNC	2.75E+08
9.78E+09 BURK ALBE 105 MARIG CHAPEL HILNC	2.75E+08
9.78E+09 PENDZICH 110 NORTH CHAPEL HILNC	2.75E+08
9.79E+09 WASHINGT 320 SEVERI CHAPEL HILING	27516
9.79E+09 WANG TIAI 301 SEVERI CHAPEL HILNC	27516
9.79E+09 MCCOMBS 8211 TERR/NORTHVILL MI	48167
9.79E+09 AUFDERHA 805 WARD CHAPEL HILNC	27516
9.79E+09 DANIEL JOH 5 OAKHURS DECATUR GA	30030
9.79E+09 SOMERS S/ 405 BARCL CHAPEL HILNC	27516
9.79E+09 HUMPHRE'801 WARD CHAPEL HILNC	2.75E+08
9.79E+09 BROWN GE 304 WESLY Chapel Hill NC	27516
9.79E+09 RUSSELL EL 832 WARD CHAPEL HII NC	27516
9.79E+09 MORSE MA 824 WARD CHAPEL HILNC	27516
9.79E+09 HAWKINS [401 BARCL CHAPEL HII NC	27516
9.79E+09 ODONNELL 808 WARD CHAPEL HII NC	2.75E+08
9.79E+09 WEISENSTE 810 WARD CHAPEL HII NC	2.75E+08
9.79E+09 RUSSELL SC 820 WARD CHAPEL HII NC	27516
9.79E+09 TLALKA IRE 11711 DELI RALEIGH NC	27617
9.79E+09 RICHMONE 4718 TIMB DURHAM NC	27707
9.79E+09 SMITH ROE 2144 RETAICHARLOTTINC	2.83E+08
9.79E+09 HEWITT ST. 804 WARD CHAPEL HII NC	2.75E+08
9.79E+09 ECONOME PO BOX 16 CHAPEL HILNC	27516
9.79E+09 FRANTZ EN 325 BARCL CHAPEL HII NC	27516
9.79E+09 SLEDGE RO 328 BARCL CHAPEL HII NC	27514
9.79E+09 GUNN THO 324 BARCL CHAPEL HII NC	27516
9.79E+09 SCHARLOC 805 POWEI CHAPEL HII NC	2.75E+08

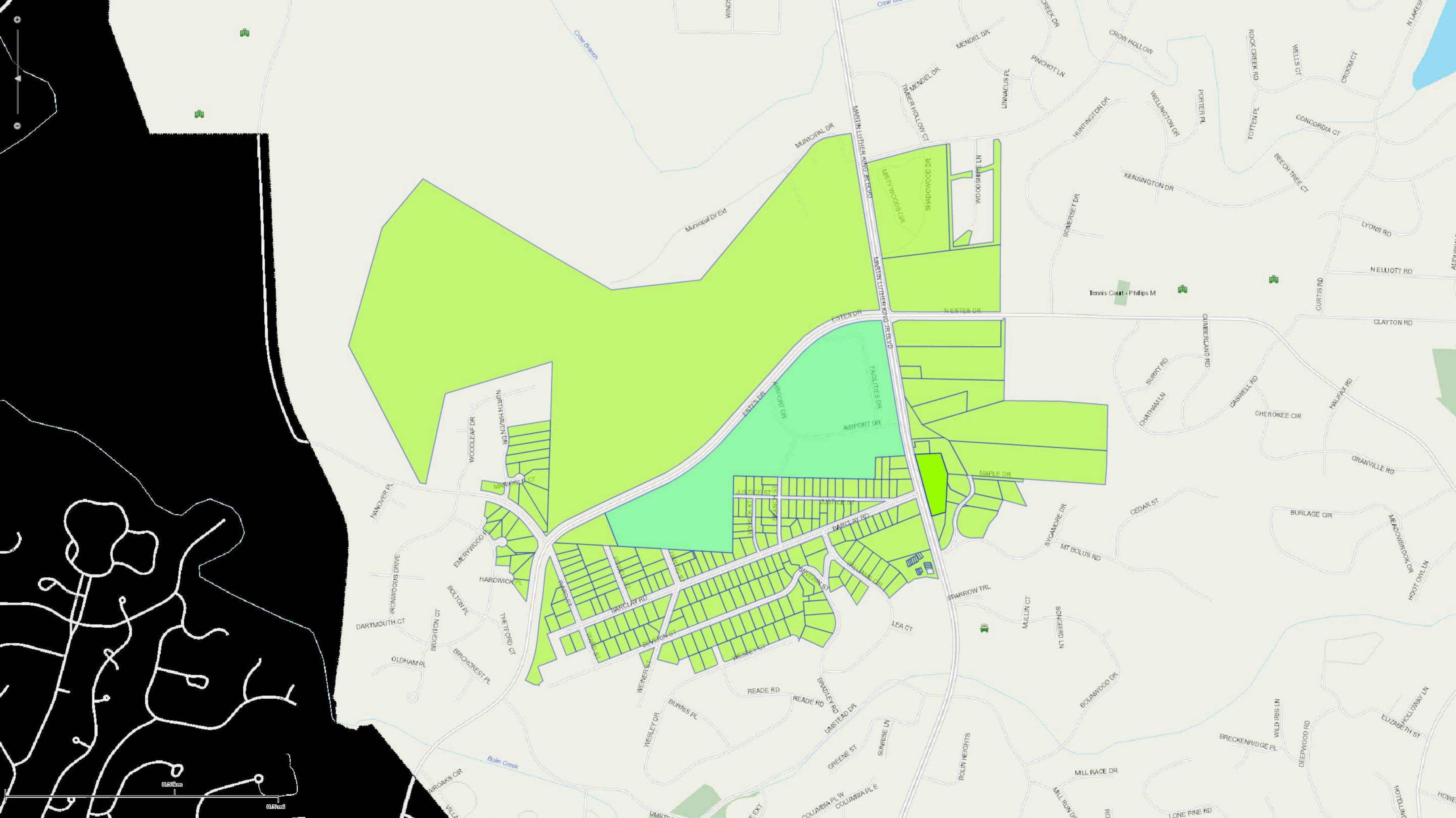
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9.79E+09 DE BRUYN 316 SEVERICHAPEL HII NC	2.75E+08
9.79E+09 BARCLAY W 2422 MOLI ROXBORO NC	27574
9.79E+09 SOUROULL 320 BARCL CHAPEL HII NC	2.75E+08
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9.79E+09 SQUIRES S18 POWELL CHAPEL HILNC	2.75E+08
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9.79E+09 MARKS JAY 3712 HAWICHAPEL HILNC	27516
9.79E+09 MUMBY DI 800 POWEI CHAPEL HII NC	2.75E+08
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9.79E+09 MOORING P O BOX 33LA GRANGINC	2.86E+08
9.79E+09 HINKLE DIA 304 SEVERI CHAPEL HII NC	2.75E+08
9.79E+09 FULCHER L 3931 KELLY DURHAM NC	27707
9.79E+09 FLAXMAN (312 BARCL CHAPEL HILNC	27516
9.79E+09 UNIVERSIT ENDOWME CHAPEL HILNC	27514
9.79E+09 SMITHERS 273 SEVERI CHAPEL HILNC	2.75E+08
9.79E+09 ROMEL JO/ 403 WESLE CHAPEL HII NC	27516
9.79E+09 MCCORMI(401 WESLE CHAPEL HII NC	27516
9.79E+09 MULCAHY, 300 SEVERI CHAPEL HILNC	27516
9.79E+09 SCHWARTZ 2017 S LAK CHAPEL HII NC	27514
9.79E+09 NAIDEN FR 308 BARCL Chapel Hill NC	27516
9.79E+09 NEEBE ALIC 1002 HIGH CHAPEL HILNC	27517
9.79E+09 MELVIN AN 3805 MANI CHAPEL HILNC	27516
9.79E+09 MULCAHY , 300 SEVERI CHAPEL HII NC	27516
9.79E+09 CLEMENTS 301 BARCL CHAPEL HII NC	27516
9.79E+09 HEARN RAN 933 AUTUN CLOVER SC	29710
9.79E+09 BURKE SIGIP O BOX 45 CHAPEL HII NC	27515
9.79E+09 MANTON F PO BOX 14 CARRBORC NC	27510
9.79E+09 LANE NANC 786 WEINE CHAPEL HII NC	2.75E+08
9.79E+09 SKAKLE SYE 269 SEVERI CHAPEL HILNC	2.75E+08
9.79E+09 WILLIAMS 1 WEINER SCHAPEL HII NC	27516
9.79E+09 NEAL ANNE 239 HEMPS CHARLOTTING	
	2.82E+08
9.79E+09 NEAL ANNE 239 HEMPS CHARLOTTING	2.82E+08
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9.79E+09 HUMPHRIE 802 HARTICCHAPEL HII NC	27516
9.79E+09 BLACK SEM 265 SEVERI CHAPEL HII NC	27516
9.79E+09 HACKNEY J 104 CAROL CHAPEL HII NC	27514
9.79E+09 SCOTT JENI 231 BARCL CHAPEL HII NC	2.75E+08
9.79E+09 THIEDA PA 226 BARCL CHAPEL HII NC	27516
9.79E+09 BLACK SEM 265 SEVERI CHAPEL HII NC	27516
9.79E+09 DAYSON CL 257 SEVERI CHAPEL HII NC	27516
9.79E+09 OKUN CAR(260 SEVERICHAPEL HIINC	27516
9.79E+09 VAUGHAN 229 BARCL CHAPEL HII NC	27516

9.79E+09 DIVARIS KII 227 BAF	RCL, CHAPEL HII NC	27516	
9.79E+09 FLASH BET 750 WE	AVI CHAPEL HII NC	2.75E+08	
9.79E+09 PARKS RON 8401 CE	DA CHAPEL HII NC	27516	
9.79E+09 SCHARLOC 253 SEV	'ERI CHAPEL HII NC	27516	
9.79E+09 RAPER JULI 248 SEV	'ERI CHAPEL HII NC	2.75E+08	
9.79E+09 BARFIELD (PO BOX	20 MCCALL SC	27570	
9.79E+09 MOORE AN 220 BAR	RCL, CHAPEL HII NC	27516	
9.79E+09 LEHNER RI(375 WE	SLE CHAPEL HII NC	27516	
9.79E+09 ANDERSON 247 SEV	'ERI CHAPEL HILNC	27516	
9.79E+09 GERARDEN 107 POI	KS CHAPEL HILNC	27516	
9.79E+09 KEARNS KA 223 BAF	RCL, CHAPEL HII NC	2.75E+08	
9.79E+09 OWEN JOH 4002 TR	OP COLUMBIA MO	6.52E+08	
9.79E+09 STANG FRE 218 BAF	RCL, CHAPEL HILNC	2.75E+08	
9.79E+09 CARPENTEI 377 WE	SLE CHAPEL HII NC	27516	
9.79E+09 LYON MILD 241 SEV	'ERI CHAPEL HILNC	27516	
9.79E+09 SOADY SHE 236 SEV	'ERI CHAPEL HILNC	27516	
9.79E+09 BARRY DANPO BOX		27510	
9.79E+09 SIMONSEN 216 BAF	RCL CHAPEL HILNC	27516	
9.79E+09 BRICE CARI 379 WE		27516	
9.79E+09 MATTHEW 235 SEV		27516	
9.79E+09 GARDNER \$4828 OI		2.77E+08	
9.79E+09 SWANSON 229 TH		27278	
9.79E+09 BROEK THC 215 BAF		_	
9.79E+09 SMITH JOA 1505 W		2.77E+08	
9.79E+09 GURGANU P O BO		27278	
9.79E+09 KILLOUGH 208 JUS		2.75E+08	
9.79E+09 CHANTON P O BOX		32302	
9.79E+09 RHODES M 998 WIL			
9.79E+09 BISHOP EL\ 4 WYRIG		27516	
9.79E+09 WILLIAMS 209 JUS		27516	
9.79E+09 LANE GREC 100 BUG	•	27516	
9.79E+09 MILLS FAM P O BOX		27349	
9.79E+09 UNIVERSIT' UNKNO		27514	
9.79E+09 LEDUC BRI/381 WE		27514	
9.79E+09 BEHRENDS 229 SEV		2.75E+08	
9.79E+09 CANOUTAS 224 SEV		2.751+08	
9.79E+09 JOLLEY VIR:115 WC		27713	
9.79E+09 ANDERSON 223 SEV		2.75E+08	
9.79E+09 MCIVER JO 218 SEV		2.7516	
9.79E+09 BAROFF RC 240 BU		27310	
		27512	
9.79E+09 REECE ROB 385 WE			
9.79E+09 SEVERIN ST31 ROG		27514	
9.79E+09 NEAL ANNE 239 HEN		28207	
9.79E+09 BARROW R 209 BAR		2.75E+08	
9.79E+09 STABLER DI 123 PRI		2.75E+08	
9.79E+09 PETTIFOR #389 WE		27516	
9.79E+09 PERKINS R(211 SEV	ERICHAPEL HILNC	27516	

9.79E+09 HANEY MA 208 SEVERI CHAPEL HII	NC 2.75E+08
9.79E+09 BROWN SU 304 WESLE CHAPEL HII	NC 27516
9.79E+09 MILLS NATI PO BOX 67 CARRBORC	NC 27510
9.79E+09 PALIOURAS 393 WESLE Chapel Hill	NC 27516
9.79E+09 MILLS NATI PO BOX 67 CARRBORC	
9.79E+09 MATTHEW 205 SEVERI CHAPEL HIL	
9.79E+09 PACE SHAN 721 BRADL CHAPEL HII	
9.79E+09 HEITSCH D(727 BRADL CHAPEL HII	NC 2.75E+08
9.79E+09 KCC VENTUPO BOX 12 RALEIGH	NC 27605
9.79E+09 MATTHEW: 205 SEVERI CHAPEL HII	NC 27514
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9.79E+09 ROBERTS L 749 WILLIA CHAPEL HII	NC 2.75E+08
9.79E+09 MCLEOD JC 737 BRADL CHAPEL HII	NC 27516
9.79E+09 LEITNER FR 112 TIMBE CHAPEL HII	
9.79E+09 COGGER LI:739 WILLIA CHAPEL HII	
9.79E+09 TORNERO \731 WILLIA CHAPEL HII	
9.79E+09 KEY REBEC 735 WILLIA CHAPEL HII	
9.79E+09 ELLIOTT CA 3 WYRICK SCHAPEL HII	
9.79E+09 WOLF MER 104 BLACK' CARRBORC	NC 2.75E+08
9.79E+09 SAUNDERS 385 MEAD(Chapel Hill	NC 27517
9.79E+09 FEW JAME: 517 ROBIN CHAPEL HII	NC 27516
9.79E+09 HEWETT M 803 BRANC Chapel Hill	NC 27516
9.79E+09 O'KEEFE D/ 805 BRANC CHAPEL HII	NC 27516
9.79E+09 STURGESS 3000 GALL(PITTSBORC	
9.79E+09 RADEMACI 7 BRANCH CHAPEL HIL	
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9.79E+09 THOMAS P. PO BOX 95 Chapel Hill	
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9.79E+09 JAHANNIA 800 BRANC CHAPEL HII	
9.79E+09 BARNEY KR 5 BRANCH CHAPEL HII	NC 27516
9.79E+09 SUMMERS 1923 EPHE CHAPEL HII	NC 27517
9.79E+09 KALOUDIS . 96 LORRAII MONTCLAI	NJ 7043
9.79E+09 OBLER JEFF 1830 17TH WASHINGT	DE 20009
9.79E+09 BROWN GE 304 WESLE CHAPEL HII	NC 27516
9.79E+09 NISBET A P 919 OXBOV CHAPEL HII	NC 27516
9.79E+09 ABERNETH 143 GOAH CHAPEL HIL	
9.79E+09 GERARDEN 107 POLKS Chapel Hill	
9.79E+09 ROBERTS D 749 WILLIA CHAPEL HII	
9.79E+09 CARTER MI 102 SEREN CHAPEL HII	
9.79E+09 KIM EUN A 212 E WINI CHAPEL HII	
9.79E+09 BERRY ANN 132 JUSTIC CHAPEL HII	
9.79E+09 WALKER C/ 716 CASWECHAPEL HII	NC 27514
9.79E+09 GIBSON KE 1755 RIDGIATLANTA	GA 3.03E+08
9.79E+09 CAMPBELL 406 BROAL Carrboro	NC 27510
9.79E+09 SHAPLEY Q 130 JUSTIC CHAPEL HII	NC 2.75E+08
9.79E+09 CRANNY TI 328 UMSTECHAPEL HII	
	NC 27510
	2,310

9.79E+09 FLASH BET 750 WEAVI CHAPEL HII	NC	2.75E+08
9.79E+09 MCADAMS 8505 ORAN CHAPEL HII	NC	2.75E+08
9.79E+09 TAL LLC 1924 MT SI Chapel Hill		27514
9.79E+09 BAKER JAS(124 JUSTIC CHAPEL HIL		27516
9.79E+09 EDMONDS 740 WILLIA CHAPEL HIL		27516
9.79E+09 WHITE JOH 2207 WHIT RALEIGH		2.76E+08
9.79E+09 MERTZ JAN 15541 QUE FORT MYEF		3.39E+08
9.79E+09 DE MARCO 120 JUSTIC Chapel Hill		27514
9.79E+09 SPARROW 727 WILLIA CHAPEL HIL	NC	27516
9.79E+09 SCHNURR E736 WILLIA CHAPEL HIL	NC	27516
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9.79E+09 JOHNSON F734 WILLIA CHAPEL HII	NC	27516
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9.79E+09 CICCONE A 33 SPINNIN MARLTON		8053
9.79E+09 MALINOW'.83 GRASSY CHAPEL HIL		27517
9.79E+09 DINO ROCK 875 MARTI CHAPEL HII		27514
9.79E+09 ELKINS J W 2511 COLG FAYETTEVII		2.83E+08
9.79E+09 DINO ROCk 103 MARIG CHAPEL HII	NC	27516
9.79E+09 A&W REAL 315 PALAF, CHAPEL HIL	NC	27516
9.79E+09 WEHR PETI 300 SUNSE HOLLY SPR	NC	27540
9.79E+09 HUENING LUNIT 19 CHAPEL HII	NC	2.75E+08
9.79E+09 SAVIT PROI 510 APPLE(MEBANE	NC	27302
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9.79E+09 WORLEY DI 9004 ONEA RALEIGH	NC	2.76E+08
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9.79E+09 LI LI 3852 WINCJAMESTOW		27213
	_	
9.79E+09 ZIEMENDO 875 MARTI CHAPEL HIL		27514
9.79E+09 JOHNSON F1549 TAYL(HIAWASSEI		3.05E+08
9.79E+09 JW&S HOLI 2207 WHIT RALEIGH	NC	27608
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9.79E+09 MILLS FAM P O BOX 52SNOW CAN	NC	27349
9.79E+09 MOORE SC 816 PINEHI CHAPEL HII	NC	27517
9.79E+09 STUBBS TH 308 MUIRF MEBANE	NC	27302
9.79E+09 ORTIZ MAR 115 BARCL Chapel Hill	NC	27516
9.79E+09 KAIROS VEI 601 W ROS CHAPEL HII		27516
9.79E+09 RESNICK M 102 GURNE Chapel Hill		27517
9.79E+09 WILLIAMS . 110 JUSTIC CHAPEL HIL		27516
9.79E+09 JOLLEY VIR 115 WOOD DURHAM		27713
9.79E+09 ROBERTS D 749 WILLIA CHAPEL HIL		2.75E+08
9.79E+09 STATE OF N 1321 MAIL RALEIGH	NC	2.77E+08
9.79E+09 STATE OF N 1321 MAIL RALEIGH	NC	2.77E+08

9.79E+09 STRATFORI PO BOX 98 RALEIGH	NC	27624
9.79E+09 PETTY KRIS 100 BARCL CHAPEL HII	NC	27516
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9.79E+09 CHAPEL HII 302 OAK TI CHAPEL HII	NC	27517
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9.79E+09 SULIK MEL/ 199 MERIW VASS	NC	28394
9.79E+09 CEDAR RID PO BOX 13 DURHAM	NC	2.77E+08
9.79E+09 CEDAR RID PO BOX 13 DURHAM	NC	2.77E+08
9.79E+09 CARVER CH 890 MARTI CHAPEL HII	NC	27514
9.79E+09 MEDICAL F CB#7565 8; CHAPEL HII	NC	27516
9.79E+09 FGV VII LLC 870 MARTI CHAPEL HII	NC	27514
9.79E+09 CORNERST SUITE 200 CHAPEL HII	NC	27514
9.79E+09 EMMERSO CUSTODIAI CHAPEL HII	NC	27514
9.79E+09 CHIMCO II 940 MARTI CHAPEL HII	NC	27514
9.79E+09 PEGASUS P SUITE 200 (CHAPEL HII	NC	27514
9.79E+09 SAWMILL EPO BOX 15 CHAPEL HII	NC	27514
9.79E+09 AMITY UNI 825 ESTES CHAPEL HII	NC	27514
9.79E+09 BUTLER KA 404 WOODAPEX	NC	27523
9.79E+09 CHAPEL HII 92 RIVER R SUMMIT	NJ	7901
9.79E+09 GOFORTH J PO BOX 20 RALEIGH	NC	27619
9.79E+09 CHILUKURI 15 MT BOL CHAPEL HII	NC	27514
9.79E+09 KIRK WILLI, 121 MAPLE CHAPEL HII	NC	2.75E+08
9.79E+09 CHARMFOI 2375 CRES SANFORD	FL	32771
9.79E+09 NEXT LEVE 4411 E LA FANAHEIM	CA	92807
9.79E+09 GOFORTH I PO BOX 20 RALEIGH	NC	2.76E+08
9.79E+09 GIJOLU LLC 319 PROVII CHAPEL HII	NC	27514
9.79E+09 PEACE JAM 11020 CHA LOS ANGEL	CA	90077
9.79E+09 SAWMILL [POBOX 15 CHAPEL HII	NC	27514
9.79E+09 CHAPEL HII 980 MARTI CHAPEL HII	NC	27514
9.79E+09 LOTZ DANII 119 MAPLE CHAPEL HII	NC	27514
9.79E+09 RICHARDS(8 KENDALL Chapel Hill	NC	27517
9.79E+09 WU YUE 122 WOOD CHAPEL HII	NC	27514
9.79E+09 COKER WO 117 WOOD CHAPEL HIL	NC	27514



Legal Description: Facilities Parcel Division

Beginning at an iron pipe located in the southern right of way of Estes Drive Extension (S.R. 1780) running thence S 21° 31′ 42″ E 346.01 feet to an iron pipe; thence S 86°46′ 22″ E 1109.76 feet to a point; thence N 00° 41′ 32″ E 744.68 feet to a point; thence N 33° 28′ 00″ W 319.16 feet to a point in the southern right of way of Estes Drive Extension (S.R. 1780); thence with the southern right of way of Estes Drive Extension (S.R. 1780) in a general northwesterly direction along a 2019.02 foot radius curve to the right, said curve having a chord bearing and distance of S 53° 57′ 42″ W 586.01 feet, to a point in the southern right of way of Estes Drive Extension (S.R. 1780); thence continuing with the southern right of way of Estes Drive Extension S 64° 46′ 47″ W 685.46 feet to an iron pipe, the place and point of beginning, containing 18.13 acres, more or less.

Legal Description: Former Dixon Property

Beginning at an iron pipe located in the southern right of way of Estes Drive Extension (S.R. 1780) running thence S 11° 23′ 00″ E 348.61 feet to an iron pipe, thence N 76° 30′ 50″ W 161.71 feet to an iron pipe; thence N 76° 30′ 50″ W 86.10 feet to an iron pipe located in the eastern right of way of Powell Street; thence N 76° 51′ 00″ W 66.45 feet to an iron pipe located in the western right of way of Powell Street; thence N 76° 29′ 00″ W 247.69 feet to an iron pipe; thence N 76° 29′ 00″ W 216.68 feet to an iron pipe located in the southern right of way of Estes Drive Extension (S.R. 1780); thence with the southern right of way of Estes Drive Extension (S.R. 1780) in a general northeasterly direction along a 557.97 foot radius curve to the right, said curve having a chord bearing and distance of N 62° 17′ 17″ E 248.62 feet, to an iron pipe in the southern right of way of Estes Drive Extension (S.R. 1780); thence continuing with the southern right of way of Estes Drive Extension (S.R. 1780) N 75° 09′ 40″ E 321.76 to an iron pipe, the point and place of beginning, containing 2.42 acres, more or less.

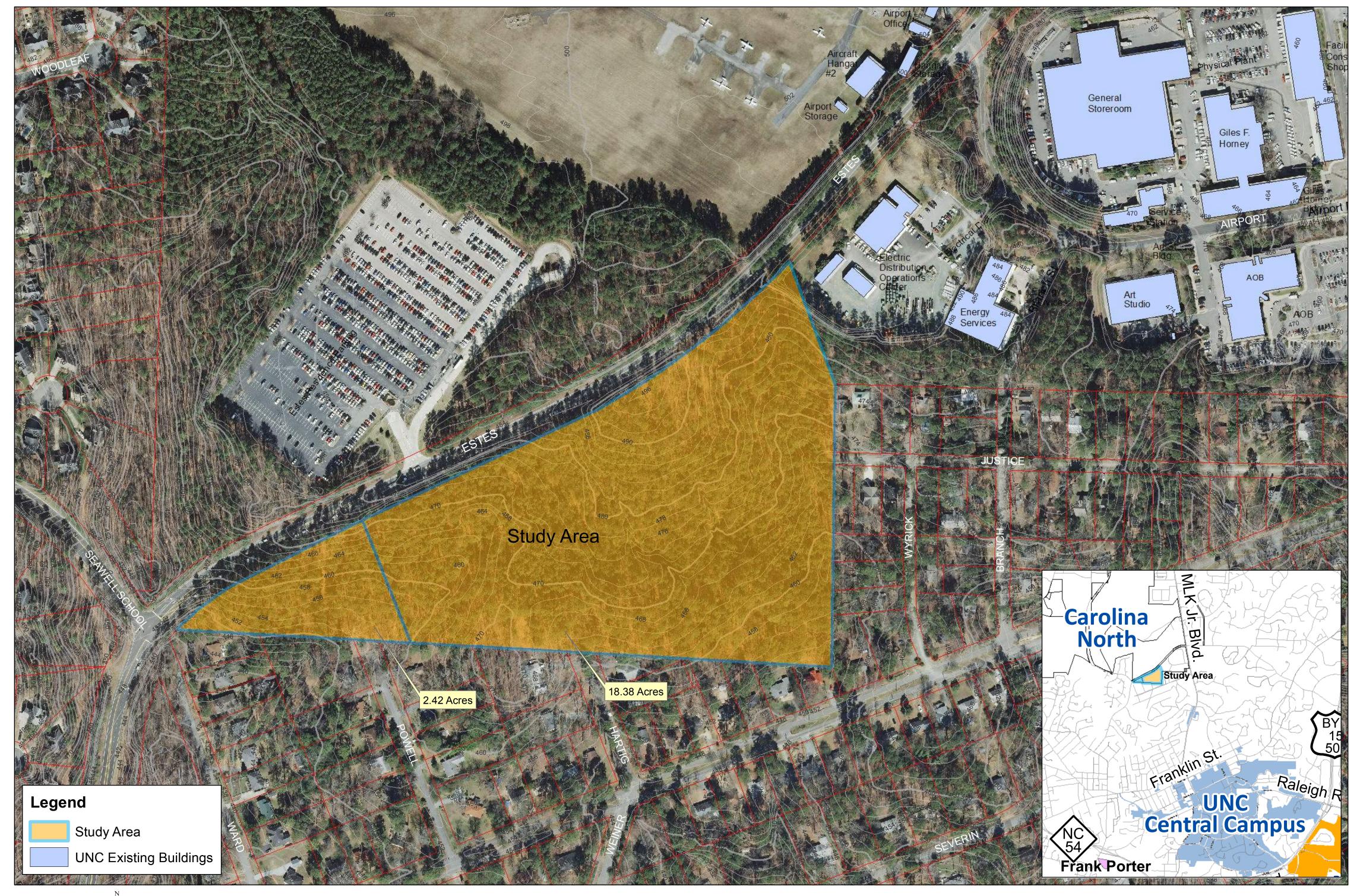






Exhibit I - Community Guiding Principles

Proposed Guiding Principles for the Municipal Services Center Development Agreement DRAFT – 1.18.2018

Residents of the Elkin Hills neighborhood and other concerned citizens who have participated in meetings about the municipal services center request that the following guiding principles be incorporated into the development agreement for this project. These guiding principles take into consideration prior documents adopted by UNC-related entities.¹

Guiding Principles

- A. After discussions with residents, the Town and the University, the final agreed-upon principles will be voted on by the Chapel Hill Town Council and incorporated into the development agreement as its guiding principles. These principles will also serve as one of the design drivers for the project.
- B. The Town and the University will continue to consult with and seek feedback from the neighborhood if and when any additional buildings beyond the municipal services center building are under consideration.

C. Site

- 1. Preserve in perpetuity at least 50% of the site² as natural, non-fragmented and contiguous space³ serving as both a buffer to the neighboring residential properties, and as preservation of the natural environment for wildlife and a sense of forest and greenspace.
- 2. Allow a maximum of 50% to be razed for development.

D. Design with Empathy

- 1. The impact on neighbors should be kept front and center in all planning and implementation.
- 2. The project should not decrease the market values of the residential properties adjacent and near-adjacent to the site. The residents request the Town to seek a qualified opinion from an impartial, licensed appraiser.
- 3. Manage development of the site to minimize impacts on adjacent neighborhood and the environment,⁴ using measurable indicators of impact, including both lighting and noise impacts (e.g., sirens used by the fire and police departments) and maximize aesthetics in accordance with the high standards set by the University.
 - a) Situate buildings away from the neighborhood and by preference along Estes Drive Extension with minimum setbacks from the street.
 - b) Extend the 100-foot required buffer to at least 200 feet.
 - c) Give strong preference to higher buildings rather than lower structures that cover more surface area.
 - d) Restrict the use and storage of hazardous materials such as fuels and chemicals.
 - e) Limit the use of fences.

E. Environmental Leadership

- 1. Demonstrate a leadership position in environmentally sensitive development regarding air quality, and energy production and consumption.⁵⁶
 - a) Construct buildings to meet high environmental standards, such as the AIA-2030 carbon reduction energy performance standards as recommended to the Town by the Environmental Stewardship Advisory Board (ESAB).⁷
 - b) Give strong preference to a multi-storied parking garage or parking under each building on the site, rather than expanded surface parking.

- c) Construct surface parking which optimizes storm water control including the use of permeable surfaces wherever possible.
- d) Where technically possible, incorporate green energy options into site design, e.g., solar panels on roofs, solar carports over surface parking areas.
- 2. Demonstrate a leadership position in environmentally sensitive development regarding landscaping and maintaining natural areas.
 - a) Retain as many mature and medium-height native trees and hardwoods as possible, particularly between the developed area and the buffer for the Elkin Hills neighborhood.⁸
 - b) Use only native plants in new and replacement landscaping for the site (as recommended by UNC's NC Botanical Garden).

F. Exceed Storm Water Requirements

- 1. Demonstrate a leadership position regarding sustainable water management, waste water treatment and reuse,⁹ and creek water quality.
 - a) Exceed storm water management regulations on the site to eliminate or greatly decrease the risk of flooding and damage to property adjacent to and downstream from the site. Ensure that development of the site will result in no net increase in storm water discharge¹⁰ and flooding of the adjacent neighborhoods, and no net increase in loading of sediment and nutrients into local streams.¹¹
 - b) Reclaim and re-use rain and storm water (e.g., flushing within buildings, watering of vegetation, etc.).
 - c) Take prudent and reasonable steps, including improvement of stream channels in the Elkin Hills neighborhood and other neighborhoods downstream from the site, to improve management of run-off and to limit impact on Bolin Creek.

¹ These documents include: A) Carolina North 2007 Plan; B) REPORT OF THE LEADERSHIP ADVISORY COMMITTEE FOR CAROLINA NORTH January 19, 2007; C) UNC response to Horace Williams Citizens Committee report, 25 January 2006, Natural areas/parks and recreational facilities; D) Faculty Council Resolution 2002-6. Urging the University Administration to Commit Itself to Sustainability Measures in its Institutional Policies and Practices. PROPOSED BY THE BUILDINGS AND GROUNDS COMMITTEE (April 4, 2002).

² UNC response to Horace Williams Citizens Committee report, 25 January 2006, Natural areas/parks and recreational facilities, Principle 1, p. 5

³ REPORT OF THE LEADERSHIP ADVISORY COMMITTEE FOR CAROLINA NORTH, January 19, 2007, V. OPEN SPACE, NATURAL AREAS, PARKS AND RECREATION, p. 5

⁴ UNC response to Horace Williams Citizens Committee report, 25 January 2006, Development principle 1, p. 4

⁵ REPORT OF THE LEADERSHIP ADVISORY COMMITTEE FOR CAROLINA NORTH, January 19, 2007, V. OPEN SPACE, NATURAL AREAS, PARKS AND RECREATION, p. 4

⁶ Three zeros environmental initiative; Frequently asked questions; https://threezeros.unc.edu/faq/

⁷ CAROLINA NORTH PLANNING PROCESS SUMMARY OF KEY INTERESTS BY CATEGORY [FEBRUARY 11, 2009], p. 184

⁸ Sustainability at UNC. Grounds. https://sustainability.unc.edu/initiatives/operations/grounds/

⁹ UNC response to Horace Williams Citizens Committee report, 25 January 2006, Water and Sewer / Stormwater Management / Air Quality Principle 1, p. 5

¹⁰ UNC response to Horace Williams Citizens Committee report, 25 January 2006, Water and Sewer / Stormwater Management / Air Quality Principle, Principle 2, p. 5

¹¹ REPORT OF THE LEADERSHIP ADVISORY COMMITTEE FOR CAROLINA NORTH, January 19, 2007, Environmental principles, p. 4

Exhibit J - Stream Determination



PUBLIC WORKS DEPARTMENT STORMWATER MANAGEMENT DIVISION

405 Martin Luther King, Jr. Blvd. Chapel Hill, NC 27514-5705 Telephone (919) 969-7246 Fax (919) 969-7276 www.townofchapelhill.org

February 28, 2018

Mr. Matthew West Dewberry 2610 Wycliff Road, Suite 410 Raleigh, North Carolina 27607 mwest@dewberry.com

RE: Stream Determination for undeveloped parcel owned by UNC Endowment Fund on Estes Drive Extension, Chapel Hill, NC (PIN 9789-03-3163)

Dear Mr. West:

As requested, the Town Public Works Department has performed a stream determination for the property identified on the attached forms. This determination indicates whether different types of streams (perennial, intermittent, and/or ephemeral) or perennial waterbodies are present on the property in question or on nearby properties. These streams and their classifications are shown on the accompanying map. Stream segments regulated by the Town's Jordan Lake Watershed Riparian Buffer regulations are highlighted. Locations of all features on the map are approximate and must be field surveyed for precise location.

This stream determination information is used to determine the location and extent of the Resource Conservation District (RCD) and Jordan Lake Watershed Riparian Buffers. Specific land use regulations and restrictions apply within the boundaries of these protected areas. If you are considering any kind of work on this property, including clearing vegetation, paving, grading, or building, please consult with the Town Planning Department to determine the possible extent of the Resource Conservation District (RCD) and Jordan Lake Watershed Riparian Buffer on this property and the applicable corresponding regulations.

This stream determination will remain in effect for five years from the date of the site visit, after which a new stream determination with site visit will be required.

In accordance with the Town's procedures, you may appeal this administrative decision to the Town Manager. If you wish to do so, you must file your written appeal accompanied by any materials you believe support your appeal, within **30 days** of receipt of this letter.

If you have questions regarding stream determinations, please contact me at (919) 969-7202 or aweakley@townofchapelhill.org. If you have questions regarding the Town's Resource Conservation District (RCD) or the Jordan Watershed Riparian Buffer regulations, please contact the Planning Department at (919) 968-2728, or view information online at: http://www.townofchapelhill.org/town-hall/departments-services/public-works/stormwater-management/regulations-ordinances.

Regards,

Allison Schwarz Weakley Stormwater Analyst

AllisonWeakley



PUBLIC WORKS DEPARTMENT STORMWATER MANAGEMENT DIVISION

405 Martin Luther King, Jr. Blvd. Chapel Hill, NC 27514-5705 Telephone (919) 969-7246 Fax (919) 969-7276 www.townofchapelhill.org

STREAM DETERMINATION SITE VISIT RESULTS

Property Information	
Parcel ID Number (PIN)	Address / Location Description
9789-03-3163	Estes Drive Extension
These are the results of a site visit t conducted on 2/23/2018 by Town Sta	to the properties listed above for a stream determination aff:
☐ No perennial, intermittent, or ep on or near the property(ies) in questi	hemeral streams or perennial waterbodies were identified on.
igtieq Perennial, intermittent, or ephem or near the property(ies) in question	neral streams, or perennial waterbodies, were identified on and shown on the attached map(s).
Riparian Buffers, and their approxi	Town flow classifications, presence of Jordan Watershed mate locations is attached. Origins or breakpoints that marked on the map. Stream classification forms and are also attached.
Other conditions exist which may af Jordan Watershed Riparian Buffer:	fect the location of the Resource Conservation District or
	ne area. Precise location of the Base Flood Elevation and strict must determined by a field survey commissioned by
_ :	ittent stream are piped in the area, as shown on the map. ciated Jordan Watershed Riparian Buffer.
	have been identified in the area. A formal review by a nal Wetland Delineation is recommended if impacts to
AllisonWeakley	
Town Staff Signature	



Town Staff signature

PUBLIC WORKS DEPARTMENT STORMWATER MANAGEMENT DIVISION

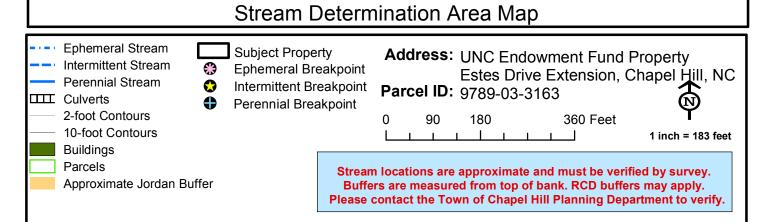
405 Martin Luther King, Jr. Blvd. Chapel Hill, NC 27514-5705 Telephone (919) 969-7246 Fax (919) 969-7276 www.townofchapelhill.org

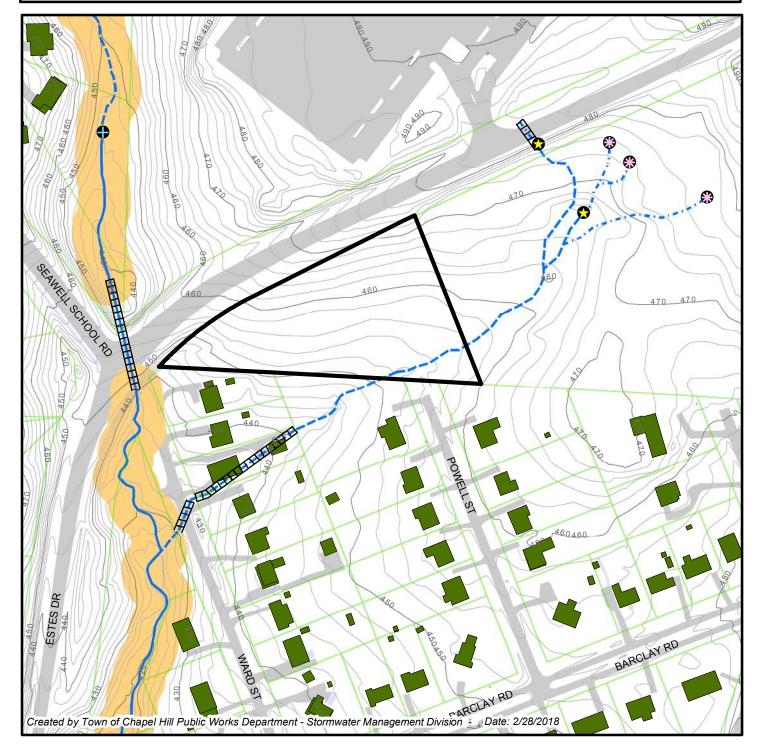
STREAM DETERMINATION RECORDS REVIEW

Property Information		
Parcel ID Number (PIN)	Address / Location Description	
9789-03-3163	Estes Drive Extension	
-	ISGS 1:24,000 Topographic maps, and County Soil Survey maps, I mination will be required for the property(ies) listed above for	
classification or determination, or unide	poodies, streams or waterbodies identified as requiring a new ntified flowlines (possible streams) are shown within 150 feet of s GIS, the USGS 1:24,000 Topographic map, or the County Soil	
	boundary was set on a recorded final plat for the property in waterbodies shown on the USGS 1:24,000 Topographic map or e property.	
\triangle A stream determination has been done for this property, a property uphill or upstream, or a nearby property as of February 23, 2013 or later, and that stream determination applies to this property. A copy of the documentation for the relevant site visit(s) is available upon request.		
Relevant PIN(s): <u>9789-24-7373 (site visit</u>	10-1-2016)	
•	r Town flow classifications, presence of Jordan Watershed of locations is attached. Origins or breakpoints that have been hap.	
Other conditions exist which may affec Watershed Riparian Buffer:	t the location of the Resource Conservation District or Jordan	
	rea. Precise location of the Base Flood Elevation and associated determined by a field survey commissioned by the owner or a	
Segments of perennial or intermitten associated Jordan Watershed Riparian B	nt stream are piped in the area. These segments do not have an uffer.	
	e been identified in the area. A formal review by a professional ation is recommended if impacts to wetlands are likely.	
AllisonWeakley		
	2/28/2018	

Date

Version 6/5/2017





USGS 24K Topographic / County Soil Survey Maps

Subject Property

150 300 450 600 Feet

Address: UNC Endowment Fund Property

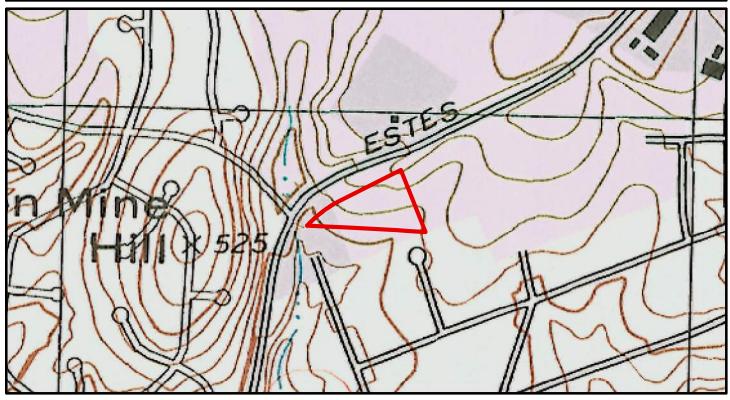
Estes Drive Extension, Chapel Hill, NC

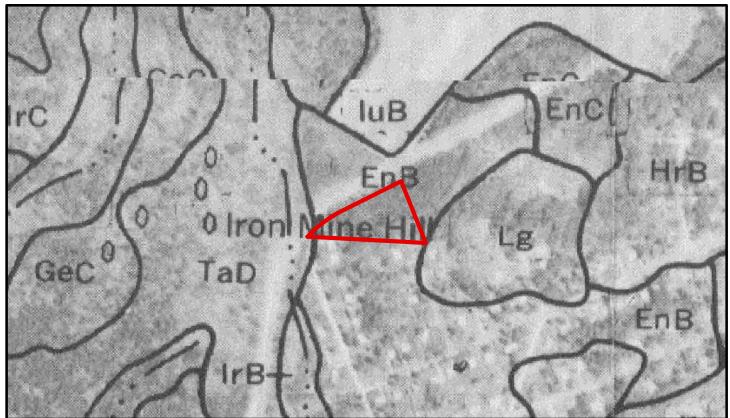
Parcel ID: 9789-03-3163

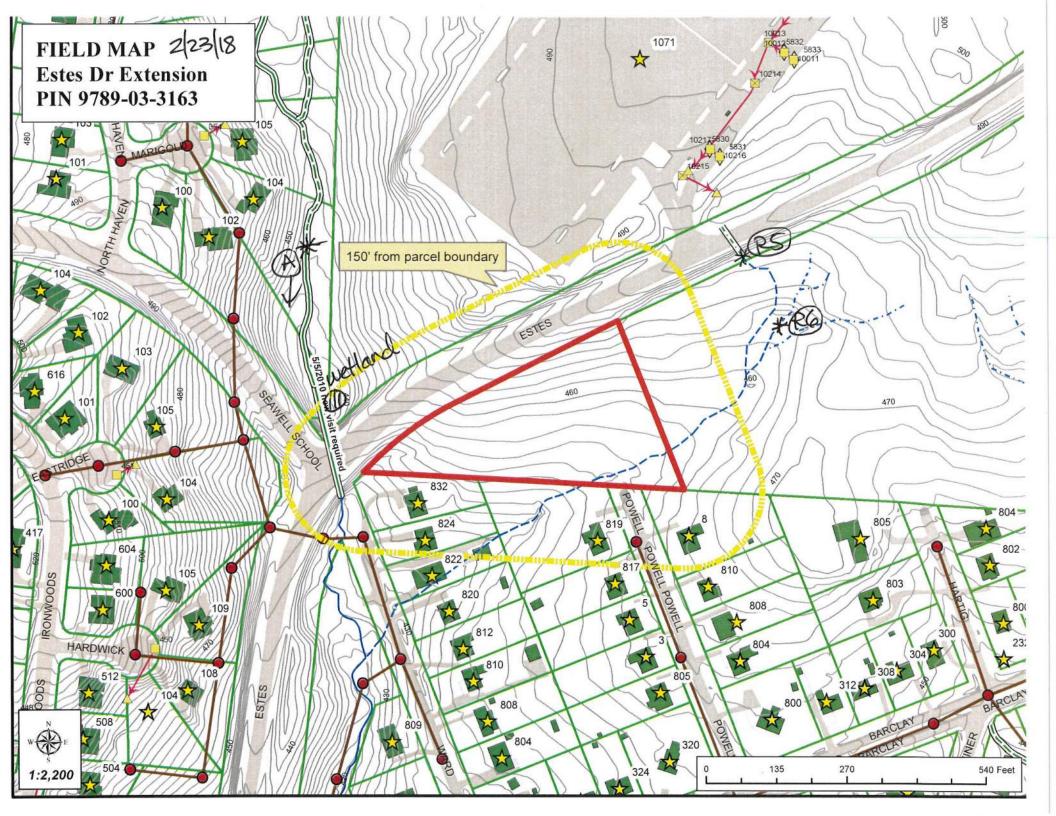


1 inch = 500 feet

Created by Town of Chapel Hill Public Works Department - Stormwater Management Division- 2/26/2018

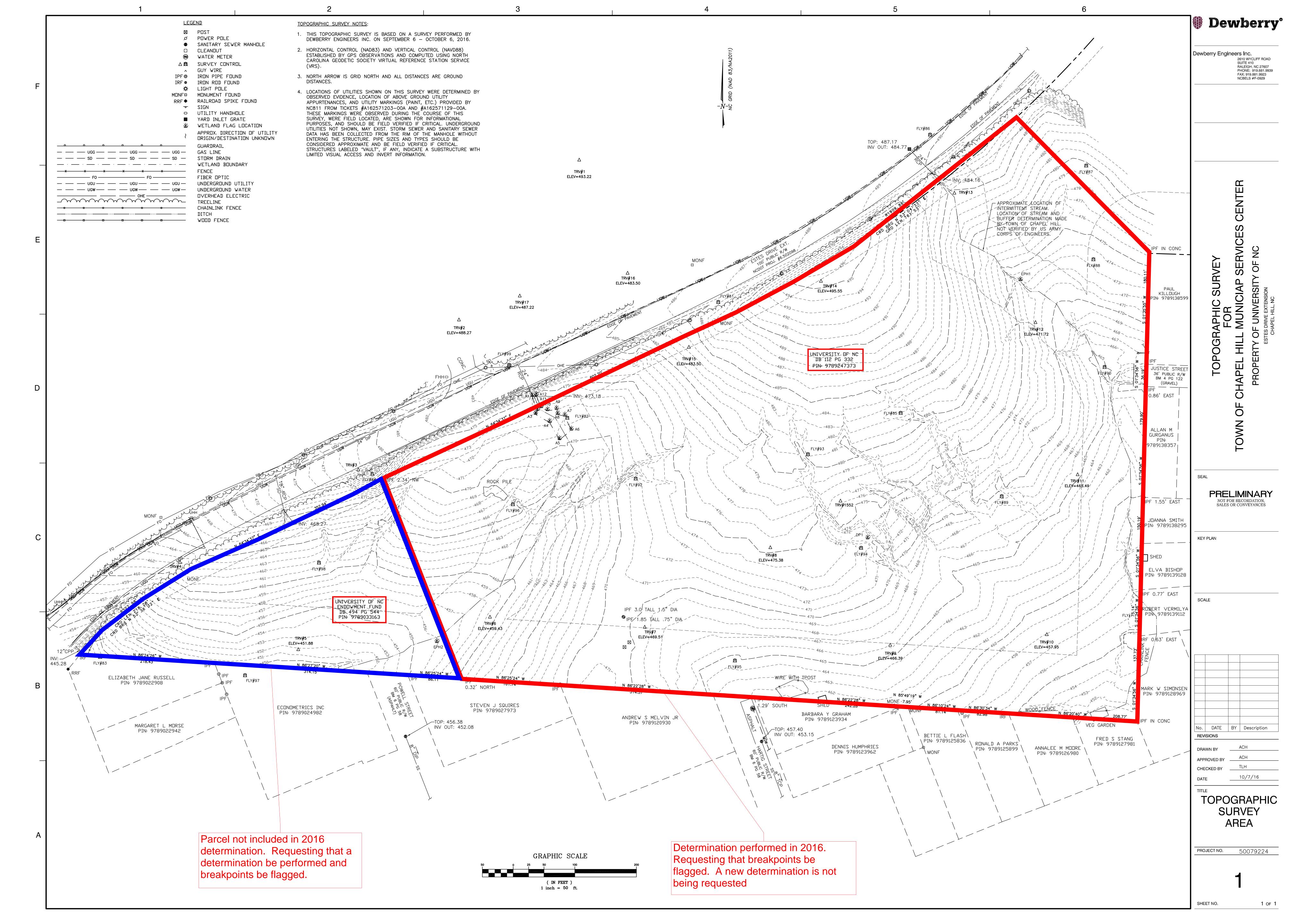






201802231134

Feature 1 Estes Dr. Ext NC DWO Stream Identification Form Version 4.11 Date: Project/Site: Latitude: MUER County: Evaluator: Longitude: -**Total Points:** Stream Determination (circle one) Other Stream is at least intermittent Ephemeral Intermittent Perennial e.g. Quad Name: if ≥ 19 or perennial if ≥ 30* Weak Absent Moderate A. Geomorphology (Subtotal = Strong 1a. Continuity of channel bed and bank 0 2 3 2. Sinuosity of channel along thalweg 0 1 2 3 3. In-channel structure: ex. riffle-pool, step-pool, 2 0 3 1 ripple-pool sequence 0 3 4. Particle size of stream substrate 2 5. Active/relict floodplain 0 1 2 3 (1 6. Depositional bars or benches 0 3 0 2 7. Recent alluvial deposits 3 0 1 8. Headcuts 3 4(1) 9. Grade control 0 0.5 1.5 0 1 10. Natural valley 0.5 1.5 11. Second or greater order channel No = 0Yes = 3 artificial ditches are not rated; see discussions in manual B. Hydrology (Subtotal = 12. Presence of Baseflow absent upper reach 0 in lower feac 13. Iron oxidizing bacteria 0 1 3 14. Leaf litter 1.5 1) 0.5 0 15. Sediment on plants or debris 0 0.5 (1.5) 16. Organic debris lines or piles 0 0.5 1.5 17. Soil-based evidence of high water table? No = 0Yes = 3 C. Biology (Subtotal = 18. Fibrous roots in streambed 0 2 3 19. Rooted upland plants in streambed 1 0 20. Macrobenthos (note diversity and abundance) 0 1. 2 3 21. Aquatic Mollusks 0 2 3 22. Fish 0 0.5 1 1.5 23. Crayfish 0 0.5 1 1.5 24. Amphibians 0 0.5 1.5 25. Algae a Oundant in lower 0 0.5 1.5 26. Wetland plants in streambed FACW = 0.75; OBL = 1.5 Other = 0 *perennial streams may also be identified using other methods. See p. 35 of manual Notes: Sketch: Feature begins cheadout (flagged)





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PUBLIC WORKS DEPARTMENT STORMWATER MANAGEMENT DIVISION

405 Martin Luther King, Jr. Blvd. Chapel Hill, NC 27514-5705 Telephone (919) 969-7246 Fax (919) 969-7276 www.townofchapelhill.org

10/7/2016

Cindy Hoffman, PLA, A SLA Dewberry 2610 Wycliff Road, Suite 410 Raleigh, NC 27607-3366

RE: Stream Determination for Parcel #9789-24-7373

Dear Ms. Hoffman:

As requested, the Town Public Works Department has performed a stream determination on the property identified on the attached forms. This determination indicates whether different types of streams (perennial, intermittent, and/or ephemeral) or perennial waterbodies are present on the property in question or on nearby properties. These streams and their classifications are shown on the accompanying map. Stream segments regulated by the Jordan Lake Stream Buffer ordinance are highlighted. Locations of all features on the map are approximate and must be field surveyed for precise location.

This stream determination information is used to determine the location and extent of the Resource Conservation District and the Jordan Lake Watershed Riparian Buffer. Specific land use regulations and restrictions apply within the boundaries of these protected areas. If you are considering any kind of work on your property, including clearing vegetation, paving, grading, or building, please consult with the Town Planning Department to determine the possible extent of the Resource Conservation District and Jordan Lake Watershed Riparian Buffer on your property and corresponding regulations.

This classification will remain in effect for five years from the date of the site visit before a request for reclassification will be considered, unless the stream channel characteristics are significantly altered as a result of watershed changes.

In accordance with the Town's procedures, you may appeal this administrative decision to the Town Manager. If you wish to do so, you must file your written appeal accompanied by any materials you believe support your appeal, within <u>30</u> days of receipt of this letter.

If you have questions regarding stream determinations, please contact me at (919) 969-5083. If you have questions regarding the Town's Resource Conservation Districts or the Jordan Watershed Riparian Buffer regulations, please contact the Planning Department at (919) 968-2728, or view information online at http://www.townofchapelhill.org/town-hall/departments-services/public-works/stormwater-management/regulations-ordinances.

Regards,

Dave Milkereit Stormwater Specialist



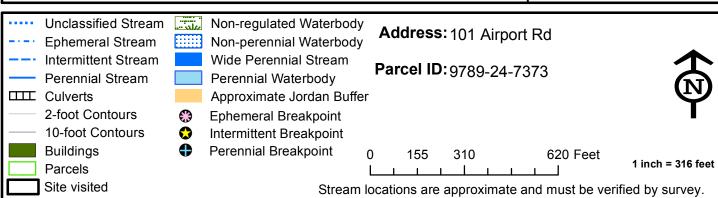
PUBLIC WORKS DEPARTMENT STORMWATER MANAGEMENT DIVISION

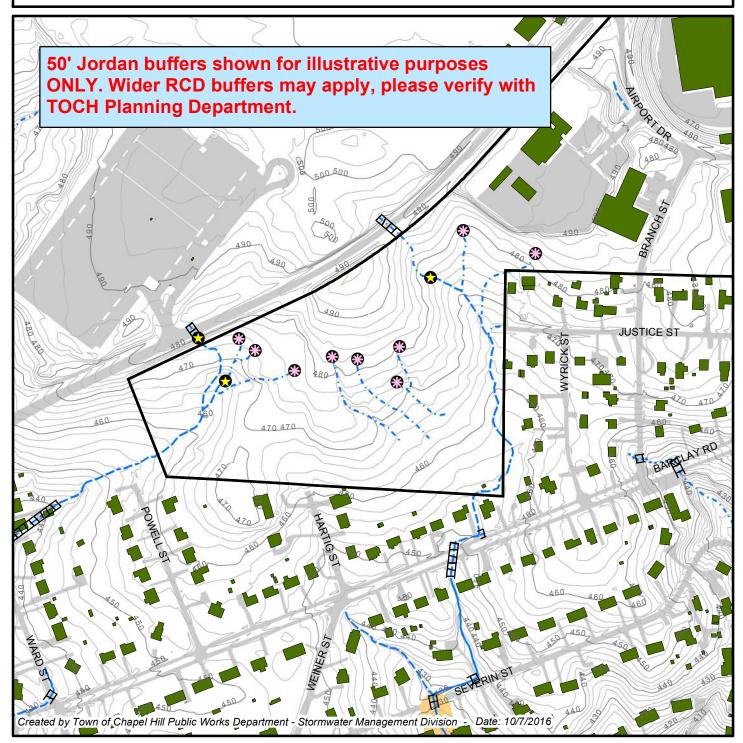
405 Martin Luther King, Jr. Blvd. Chapel Hill, NC 27514-5705 Telephone (919) 969-7246 Fax (919) 969-7276 www.townofchapelhill.org

STREAM DETERMINATION SITE VISIT RESULTS

Property Information	
Parcel ID Number (PIN)	Address / Location Description
9789-24-7373	101 Airport Rd/Estes Dr Extension
	t to the properties listed above for a stream 10/6 & 10/7/2016 by Town Staff:
☐ No perennial, intermittent, or e identified on or near the property(i	phemeral streams or perennial waterbodies were es) in question.
· · · · · · · · · · · · · · · · · · ·	emeral streams, or perennial waterbodies, were es) in question and shown on the attached map(s).
Jordan Watershed Riparian Buf Origins or breakpoints that have	their Town flow classifications, presence of fers, and their approximate locations is attached. e been flagged in the field are marked on the map. I additional site visit notes and maps are also
Other conditions exist which may a District or Jordan Watershed Ripa	affect the location of the Resource Conservation rian Buffer:
	the area. Precise location of the Base Flood ce Conservation District must determined by a field er or a representative.
•	mittent stream are piped in the area, as shown on the re an associated Jordan Watershed Riparian Buffer.
	ds have been identified in the area. A formal review dictional Wetland Delineation is recommended.
	10/7/2016
Town Staff signature	date

Stream Determination Area Map





USGS 24K Topographic / County Soil Survey Maps

s s

Site Parcel Boundary

190 380 570 760 Feet

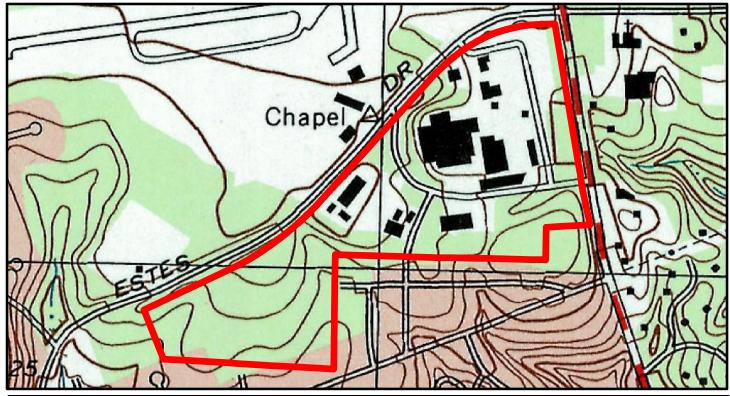
1 inch = 625 feet

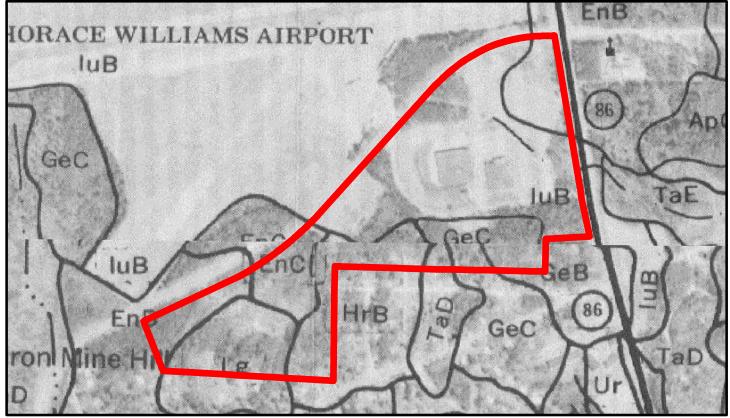
Address: 101 Airport Rd

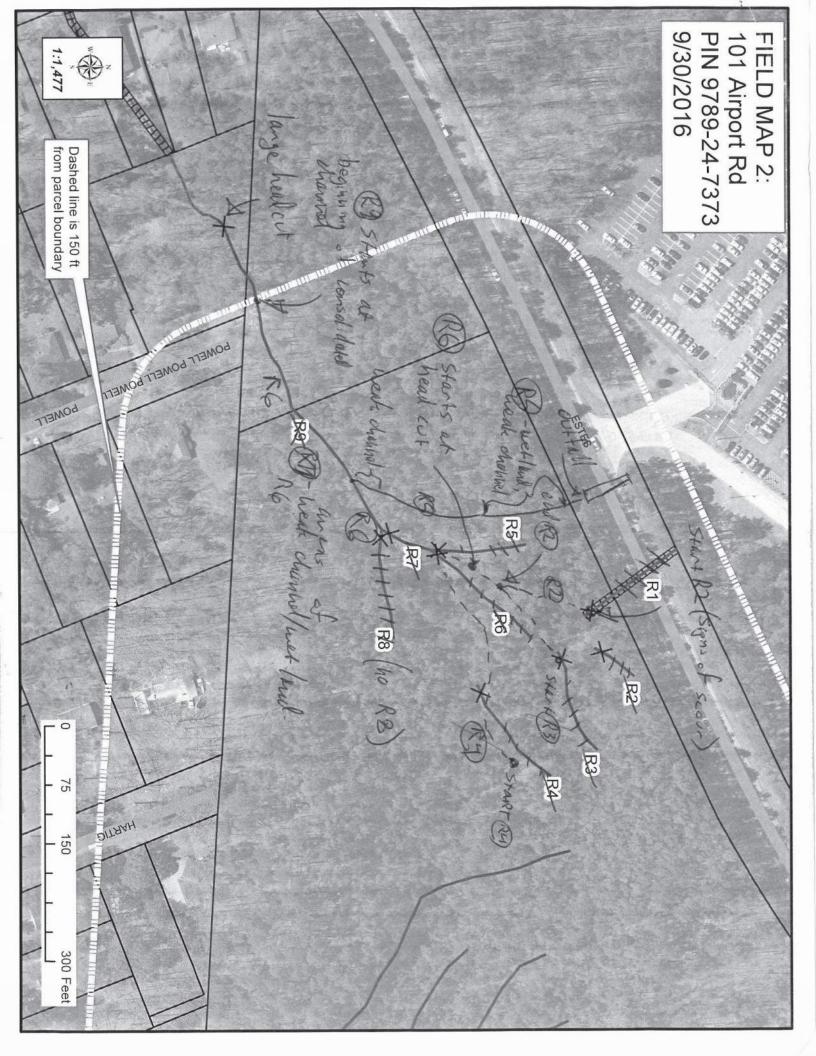
Parcel ID: 9789-24-7373

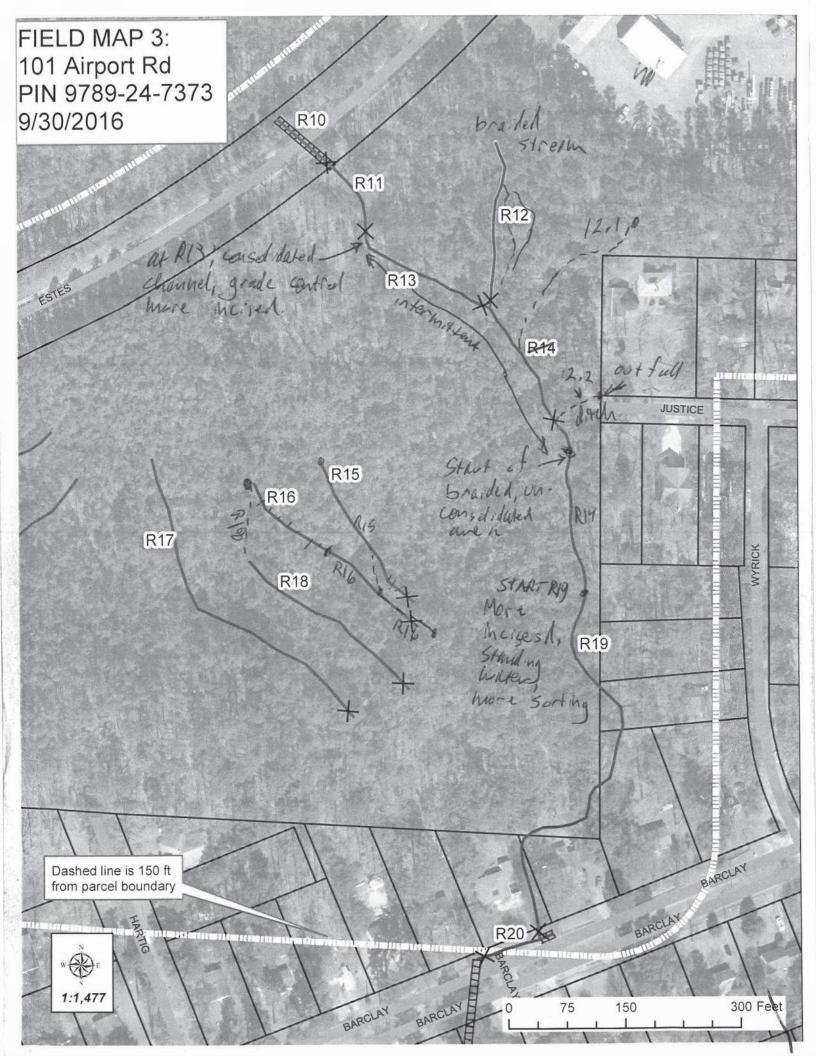


Created by Town of Chapel Hill Public Works Department - Stormwater Management Division- 9/29/2016









Date	Project Site 9789 - 24 - 7373			Latituda		
Evaluator: DM	County:			Longitude		
Total Points: Stream is at least intermine it 28, 15 fiz 18 or derennia fiz 301	Stream Determination (sircle one) Ephemeral Intermittent Perennial			Other Leg Guas Name:		
A. Geomorphology Subtatal = 16.5,	Absent		Weak		Moderate	Strong
11 Continuity of channel bed and park	2		1		(3)	3
2. Sinupsity of phannel along thalway	0		1		2	3
 In-channel structure lek riffe-pool step-pool rippie-pool sequence 	ð		1		2	3
4. Particle size of stream substrate	. 0	- 6	0		2	3
5. Active reliat ficodolain	٥	1	11		2	3
8. Depositional bars or benones	Ö		(1)	Tool-leville	2	3
7. Recent alluvia, decosits	9		0	130	2	1 3
B. Headouts	9		1		2	(D)
9 Grade control	0		0.5		<u> </u>	1.5
10. Natural valley	0		(5)		1	1.5
11. Second or greater order channel	No(€ 2) Yes				= 3	
familidial ditores are not rated, see dispussions in manual. B. Hydrology (Subtotal =						
12. Presence of Basefow	0		1		2	3
13 Iron oxidizing bacteria	(9)		1		2	3
14. Lear litter	1.5	1	U	3	0.5	0
15. Sediment on stanta or decris	0		03			1 5
18. Organic depris lines prolles	0		0.5		1	1.3
17. Soil-based evidence of high water table?		NO =	0	91	r'as	(E3)
C. Biology (Subtotal =- 6.75)		*	k			
13 Fibrous roots in streamped	+ 3		(3)	1	1	1 0
19. Rooted upland plants in streamped	3	. E	(2)		1	2
20. Macrobenthos inote diversity and abundance)	0	30	1		2	3
21. Aquatic Mailusks			1		2	3
22. Fish	0		0.5		1	1.5
23 Crayfish	3		0.5		1	1.5
21. Amonibians	<u>(2)</u>	3	0.5	1	1	1.5
25 Aigae	Ď		0.5		0	15
23. Wetland crants in streambed	1		FACW = 0.7) CB	_= 15 Other=	: 0
A CONTROL OF A DESCRIPTION OF THE SECOND OF	oda Seelo 35 of ma	nual.				
foerennial streams may also be dentified using other metri-						

Sieton 26: hiero stegerm, shidax

Date: (0/6/16	Project Site 9789 - 2 - 7373 County: Stream Determination (circle one) Ephameral Intermittent Perennial			Longitude		
Evaluator: DM						
Total Points: Stream is at least infamilie 1 17. 76 in 18. 19. 31. 94. 17. 17. 17. 17. 17. 18. 19. 19. 19. 19. 19. 19. 19. 19. 19. 19						
A. Geomorphology (Subtatal = / /	Absent	1	Weak	Moderate	Strong	
13 Continuity of channel bed and park	0		(V)	2	3	
2. Sinupsity of phannel along the weg	0	T.	1	(2)	3	
3 In-grannel structura ex riffe-cool step-pool mobie-opol seguence	0		0	2	3	
4 Particle size of stream substrate	0		0	2	3	
5. Active religtificado a n	0		0	2	3	
6. Depositional bars or benches	0		1	2	3	
7. Recent alluvial decesits	. 0		1	2	3	
3. Headcuts) 0		1 1	(2)	3	
9 Grade control	0	1	0.5	0	1.5	
13 Natural valley	(0)		0.5	1	1.5	
11. Second or greater order channel		Not	3	Y = 3		
fartificial ditories are not rated, see plactuasions in manual B. Hydrology (Subtotal =)						
12. Presence of Basefoliu	(0)		1	2	3	
13 Iron oxidizing pasteria			1	2	3	
14 Leaf litter	1.5	1	4 - 4	(05).	0	
15 Sediment on plants or decris	0		0.5	1	1.5	
13 Organic depris lines profes	0	- 1	0.5	1	(15)	
17. Soil-based evidence of high water table?	No = 0		fas	= 1		
C. Biology (Subtotal = 4, 15)			1			
13 Fibrous roots in streamped	† 3		(2)	(1)	0	
19. Rooted upland plants in streambed	1 3		2	CD	2	
20. Macrobienthos note diversity and acundance)	0		1	2	3	
21. Aquatic Moilusks	a		1	2	3	
	A		0.5	1	1.5	

0

30

0.5

0.5

05

) 5

15

15

15

0

FACW = 675 OBL = 15 Other = 0

26 Wetland plants in streambed in operating streams may also be dentified using other methods. See p. 35 of manual representations.

Niotes:

22. Fish

23 Orayfish

25 Algae

24 Amonibians

Skatch.

Date: 10/6/6	Project Site976	39-2-1-73	73 Latituda		
Evaluator: DM	County:		Longitude		
Total Points: Stream is at least interminent 9.75 if a 19 or perennal if a 30"	Stream Determi Ephameral Inte	e) Other ial e.g. Quas Name			
2 1		147 -1	No de serv		
A. Geomorphology (Subtotal = 7.7)	Absent	Weak	Moderate	Strong	
1* Continuity of shannel bed and pank	9	0	2	3	
2. Sinuosity of charnel along thaliweg	9		2	3	
3 In-grannel structure ex rif e-pool step-pool	a l	1	(2)	3	
npole-pool sequence 4. Particle size of stream substrate	. 0	(1)	2	3	
5 Active religt floodolan	(D)	1	2	3	
6. Depositional bars or benones	3	1	2	3	
7. Recent alluvial deposits	(a)	1	2	3	
8. Headouts	18	1	2	3	
9 Grade control		03	1	15	
1) Natural valley	0	0.5	1	1.5	
11. Second or greater order channel	115=7		Y'as	= 3	
*artificial ditches are not rated, see discussions in manual B. Hydrology (Subtotal =				y some	
12. Presence of Basero M		1	2	3	
13 Iron o cidizing bactana	0	1	2	3	
14, Leaf litter	1.5	1	(5)	1 0	
15 Sediment on plants or depris	(0)	0.5	1	1.5	
18 Organic depris lines profes	Ó I	0.5	0	1.5	
17. Soil-based evidence of high water (aple?	813	£3/	res	= 3	
C. Biology (Subtotal = 2 + 15)	7	· .			
13. Fibrous roots in streamped	3	(3)	1	9	
19. Rooted upland plants in streamped	(3)	2	1	2	
20. Macrobenthos (note diversity and adundance)	(0)	1	2	3	
21. Aquatic Mollusks		1	2	3	
22. Flan	0	0.5	1	1 5	
23 Grayfish	(3)	0.5	1	1.5	
24. Amphibians	ă l	0.5	1	1 5	
25 Algae	Ŏ	3.5	III.	1.5	
26 Watland plants in streambed	-	FACV = 073	OBL = 15 Other =	0	
fperennial streams may also be identified using other met	nods Seep 35 bilmanua	į.			

Skatch.

Date: 10/6/16	Project Site9789-24 - 7373		Latitude	Latituda			
Evaluator: DM	County:		Longitude				
Total Points: Stream is at least intermine it 16 15	Stream Determination (circle one) Ephemeral Intermittent Perannial		Other (e.g. Guas Name:				
A. Geomorphology (Subtotal = 8	Absent	Weak	Moderate	Strong			
13 Continuity of channel bed and bank	0	1	(2)	3			
2 Sinupsity of grannel along that wag	0	1	(2)	3			
3 In-grannel structure ex nife-book step-book	a l	,	(3)	3			
ropie-pool sequence	3		0				
4. Particle size of stream substrate	A :	1	2	3			
5 Active relict floodolain	0	0	2	3			
6. Depositional bars or benones	(3)	1	-	3			
7 Recent alluvia deposits	(9)		2	3			
8. Headouts	(3)	1	2	3			
9 Grade control	0	0.5	2	1.5			
10 Natural valley	(A)	0.5	1	1.5			
11. Second or greater order channel	i No	= 0	Yes = 3				
*artificial ditories are not rated, see discussions in manual B. Hydrology (Subtotal = 4,7)			2				
12. Presence of Base ⁿ ow	(9)	1 1	2	3			
13 Iron oxidizing baotena	75	1	2	3			
14. Leaf litter		1 0	(03)	0			
15 Sediment on plants or depris	0	0.5	1	1.5			
16. Organic deons lines or piles	0 1	0.5		1.5			
17. Soil-based airdence of high water table?	No.	= 3	f as	707			
C. Biology (Subtotal = 3.25)							
13 Fibrous roots in streamced	3	(2)	11	0			
19. Rooted upland plants in streambed	(3,)	2	1	9			
20. Magrobenthos inote diversity and abundance)	3	1 1	2	3			
21. Aquatic Modusks		1	2	3			
22. Flan	a	0.5	1	1.5			
23 Crayfish	0	2.5	1	1.5			
24. Amphioians	0	0.5	1	1.5			
25 Algae	0	03	1	1.5			
26 Westland plants in streambed	3	FACN = 073) 08	BL = 15 Owner =	0			
foerennial streams may also be dentified using other metro	oda Šeelo Šālof manuai						
Nintes							

Date: 10/6/16	Project Sitegre	Latituda			
Evaluator: DM	County:	Longitude			
Total Points: Stream is at least interminent 13.25 if a 19 or perennia if a 30"	Stream Determination (zirole one) Ephemeral Intermittent Perennial		Other e.g. Suad Name:		
A. Geomorphology (Suptotal = 5,5)	Absent	Weak	Moderate	Strong	
A. Geomorphology (Suptotal =//			2	3.10113	
1 Country of elementations and a series	0	9	2	3	
2. Sinuosity of channel along thalweg	0	0			
 In-channel structure ex riffe-cool step-cool rippie-pool sequence 	0	0	2	3	
4. Particle size of stream substrate	0	(n) -	2	3	
5 Active/religt floodgrain	(D)	1	2	3	
6 Depositional bars or benones	(6)	1	2	3	
7. Recent alluvial deposits	(0)	1	2	. 3	
3. Headouts	1 0	0	2	3	
9 Grade control	0	C 3> 1		1.5	
10. Natural valley	0	0.5	1	1.5	
11. Second or greater order channel	1 117=3)		Yas = 3		
antificial ditches are not rated, see 3.82488 one in manual					
D. (1) 4: - (-g)		1 1	2	3	
12. Presence of Baseflow	0		2	3	
13 Iron oxidizing bacteria	1.5	1	(05)	i c	
14 Leaf litter		0.5	1	1.5	
15 Sediment on plants or decris	\$	0.5	1	1.5	
16 Organic depris lines provies		=) :	Yas ₹ D		
17. Soil-based evidence of nigh water table?	- 10		-	0	
C. Biology (Subtotal = 3.15)	3	2	CD	1 0	
13 Fibrous roots in streamped	: 3	(2)	1	0	
19 Repoted upland plants in streambed	0	4	2	3	
20. Macrobenthos inote diversity and abundance)	3	1	2	3	
21. Aquatic Mollusks	- 3	0.5		1 5	
22, Fish 23, Crayfish	@	3.5	1	1.5	
	07	0.5	1	15	
24 Amphibians	8	3.5	1	15	
25 Algae		FAC'N = 0'75) OS	N = 15 Crase=		
26 Wetland clants in streambed	na Sasa 15 alman a				
foerennial streams may also be dentified using other method	73 344 0 12 31 Mg./79				

Date: 10)	16/16	Project Site 9789-24-7373	Latitude
Evaluator:	DM	County	Longitude
Total Points: Stream is at least if if a 19 or perennia.		Stream Determination (sircle one) Ephemeral Intermittent Perennial	

A. Geomorphology (Subtotal =	-	Absent		Weak		Moderate	Strong
11 Continuity of channel bed and pank		0		1		(2)	3
2. Sinupsity of channel along thalivery		0		(1)		2	3
3 In-channel structure ex riffe-pool step-pool noble-pool sequence		ð	1	1		(2)	3
4 Particle size of stream sucstrate		0		(1)	- 1	2	3
5 Activa relict floodolain		(0)		1	11.200	2	3
6. Depositional bars or benones		0		1		2	: 3
7 Recent alluvia decosits	- Y	(2)	i	1		2	3
8. Headouts	1	(0)		1		2	3
9 Grade control		0	4	6.5	1	0	1.5
10 Natural valley		(3)		0.5		1	1.5
11. Second or greater order channel	1		No €	3)		1 35	= 3
artificiai ditones are not rated, see discussions in manual							
B. Hydrology (Subiotal = 7.5)							
12. Presence of Basero N		0		0		2	3
13 Iron okidizing bastana		0		(1)		2	3
14. Leaf litter	1-	1.5	4	0		0.5	1 0
15 Sediment on plants or depris	5	0	1	(0.5)		1	1.5
13 Organic depris lines provies		0	1	0.5		0	1 15
17. Soil-based evidence of high water table?			110 =	3	- 1	Yes	(3)
C. Biology (Subtotal = 8,75)				K 1			
13 Fibrous roots in streamged	d.	(3)		2	4	1	1 0
19 Rooted upland plants in streamced	1	(3)		2		1	2
20. Macrobientines (note diversity and adundance)	120	0		(1)		2	3
21. Aquatic Mollusks		(3)		1		2	3
22. Fish	- 1	(B)		0.5		1	1.5
23 Crayfish		0		3.5	/4		1.5
24. Amphibians	1	C	11	0.5	1	1	1.5
25 Algae	1	0) 5	(6)	0	1.5
28 Wetland plants in streambed	1			FACK = Ø	5/ OBL	.=15 Other=	<u>G</u>
foerannial streams may also be dentified using other metr	ods See	o 35 si ma	rual				

Date: 10/6/16	Project Site 9 789-24-7373	Latitude
Evaluator: DM	County:	Longitude:
Total Points: Stream is at least intermittent if ≥ 19 or perennia, if ≥ 30° 18,25	Stream Determination (sircle one) Ephemeral Intermittent Perennial	Other e.g. Quad Name:

A. Geomorphology (Subtotal = 8 + 5)	Absent	Weak	Moderate	Strong
1° Continuity of channel bed and bank	0	1	(27)	3
2. Sinuosity of channel along thalwag	0	1	(2)	3
3 In-channel structure ex riffle-pool step-pool rippie-pool sequence	0	(2)	2	3
Particle size of stream substrate	0		2	3
5 Active/reliet floodolain	(3)	1	2	3
6. Depositional bars or benches		1	2	3
7. Recent alluvial deposits	0	(D)	2	3
8. Headcuts	0	(T)	2	3
9 Grade control	0	(0.5)	1	1.5
10. Natural valley	(0)	0.5	1	1.5
11. Second or greater order channel	cif	(=0)	Yes	= 3
artificial ditches are not rated, see discussions in manual B. Hydrology (Subtotal =H)				
12. Presence of Baseflow	(0)	1	2	3
13. Iron oxidizing bacteria	(0)	1	2	3
14. Leaf litter	1.5	(1)	0.5	0
15 Sediment on plants or depris	02 1	0.5	1	1.5
16. Organic debris lines or piles	0 1	(0.5)	1	1.5
17. Soil-based evidence of high water table?	No	= 0	Yes	=3)
C. Biology (Subtotal = 5 , 75)		* *		No.
13 Fibrous roots in streamped	3	2	(1)	1 0
19. Rooted upland plants in streambed	1 3	2	1	0
20. Macrobenthos (note diversity and abundance)	l &	1	2	3
21. Aquatic Mollusks	0	. 1	2	3
22. Fish	0	0.5	1	1.5
23. Crayfish	8	0.5	1	1.5
24. Amphibians	0	0.5	1 1	1.5
25 Alcae	0	0.5	0	1.5
26 Wetland plants in streambed	1	FACW = 073	OBL = 15 Other =	0
perannial streams may also be dentified using other metho	ss Seep 35 of manua	i.		
Notes:				

Date	10/6/16	Project S (4) 789-24-7373	Latitude
Evaluator:	DM	County	Longitude
Total Point Stream is at le	ast wite mutter 1 / o/	Stream Determination (circle one) Ephemeral Intermittent Perennial	Other e.g. Suad Name:

A. Geomorphology (Subtotal = 8)	Absent		Weak	Moderate	Strong
1. Continuity of channel bed and pank	9		(1)	2	3
2 Sinusity of channel along that weg	Ð	7	1	(2)	3
3 in-channel structure ex riffe-cool step-bool rippie-book seguence	i 0		0	2	3
Particle size of sizeam substrate	. 0		(1)	2	3
5 Activa raligt floodgrain	0	1	1	2	3
6. Depositional bars or benones	g		cb	2	3
7. Recent alfuvia: deposits	9	1	0	2	3
3. Headouts	0		1	2	3
9 Grade control	0		C 5	0	1.5
13. Natural valley	. 0	7	0.5	1	1.5
11. Second or greater order shannel	i	No E	2)	/ as	5 = 3
fantificial dispressions not rated, see dispussions in manual B. Hydrology (Subtotal =					
12. Presence of Base ^P O N	0		1	2	3
13 Iron okidizing basteria	(3)			2	3
14. Leaf litter	1.5	,	1	(0,5)	0
13 Sediment on plants or depris	(3)		0.5	1	1.5
18 Organic depris thes or piles	0	1	0.5	0	1,5
17. Soil-based evidence of night water table?		N12 =	2	ra	3 = 3
C. Biology (Subtotal = 6.75)	The state of the s		H G		
13 Fibrous roots in streamped	3		(2)	1	1 0
19 Rooted upland plants in streamped	3		2	11	0
20. Magrobienthos inote diversity and adundance)	(9)		1	2	3
21. Aquatic Mollusks	(6)		1	2	3
22 Fish	0		0.5	1 1	1.5
23 Crayfian	0	1	0.5	. 1	1.5
21 Amphibians	0		0.5	11	1.5
25 Algae	0		0.5	· 0	1.5
28 Watland crants in streamped			FACY =075	OBL = 1.5 Other	= 10
foerennia: streams may also be identified using other metho	cas See o 35 of ma	nua:			
Notes:					

2016/006/220

RIS

NC DWQ Stream Identification Form Version 4.11

Date: 10	16/2016	Project Sta 9789-24-7373	Latituda
Evaluator:	DM	County	Longitude
Total Points: Stream is at least inte if 2-19 or serennia if		Stream Determination (sircle one) Ephemeral Intermittent Perennial	Other e.g. 2.sad Namel

A. Geomorphology Subtotal = 7, 5	Absent		Weak	1	Moderate	Strong
13 Continuity of channel bed and park	ō		(1)		2	3
2 Sinuosity of channel along the weg	0	E	1		(3)	3
3 In-orianne structure ex nif e-cool step-pool rippie-pool sequence	0		1		3	3
1 Particle's ze of sysam sucsuate	. 0	1	0	Jal	2	3
5 Activalization floodolarn	0		1		2	3
6. Depositional bars or benones	(0)		1	3	2	3
7 Recent alluvia deposits	0		(1)		2	3
8. Headouts	(%)		1	t .	2	3
9 Grade sontrol	0		(0.2)	1	1	1.5
1) Natural valley	. (2)		0.5		1	1.5
11. Second or greater order shannel	ì	MXE	0)	8	1 95	= 3
artificial ditanes are not rated, see a sousaions in manual				X 4		
B. Hydrology (Subtotal = 4.5)						
12. Presence of Base [#] ow	0		1		2	3
13 Iran okidizing bastena	(2)		1		2	3
14. Leaf litter	1.5	- 6	1	16	(93)	1 0
15 Sediment on plants or depris	0		0.5		1	1.5
16 Organic debris lines or piles	٥	1	0.5		(D)	1.5
17. Soil-based evidence of high water table?)10 =	3	3/	198	= 3)
C. Biology (Subtotal = 2,25)						
13 Florous roots in streamped	3		2		(1)	9
19. Rooted upland plants in streamped	3		2		0	0
20. Magropienthos india diversity and adundance)	0		1		2	3
21. Aquatic Mollusks	1 3		1		2	3
22. Fian	0		0.5		1	15
23 Crayfish	19	- 1	0.5		1	1.5
21 Amphibians	<u>ල</u>	1	0.5		1	15
25 Alcae	0		0.5.)	- 1	1	1.5
28 Wetland prants in streamped	4.		FACW = 07	3) OSL	= 1.5 Other=	0

2016/006/226

RII

Date: 10/6/16	Project/Site:	Project/Site: 739-24-7373 County:			
Evaluator: DM	County:			Longitude:	
Total Points: Stream is at least intermittent f ≥ 19 or perennial if ≥ 30*		nation (circle one) rmittent Perennial	Other e.g. Quad Name:		
A. Geomorphology (Subtotal = 4.5)	Absent	Weak	Moderate	Strong	
^a Continuity of channel bed and bank	0	0	2	3	
2. Sinuosity of channel along thalweg	(0)	1	2	3	
In-channel structure: ex. riffle-pool, step-pool, ripple-pool sequence	0	1	2	3	
Particle size of stream substrate	0	1	2	3	
5. Active/relict floodplain	(0)	1	2	3	
6. Depositional bars or benches	(0)	1	2	3	
7. Recent alluvial deposits	0	(D)	2	3	
3. Headcuts	0	1	2	3	
Grade control	0	0.5	1	1.5	
Natural valley	0	0.5	1	1.5	
Second or greater order channel	No	= 0	Yes :	= 3	
artificial ditches are not rated; see discussions in manual 3. Hvdrology (Subtotal = 2)					
	(2)	4	2	2	
2. Presence of Baseflow	0	1	2	3	
Iron oxidizing bacteria	0	1	2	3	
4. Leaf litter	1.5		0.5	0	
5. Sediment on plants or debris	(0)	0.5	1	1.5	
6. Organic debris lines or piles	0	0.5	(1)	1.5	
17. Soil-based evidence of high water table?	No	=(0)	Yes	- 3	
C. Biology (Subtotal = 6,25)					
8. Fibrous roots in streambed	3	2	1	0	
9. Rooted upland plants in streambed	(3)	2	1	0	
20. Macrobenthos (note diversity and abundance)	(0)	1	2	3	
21. Aquatic Mollusks	0	1	2	3	
22. Fish	(0)	0.5	1	1.5	
23. Crayfish	0	0.5	1	1.5	
24. Amphibians	(0)	0.5	1	1.5	
25. Algae	0	0.5)	1	1.5	
26. Wetland plants in streambed		FACW = 0(75;) OBI	L=1.5 Other = 0	V	
*perennial streams may also be identified using other meth-	ods. See p. 35 of manua	l.			
Notes:					
Sketch:					

RIZ

Date: 10/6//6		9-24-1373	Latitude:		
Evaluator: DM	County:		Longitude:		
Total Points: Stream is at least intermittent if ≥ 19 or perennial if ≥ 30*		nation (circle one) mittent Perennial			
A. Geomorphology (Subtotal = 4.5)	Absent	Weak	Moderate	Strong	
1ª. Continuity of channel bed and bank	0	(1)	2	3	
Sinuosity of channel along thalweg	0	1	2	3	
In-channel structure: ex. riffle-pool, step-pool, ripple-pool sequence	0	0	2	3	
4. Particle size of stream substrate	0	1	2	3	
5. Active/relict floodplain	0	1	2	3	
6. Depositional bars or benches	0	1	2	3	
7. Recent alluvial deposits	Ö	(1)	2	3	
8. Headcuts	0	(1)	2	3	
9. Grade control	0	(0.5)	1	1.5	
10. Natural valley	(0)	0.5	1	1.5	
11. Second or greater order channel	No € 0 Yes = 3				
a artificial ditches are not rated; see discussions in manual B. Hydrology (Subtotal =)					
12. Presence of Baseflow	(0)	1	2	3	
13. Iron oxidizing bacteria	0	1	2	3	
14. Leaf litter	1.5	1	0.5	0	
15. Sediment on plants or debris	0,	0.5	1	1.5	
16. Organic debris lines or piles	0	0.5	1	1.5	
17. Soil-based evidence of high water table?	No	= 0	Yes	3	
C. Biology (Subtotal =)					
18. Fibrous roots in streambed	(3)	2	1	0	
19. Rooted upland plants in streambed	(3)	2	1	0	
20. Macrobenthos (note diversity and abundance)	0	1	2	3	
21. Aquatic Mollusks	(0)	1	2	3	
22. Fish	(0)	0.5	1	1.5	
23. Crayfish	0)	0.5	1	1.5	
24. Amphibians	0)	0.5	11	1.5	
25. Algae	(0)	0.5	1	1.5	
26. Wetland plants in streambed	0	FACW = 0.75; OB	L = 1.5 Other = 0	0	
*perennial streams may also be identified using other methods	s. See p. 35 of manual				
Notes:					
Sketch:					

12,1

etermination (circle al Intermittent Pere nt	Moderate 2 2 2 2 2 2 2 1 1 Yes 2 2 2 2 2 2 2 2 2	e: Strong 3 3 3 3 3 3 3 1.5 1.5 1.5 s = 3 3 3 0 1.5
nt Weak 1 1 1 1 1 0.5 No ∈ 0 1 1 0.5 0.5 0.5	Moderate 2 2 2 2 2 2 2 1 1 Yes 2 2 2 2 2 2 2 2 2	Strong 3 3 3 3 3 3 3 3 1.5 1.5 1.5 5 = 3
1 1 1 1 1 1 1 1 0.5 0.5 0.5 No \(\infty\)	2 2 2 2 2 2 1 1 1 Yes 2 2 2 2 2 2 2 2 2 2 2 1 1 1 1 Yes 2 2 2 2 2 1 1 1 1 Yes 2 2 2 1 1 1 1 Yes 2 2 2 1 1 1 1 Yes 2 2 1 1 1 1 Yes 2 2 1 1 1 1 Yes 2 1 1 Yes 2 1 1 Yes 2 1 1 Yes 2	3 3 3 3 3 3 3 3 1.5 1.5 1.5 3 0
1 1 1 1 1 1 1 1 0.5 0.5 0.5 No \(\infty\)	2 2 2 2 2 2 1 1 1 Yes 2 2 2 2 2 2 2 2 2 2 2 1 1 1 1 Yes 2 2 2 2 2 1 1 1 1 Yes 2 2 2 1 1 1 1 Yes 2 2 2 1 1 1 1 Yes 2 2 1 1 1 1 Yes 2 2 1 1 1 1 Yes 2 1 1 Yes 2 1 1 Yes 2 1 1 Yes 2	3 3 3 3 3 3 3 3 1.5 1.5 1.5 3 0
1 1 1 1 1 1 1 1 1 1 0.5 (0.5)	2 2 2 2 1 1 1 Yes 2 2 0.5 1	3 3 3 3 3 1.5 1.5 5 = 3
1 1 1 1 1 1 1 1 1 1 0.5 (0.5)	2 2 2 2 1 1 1 Yes 2 2 2 0.5 1	3 3 3 3 3 1.5 1.5 1.5 8 = 3
1 1 0.5 0.5 0.5 No ∈ 0	2 2 2 1 1 1 Yes 2 2 (0.5) 1	3 3 3 1.5 1.5 1.5 s = 3
1 1 0.5 0.5 0.5 No ∈ 0	2 2 2 1 1 1 Yes 2 2 2 (0.5) 1	3 3 1.5 1.5 1.5 s = 3
1 0.5 0.5 0.5 No © 0	2 2 1 1 1 Yes 2 2 (0.5) 1	3 3 1.5 1.5 1.5 s = 3
1 (0.5 0.5 No ∈ 0)	2 1 1 Yes 2 2 0.5	3 1.5 1.5 s = 3
0.5 0.5 No ∈ 0 1 1 1 0.5 (0.5)	2 2 2 0.5	1.5 1.5 s = 3
0.5 No ∈ 0 1 1 1 0.5 (0.5)	2 2 2 0.5	1.5 s = 3
No ∈ 0) 1 1 1 0.5 (0.5)	2 2 0.5	3 3 0
1 1 1 0.5 (0.5)	2 2 (0.5) 1	3 3 0
1 1 0.5 (0.5)	2 (0.5) 1	3 0
1 1 0.5 (0.5)	2 (0.5) 1	3 0
1 1 0.5 (0.5)	2 (0.5) 1	3 0
1 0.5 (0,5)	0.5	0
0.5	1	-
(0,5)		1.5
7866-5	831	1.0
No = 0	1	1.5
(1980) [] [] []	Yes	s ₹3
2	0	0
(2)	1	0
1	2	3
1	2	3
0.5	1	1.5
0.5	1	1.5
	1	1.5
(0.5)	1	1.5
	75; OBL = 1.5 Other =	= 0
manual.		
f	1 0.5 0.5 0.5 0.5 (0.5)	1 2 1 2 0.5 1 0.5 1 0.5 1 0.5 1 0.5 1 FACW = 0.75; OBL = 1.5 Other =

Date: 10/6/16	Project/Site:	Project/Site: County:		
Evaluator: DM	County:			Longitude:
Total Points: Stream is at least intermittent	Stream-Determination (circle one) Ephemeral Intermittent Perennial		Other e.g. Quad Name:	
A. Geomorphology (Subtotal = 2)	Absent	Weak	Moderate	Strong
1 ^a Continuity of channel bed and bank	0	1	(2)	3
2. Sinuosity of channel along thalweg	0.7	1	2	3
In-channel structure: ex. riffle-pool, step-pool, ripple-pool sequence	0	1	2	3
Particle size of stream substrate	(0)	1	2	3
5. Active/relict floodplain	0	1	2	3
6. Depositional bars or benches	0	1	2	3
7. Recent alluvial deposits	0	1	2	3
8. Headcuts	0	1	2	3
9. Grade control	(0)	0.5	1	1.5
10. Natural valley	0	0.5	1	1.5
11. Second or greater order channel	No	=(0)	Yes :	= 3
artificial ditches are not rated; see discussions in manual				
B. Hydrology (Subtotal = <u>5.9</u>)				
12. Presence of Baseflow	0	0	2	3
13. Iron oxidizing bacteria	(0)	1	2	3
14. Leaf litter	1.5	12	0.5	0
15. Sediment on plants or debris	0	0.5	1	1.5
16. Organic debris lines or piles	0	0.5	1	1.5
17. Soil-based evidence of high water table?	No	= 0	Yes :	= 3)
C. Biology (Subtotal =)				
18. Fibrous roots in streambed	3	(2)	1	0
19. Rooted upland plants in streambed	(3)	2	1	0
20. Macrobenthos (note diversity and abundance)	(0)	1	2	3
21. Aquatic Mollusks	(0)	1	2	3
22. Fish	Ø	0.5	1	1.5
23. Crayfish	0	0.5	1	1.5
24. Amphibians	0	0.5	1	1.5
25. Algae	0	0.5	1	1.5
26. Wetland plants in streambed		FACW = 0.75; OBL	_ = 1.5 Other = 0	i e
*perennial streams may also be identified using other meth	ods. See p. 35 of manual	+		
Notes:				
Sketch:				



PUBLIC WORKS DEPARTMENT STORMWATER MANAGEMENT DIVISION

405 Martin Luther King, Jr. Blvd. Chapel Hill, NC 27514-5705 Telephone (919) 969-7246 Fax (919) 969-7276 www.townofchapelhill.org

REQUEST FOR STREAM DETERMINATION

Stream determinations are used to determine whether the Resource Conservation District or the Jordan Stream Buffer will apply to a property, and the areas protected if that is the case. There is no fee for stream determinations. By default, we will search records and notify you if a site visit is not needed for a property.

Check here if you want Town staff to conduct a stream determination. A new site visit may not be needed if a determination has been done in the last five years. Turnaround time is within two weeks for single-family lots depending on weather conditions, staff availability, and size of the lots. Requests may be emailed (DMilkereit@townofchapelhill.org), faxed, dropped off at Town Hall or the Stormwater Office, or mailed to the above address care of "Stormwater Specialist". Requestor's Name: Dewberry Mailing Address: 2610 Wycliff Road, Suite 410 City, State, ZIP: Raleigh, NC 27607 Phone / FAX / Email: 919-424-3767/choffman@dewberrv.com Check method(s) for US Mail report to be sent: ☑ Email □ FAX Call for pickup Signature of property owner or designated legal agent granting permission to Town Staff to enter the property(jes) indicated below for purposes of a Stream Determination: (Signature) (Date) Owner Name(s):

Parcel ID Number (PIN)	Address / Location Description
Western Portion 9789247373	101 Airport Road/Estes Drive Extension
Warner	

Where the total area of the property(ies) to visit is over 3 acres please attach an as-built drawing or a topographic map with current landmarks.

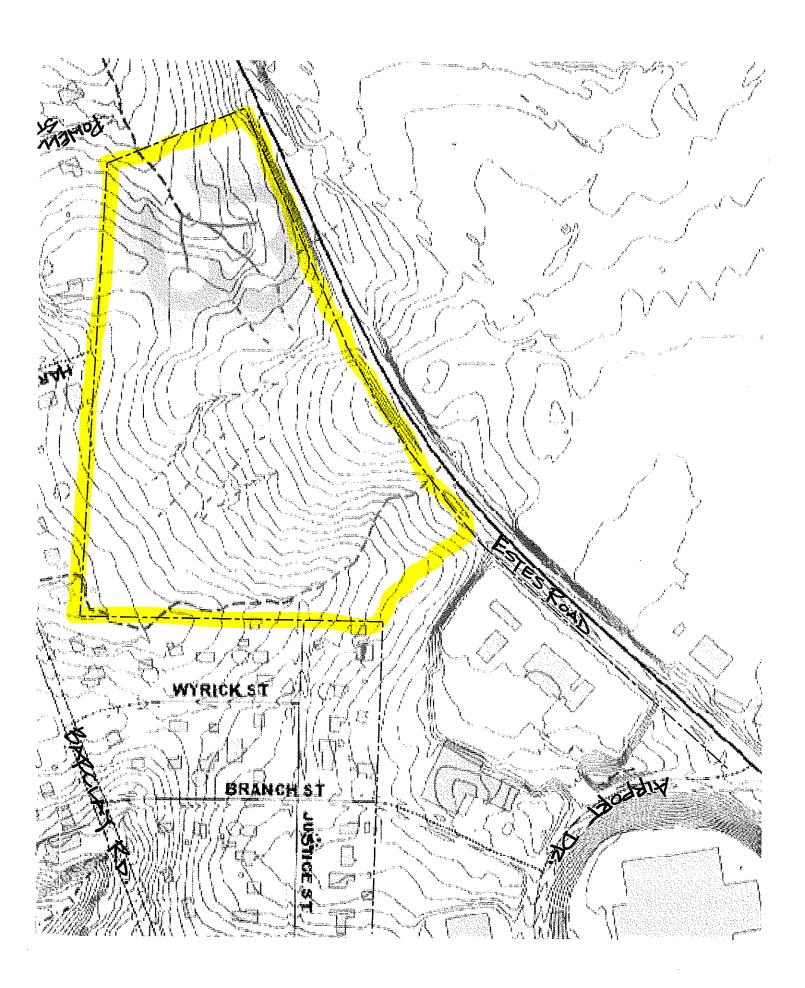


Exhibit K - Impact Analysis on the Value of Contiguous Properties of a Proposed Municipal Services Center

IMPACT ANALYSIS

ON THE VALUE OF CONTIGUOUS PROPERTIES OF A PROPOSED MUNICIPAL SERVICES CENTER

LOCATED ON

ESTES DRIVE CHAPEL HILL, NORTH CAROLINA

AS OF

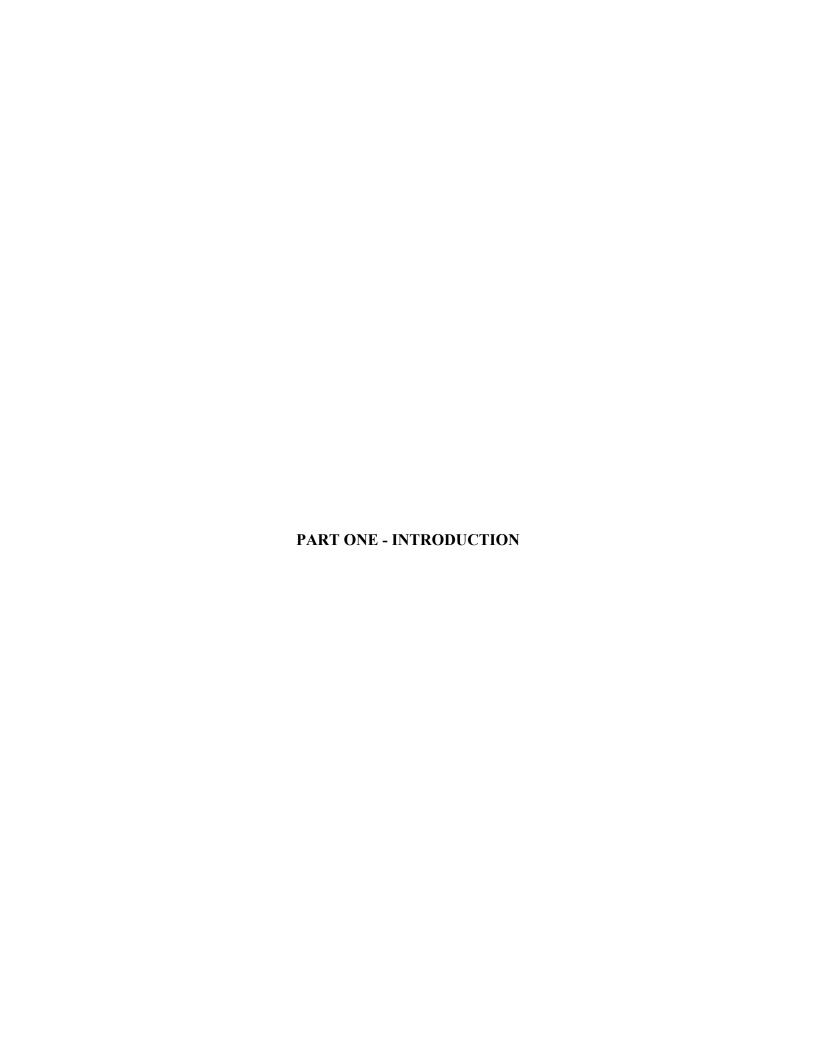
APRIL 2, 2018

FOR

TOWN OF CHAPEL HILL 405 MARTIN LUTHER KING, JR. BOULEVARD CHAPEL HILL, NC 27514

BY

DAVID A SMITH, MAI, SRA POST OFFICE BOX 51597 DURHAM, NORTH CAROLINA 27717-1597





DAVID A. SMITH, MAI, SRA

P.O. BOX 51597 DURHAM, NORTH CAROLINA 27717-1597 PHONE (919) 493-1534 smithappraiser@verizon.net



April 3, 2018

Town of Chapel Hill 405 Martin Luther King, Jr. Boulevard Chapel Hill, NC 27514

As requested, I have inspected the properties contiguous to a proposed municipal services center to be located on Estes Drive in Chapel Hill, North Carolina.

The purpose of this analysis is to determine if a proposed municipal services center is "located, designed, and proposed to be operated so as to maintain or enhance the value of contiguous property." The intended use of this report is to assist the approving body in determining the effect of the proposed municipal services center. The intended users of this report are officers and employees of the Town of Chapel Hill and anyone they designate.

As requested, a summary report has been prepared. This is not an appraisal, but a consulting assignment.

The property was last inspected on April 2, 2018 which is the effective date of this report and analysis. Based on an inspection of the property and the contiguous properties, an analysis of data gathered and facts and conclusions as contained in the following report of 17 pages, and subject to the assumptions and limiting conditions as stated, it is my opinion that the **proposed municipal services center will maintain or enhance the value of contiguous property.**

I certify that I have personally inspected the property and the contiguous properties. I further certify that I have no interest either present or contemplated in the properties and that neither the employment to make this analysis nor the compensation is contingent upon the result of the analysis.

Respectfully submitted,

David 9. Smith

David A. Smith, MAI, SRA NC State-Certified General Real Estate Appraiser #A281



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CERTIFICATION

I certify that, to the best of my knowledge and belief,...

The statements of fact contained in this report are true and correct.

The reported analyses, opinions, and conclusions are limited only by the reported assumptions and limiting conditions, and are my personal, impartial, and unbiased professional analyses, opinions, and conclusions.

I have no present or prospective interest in the property that is the subject of this report, and no personal interest with respect to the parties involved.

I have no bias with respect to the property that is the subject of this report or to the parties involved with this assignment.

My engagement in this assignment was not contingent upon developing or reporting predetermined results.

My compensation for completing this assignment is not contingent upon the development or reporting of a predetermined value or direction in value that favors the cause of the client, the amount of the value opinion, the attainment of a stipulated result, or the occurrence of a subsequent event directly related to the intended use of this appraisal.

My analyses, opinions, and conclusions were developed, and this report has been prepared, in conformity with the *Uniform Standards of Professional Appraisal Practice*.

I have made a personal inspection of the property that is the subject of this report.

No one provided significant real property appraisal assistance to the person signing this certification.

The reported analysis, opinions, and conclusions were developed, and this report has been prepared, in conformity with the requirements of the Code of Professional Ethics and the Standards of Professional Appraisal Practice of the Appraisal Institute.

The use of this report is subject to the requirements of the Appraisal Institute relating to review by its duly authorized representatives.

As of the date of the report, I have completed the requirements of the continuing education program of the Appraisal Institute.

This assignment was not made, nor was the appraisal rendered on the basis of a requested minimum valuation, specific valuation, or an amount, which would result in approval of a credit transaction.

David A. Smith, MAI, SRA

David 9. Smith



STATEMENT OF COMPETENCE

I have completed all of the requirements to become a state certified-general appraiser for the State of North Carolina and all of the requirements for the MAI designation. In addition I have successfully completed USPAP courses and continuing education seminars for over thirty years. More detailed information about these courses and seminars are in the qualifications section of this report. I have prepared similar analyses and feel competent to perform this analysis.

EXTRAORDINARY ASSUMPTIONS AND HYPOTHETICAL CONDITIONS

An extraordinary assumption is an assumption, directly related to a specific assignment, which if found to be false, could alter the appraiser's opinions or conclusions. A hypothetical condition is something that is contrary to what exists but is supposed for the purpose of the analysis. This analysis assumes that the property will be improved with the municipal services center as planned. No other extraordinary assumptions or hypothetical conditions are made.

GENERAL ASSUMPTIONS AND LIMITING CONDITIONS

The report has been made with the following general assumptions:

- 1. Possession of this report, or a copy thereof, does not carry with it the right of publication.
- 2. The appraiser by reason of this report is not required to give further consultation or testimony or to be in attendance in court with reference to the property in question unless arrangements have been previously made.
- 3. Neither all nor any part of the contents of this report (especially any conclusions, the identity of the appraiser, or the firm with which the appraiser is connected) shall be disseminated to the

public through advertising, public relations, news, sales or other media without the prior written consent and approval of the appraiser.

- 4. Definitions used in this report have been taken from *The Dictionary of Real Estate Appraisal*, 5th ed., published by the Appraisal Institute, copyright 2010.
- 5. Descriptions of the site and proposed improvements is based on a personal inspection of the property, public records and information supplied by the Town of Chapel Hill. This includes a set of plans entitled "Town of Chapel Hill, Municipal Services Building, Site and Building Design" dated February 22, 2018. For purposes of this report this information is assumed to be correct. Copies of these plans are in the addenda.

PURPOSE, INTENDED USE AND USERS OF THE REPORT

The purpose of this analysis is to determine if a proposed municipal service center will maintain or enhance the value of contiguous properties. The intended use of the appraisal is to assist the approving body in determining the effect of the proposed center. The intended users of this report are offices and employees of the Town of Chapel Hill and anyone they designate.

DEFINITION OF VALUE

The opinion of value in this appraisal is the market value. The definition of market value is that used by federally regulated financial institutions

The most probable price which a property should bring in a competitive and open market under all conditions requisite to a fair sale, the buyer and seller each acting prudently and knowledgeably, and assuming the price is not affected by undue stimulus. Implicit in this definition is the consummation of a sale as of a specified date and the passing of title from seller to buyer under conditions whereby:

- 1. buyer and seller are typically motivated;
- 2. both parties are well informed or well advised, and acting in what they consider their own best interests;
- 3. a reasonable time is allowed for exposure in the open market;
- 4. payment is made in terms of cash in United States dollars or in terms of financial arrangements comparable thereto; and
- 5. the price represents the normal consideration for the property sold unaffected by special or creative financing or sales concessions granted by anyone associated with the sale.

DATE OF ANALYSIS AND DATE OF REPORT

The effective date of the analysis is April 2, 2018. The date of the report is April 3, 2018.

PROPERTY RIGHTS

The ownership interest considered in this report is the fee simple interest. The contiguous properties may be leased or have other property rights transferred, but the effect is for the fee simple value of the properties. The definition of fee simple as used in this report is:

Absolute ownership unencumbered by any other interest or estate, subject only to the limitations imposed by the governmental powers of taxation, eminent domain, police power, and escheat.

SCOPE OF WORK

The scope of the report involves collection and confirmation of data relative to the municipal services center property and the contiguous properties. I inspected the property and the plans provided and observed the contiguous properties from the street right-of-ways. I performed an analysis based on properties that are contiguous to an existing municipal facility to determine the effect on other properties. From this information, I determined that the proposed use will maintain or enhance the value of contiguous properties.



DESCRIPTION OF PROPERTY

Since the purpose of this report is to determine the effect of the proposed municipal services center on contiguous properties and not the property where the center will be located, only a brief description of the municipal services center property is given. It consists of two parcels with tax IDs of 9789-03-3163 and 9789-24-7373. The first parcel is owned by the University of North Carolina Endowment Fund Trustees, contains 2.5 acres and is heavily wooded. The second is owned by the University of North Carolina and contains 59.33 acres. The eastern section of this parcel is improved with facilities for the University of North Carolina but the western area where the proposed municipal services center will be located is unimproved and heavily wooded.

The property is located on the south side of Estes Drive a short distance north of the Estes /Seawell School Road intersection. Four roads dead end into the property, Justice Street, Hartig Street, Powell Street and Ward Street. Traffic would enter and leave the site at two points on Estes Drive roughly in the middle of the site.

The building will be three and a half stories in height and contain about 72,000 square feet of enclosed area. The exterior walls will be mostly glass. Three will be several paved surface parking lots with a total of 285 to 330 spaces.

The land will be leased to the Town by the University. Uses on the property include: Police Headquarters, Fire Administration, Parks and Recreation Administration and other small town functions (wellness clinic, omsbud, etc). The police will leave the property at 6 pm and the patrol is out in the community until 6 am. The building is not occupied 24/7 although there may occasionally be an individual officer or cleaning service on site.

The Town plans to pursue LEED designation for the Municipal Services Center and has committed to minimum light, sound, noise pollution through building design, technology, and the preservation of existing perimeter vegetation as well as additional landscaping around the disturbed area of the site.

The Town will also build a storm water treatment system. Currently, storm water is unmanaged and exits the property in the southeastern corner occasionally flooding the residential properties in that location. Post-development storm water runoff cannot exceed pre-development runoff and might actually be mitigated or improved.

Future facilities could include: A UNC building with similar office/administrative uses, a smaller town or university building with similar uses, or a fire station on the western end of the site with access to Estes Drive.

Copies of the site plan, tax cards, zoning map, aerial, floodplain map of the property from the GIS records are in the addenda. Photos of the property are also in the addenda.

CONTIGUOUS PROPERTIES

The proposed municipal services center is located in Orange County and in the city limits of Chapel Hill. The property is located on the south side of Estes Road across the street from the Chapel Hill Airport. Some of the site is part of a larger parcel and only those properties that adjoin that portion of the parcel where the center will be constructed are considered.

There are sixteen properties that will be contiguous to the proposed municipal services center. I did not enter any of the buildings but observed them all from the street. All are improved with single family dwellings. The following information is from tax records. One of the dwellings was built in 2002 but the rest were constructed between 1941 and 1962. One is 2,026 square feet in size but the rest range from 700 to 1,728 square feet in size. The average age is 67 years and the average size is 1,099 square feet. All of the properties appear to be in good condition and the neighborhood is a stable middle class one. The dwellings are occupied by a mixture of owners and tenants. A chart of the contiguous properties is on the following page. Photographs of some of the dwellings are in the addenda.

DAVID A. SMITH, MAI, SRA

Pin	Address	Size	Land	Building	Total	Year	Building
			Value	Value	Value	Built	Size
9789123934	804 Hartig St	0.28	\$100,000	\$172,000	\$272,000	1954	2,026
9789022908	832 Ward St	0.47	\$100,000	\$124,100	\$224,100	1956	1,333
9789027973	8 Powell St	0.72	\$100,000	\$112,100	\$212,100	1962	1,148
9789120930	805 Hartig St	0.81	\$100,000	\$154,300	\$254,300	1941	1,456
9789125836	224 Barclay Rd	0.37	\$100,000	\$88,400	\$188,400	1942	806
9789125899	222 Barclay Rd	0.31	\$100,000	\$102,200	\$202,200	1942	962
9789126980	220 Barclay Rd	0.31	\$100,000	\$125,800	\$225,800	1947	1,385
9789127981	218 Barclay Rd	0.21	\$100,000	\$126,800	\$226,800	1955	1,401
9789128969	216 Barclay Rd	0.46	\$100,000	\$92,200	\$192,200	1942	800
9789139112	1 Wyrick St	0.31	\$100,000	\$77,200	\$177,200	1947	768
9789139128	4 Wyrick St	0.3	\$100,000	\$47,000	\$147,000	1947	750
9789138295	807 Wyrick St	0.3	\$100,000	\$79,800	\$179,800	1947	775
9789138599	208 Justice St	0.5	\$100,000	\$163,300	\$263,300	2002	1,728
9789139599	206 Justice St	0.31	\$100,000	\$74,000	\$174,000	1942	700
9789230579	204 Justice St	0.33	\$100,000	\$100,900	\$200,900	1942	850
9789138357	211 Justice St	0.22	\$100,000	\$39,000	\$139,000	1952	700



EFFECT OF THE PROPOSED MUNICIPAL SERVICES CENTER

The potential adverse effects from any proposed use are environmental hazards, odor, noise, lighting, traffic and visual impact. Based on the information supplied, there will be no environmental hazards or increased odor associated with the proposed use. Lighting and visual impact will be minor and not more than a typical office building which is currently allowed without further approval. Traffic and noise should also not be higher than if the building was used as an office building. Police leave at 6 pm and do not return until 6am. There will be no direct access between the center and the adjacent neighborhood.

In order to estimate the effect of the proposed municipal services center on contiguous properties, I researched sales of dwellings in close proximity to existing municipal facilities. A short distance to the east of the proposed center are similar facilities for UNC. These facilities are contiguous to properties along the north side of Justice Street. I located three contiguous properties on Justice Street and compared them with four properties on the south side of Justice Street that are not contiguous to the facilities. The properties are similar in most respects. I only considered properties that sold since 2010 and were built between 1942 and 1955. I adjusted them for differences in market conditions (time) and divided the result by the square footage of the dwellings. A chart showing these properties is on the following page.

The three properties on the north side of Justice Street (contiguous to the municipal facilities) give an average adjusted selling price of \$191.44 per square foot. Those on the south side (not contiguous to the facilities) give and average adjusted selling price of \$187.07 per square foot or about 2.00% less. That is the properties that are contiguous to the municipal facilities actually sold for more. However, this is a very small difference and is within the margin of error.

Pin	Address	Land Size	Date Sold	Sa	les Price	Year Built	Build Sq Ft	Market	Adjusted
								Condition	
	CONTIGUOUS TO MUNICIPAL FACITLITES								
9789230579	204 Justice	0.33	6/13/2012	\$	175,000	1942	850	14.51%	\$235.76
9789234596	134 Justice	0.31	12/21/2015	\$	299,500	1948	1790	5.71%	\$176.86
9789239576	120 Justice	0.32	12/12/2010	\$	295,000	1955	2158	18.27%	\$161.68
								Average	\$191.44
		N	OT CONTIGUO	US TO	MUNICI	PAL FACILITIE	S		
9789234307	135 Justice	0.31	10/27/2014	\$	158,000	1942	1216	8.58%	\$141.09
9789235358	131 Justice	0.31	8/10/2017	\$	342,000	1942	2064	1.61%	\$168.36
9789236326	129 Justice	0.31	7/30/2012	\$	165,000	1942	1121	14.19%	\$168.08
9789239434	123 Justice	0.33	7/25/2013	\$	190,000	1942	784	11.73%	\$270.76
								Average	\$187.07

CONCLUSION AND SUMMARY

To determine if the proposed municipal services center will maintain or enhance the value of contiguous property, I considered properties near the proposed center that are currently contiguous to similar municipal facilities. I compared properties contiguous to these facilities with those that are not contiguous and found little difference in the selling prices. Based on this it is my opinion that the proposed municipal services center will not adversely affect the property values of those dwellings that will be contiguous to the center and the proposed use will maintain the value of the contiguous property.



DAVID A. SMITH, MAI, SRA

DAVID A SMITH & ASSOCIATES, INC. P.O. BOX 51597 DURHAM, NORTH CAROLINA 27717-1597 PHONE (919) 493-1534 smithappraiser@frontier.com



QUALIFICATIONS OF DAVID A. SMITH, MAI, SRA

The appraiser, David A. Smith, has been involved in the appraisal of real estate for over thirty years. He worked with his father, Charles W. Smith, from 1976 to 2003. After the retirement of Charles W. Smith in 2003 he formed Smith & Whitfield, Inc. and later David A. Smith & Associates. In 1988 he was awarded the RM designation. With the merger of the American Institute of Real Estate Appraisers and the Society of Real Estate Appraisers in January of 1991, the RM designation was changed to the SRA designation. In 1991 he was awarded the MAI designation of the Appraisal Institute. He became a state-certified real estate appraiser in 1991 the year the state first began licensing real estate appraisers and his certification number is A281.

He has also trained and supervised several appraisers and has prepared all types of appraisal reports. His primary focus is Durham County and the adjoining counties of Orange, Person, Granville and Chatham.

EDUCATION: Graduate Episcopal High School, Alexandria, VA, 1976 A.B., Duke University, Durham, NC, 1981

APPRAISAL INSTITUTE COURSES:

Real Estate Appraisal Principles (Exam 1A-1/8-1), University of North Carolina, 1981 Residential Valuation (Exam 8-2), University of North Carolina, 1981 Basic Valuation Procedures (Exam 1A-2), University of North Carolina, 1983 Standards of Professional Practice (Exam SPP), University of North Carolina, 1983 Capitalization Theory & Techniques, A (Exam 1B-A), University of Colorado, 1984 Capitalization Theory & Techniques, B (Exam 1B-B), University of Colorado, 1984 Valuation Analysis and Report Writing (Exam 2-2), University of North Carolina, 1987 Case Studies in Real Estate Valuation (Exam 2-1), University of North Carolina, 1987 Advanced Sales Comparison & Cost Approaches, Atlanta, Georgia, 2002 General Appraiser Market Analysis and Highest and Best Use, Atlanta, Georgia, 2007 Online Business Practices and Ethics, Chicago, Illinois, 2007 Appraisal Curriculum Overview, 2009 Condemnation Appraising: Principles & Applications, Greensboro, NC, 2011

APPRAISAL INSTITUTE SEMINARS:

Highest and Best Use, 1988

Industrial Valuation, 1988

Rates, Ratios and Reasonableness, 1988

Valuation of Leased Fee Interests, 1989

Current Problems in Industrial Valuation, 1989

Methods of Subdivision Analysis, 1989

Expert Witness in Litigation, 1989

Discounted Cash Flow, 1990

RTC Appraisal Standards, 1990

Preparation and Use of the UCIAR Form, 1990

Standards of Professional Practice Update, 1990

Commercial Construction Overview, 1991

Appraising Troubled Properties, 1991

Appraisal Regulations of the Federal Banking Agency, 1992

Real Estate Law for Appraisals, 1992

Appraising Apartments, 1993

Discounted Cash Flow Analysis, 1994

Appraiser's Legal Liabilities, 1994

Understanding Limited Appraisals, 1994

Analysis Operating Expenses, 1995

Future of Appraisals, 1996

Highest and Best Use Applications, 1996

Standards of Professional Practice, Parts A & B, 1997

Litigation Skills for the Appraiser, 1997

Eminent Domain & Condemnation Appraising, 1998

Matched Pairs/Highest & Best Use/Revisiting Report Options, 1998

Valuation of Detrimental Conditions, 1998

Appraisal of Nonconforming Uses, 2000

How GIS Can Help Appraisers Keep Pace with Changes in R E Industry, 2001

Feasibility Analysis, Market Value and Investment Timing, 2002

Analyzing Commercial Lease Clauses, 2002

Standards of Professional Appraisal Practice, 2002

Effective Appraisal Writing, 2003

Supporting Capitalization Rates, 2004

National USPAP Update, 2004

Rates and Ratios: Making Sense of GIMs, OARs, and DCFs, 2005

The Road Less Traveled: Special Purpose Properties, 2005

National USPAP Update, 2006

Appraisal Consulting: A Solutions Approach for Professionals, 2006

What Clients Would Like Their Appraisers to Know, 2007

Valuation of Detrimental Conditions, 2007

Business Practice and Ethics, 2007

Office Building Valuation: A Contemporary Perspective, 2008

Subdivision Valuation, 2008

National USPAP Update, 2009

Effective Appraisal Writing, 2009

Appraisal Curriculum Overview, 2009

Discounted Cash Flow Model: Concepts, Issues and Apps., 2010

National USPAP Update, 2010

Rates and Ratios: Making sense of GIMs, OARs and DCFs, 2011

National USPAP Update, 2012 Business Practices and Ethics, 2012

Marketability Studies: Advanced Considerations & Applications, 2013

Real Estate Valuation Conference, 2013

National USPAP Update, 2014

OTHER SEMINARS:

Commercial Segregated Cost Seminar, Marshall & Swift, 1988 Appraisal Guide and Legal Principles, Department of Transportation, 1993 The Grammar Game, Career Track, 1994

MEMBERSHIPS:

Appraisal Institute, MAI #09090 Appraisal Institute, SRA/RM #2248 Durham Board of Realtors North Carolina Association of Realtors National Association of Realtors

CERTIFICATION:

State Certified General Real Estate Appraiser for North Carolina, #A281

OTHER:

NC Property Tax Commission, 2013 – Present
Durham Civilian Police Review Board, 2009 - Present, Past Chair
Durham County Board of Equalization and Review, 2013 – Present
Durham Public Schools Budget Advisory Committee, 2013 - Present
City of Durham Audit Oversight Committee, 2002 – 2006
Durham Board of Adjustment, 1994 - 2002
Durham City/County Zoning Commission, 1990 – 1995
John Avery Boys and Girls Club, 1994-2002
Historical Preservation Society, 1992 - 1995
Vice President of the Candidates, 1989, NC Chapter 40
President of the Candidates, 1990, NC Chapter 40
Candidate of the Year, 1990, NC Chapter 40

RECENT CLIENTS (within the past five years):

LENDING INSTITUTIONS
American National Bank & Trust Company
AMEX Financial
BB&T

Citizens National Bank

CommunityOne Bank NA

Fidelity Bank

First South Bank

Harrington Bank

KeySource Commercial Bank

Live Oak Banking Company

Mechanics & Farmers Bank

Pacific International Bank

PNC Bank

RBC Bank

Self-Help

State Farm Bank

SunTrust Bank

Wells Fargo Bank

MUNICIPALITIES AND OTHER GOVERNMENT AGENCIES

Chapel Hill Transit

City of Durham

NC Department of Administration

Durham County

Durham Public Schools

Durham Technical Community College

Housing Authority of the City of Durham

NCDOT

Orange County

Orange Water and Sewer Authority

Person County

Town of Chapel Hill

OTHER

Allenton Management

AND Associates

Barcosnic

Builders of Hope

BCG Properties

Blanchard, Miller, Lewis & Styers Attorneys at Law

Blue Cross & Blue Shield of NC

Boulevard Proeprties

Bugg & Wolf Attorneys at Law

Carolina Land Acquisitions

CRC Health Corporation

Development Ventures Inc.

Duke Energy

Durham Academy

Durham Rescue Mission

Durham Technical Community College

Edward Jones Trust Company

Farrington Road Baptist Church

Forest History Society GBS Properties of Durham, LLC Hayden Stanziale Georgia Towers, LLC Hawthorne Retail Partners Integral **Investors Title Insurance IUKA** Development Joelepa Associates LP John and Mary Hebrank LCFCU Financial Partners Manor Associates McDonald's USA Mt. Gilead Baptist Church Northgate Realty, LLC Property Advisory Services, Inc. Rand Enterprises Research Triangle Foundation Sehed Development Corporation Simba Management Southwest Durham Partners, LLC Stirling Bridge Group, LLC Styers, Kemerait & Mitchell, PLLC Talbert & Bright Attorneys at Law Teer Associates Thalle Construction The Bogey Group **TKTK Accountants** Treyburn Corporate Park, LLC

Trinity Properties
UNC Hospitals
Voyager Academy
Wilhekan Associates

In addition, Mr. Smith has made appraisals for other lending institutions, municipalities, individuals, corporations, estates and attorneys. Appraisal assignments have been made throughout the Triangle, North Carolina, and South Carolina.

Properties appraised include all types of single family residential, multi-family residential, office, retail, commercial, industrial, churches, schools and other specialty type uses, vacant and improved, existing and proposed.

Appraisal assignments were for a variety of purposes including: mortgage loans, estate planning, condemnation, bankruptcy and equitable distribution.





Street Scene along Estes Drive



Street Scene along Estes Drive



Hartig Street Dead End into Property



805 Hartig Street



804 Hartig Street



Powell Street Dead End into Property



819 Powell Street



820 Powell Street



Ward Street Dead End



832 Ward Street



Justice Street Dead End into Property



211 Justice Street



200 Justice Street



Municipal Buildings Contiguous to Justice Street



Municipal Building Contiguous to Justice Street



Municipal Building Contiguous to Justice Street



Justice Street Property Seen from Contiguous Parking Lot



Justice Street Property Seen from Contiguous Parking Lot



April 2, 2018

0.16 mi

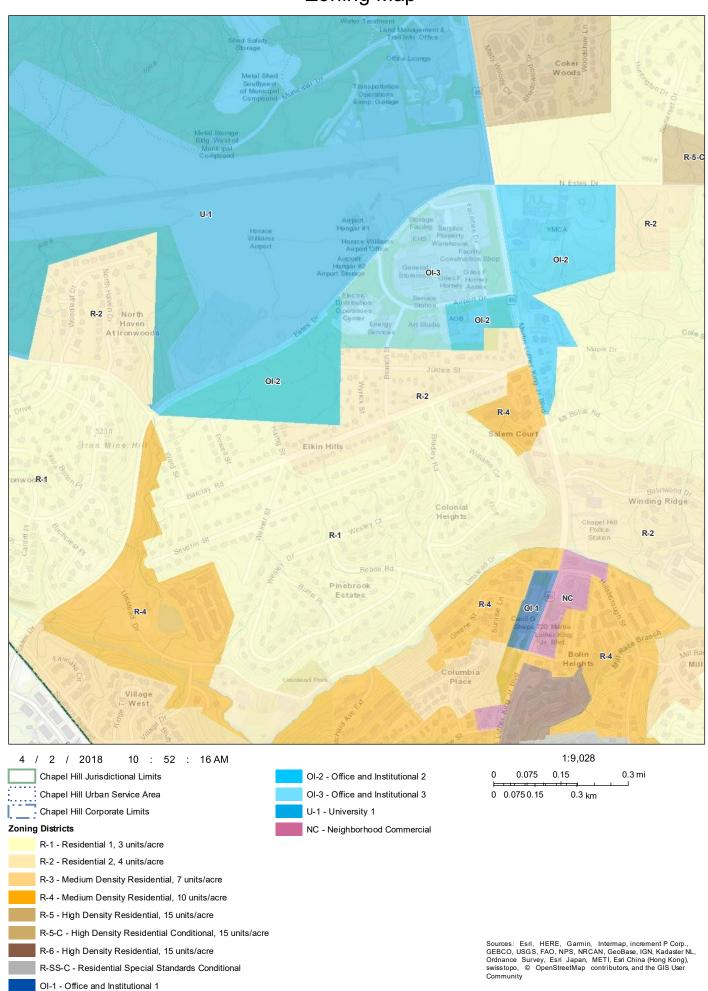
0.04

0.2 km

0.1

0.05

Zoning Map



GIS/Tax Data



This map contains parcels prepared for the inventory of real property within Orange County, and is compiled from recorded deed, plats, and other public records and data. Users of this map are hereby notified that the aforementioned public primary information sources should be consulted for verification of the information contained on this map. The county and its mapping companies assume no legal responsibility for the information on this map.

2.5 A

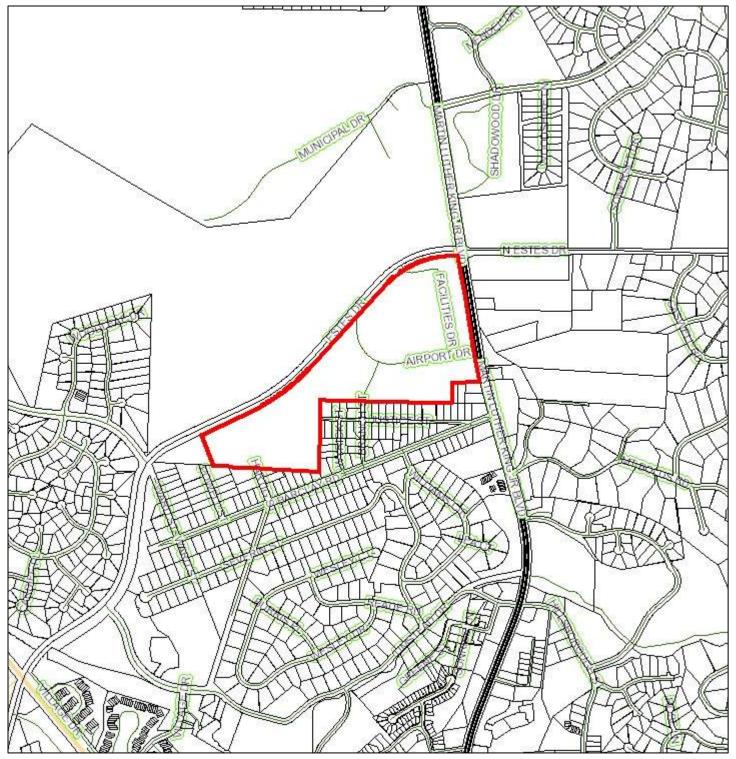
BUILDING COUNT:

SIZE:

April 2, 2018

PIN:	9789033163	SIZE:	2.5 A	BUILDING COUNT:		
OWNER 1:	UNIVERSITY OF N C ENDOWMEN	TELENTO PREUTS	T ≝9 45/544	LAND VALUE:	\$0	
OWNER 2:		RATECODE:	22	BLDG_VALUE:	\$0	
ADDRESS 1:	ENDOWMENT FUND TRUSTEES	DATE SOLD	: 12/14/1984	USE VALUE:	\$0	
ADDRESS 2:		BLDG SQFT	:	TOTAL VALUE:	\$0	
CITY: (CHAPEL HILL	YEAR BUILT	:		1:4,800	
	NC 27514 S/S SR 1780			0 0.04 	0.08 0.2 km	0.16 mi

Tax/GIS Map



This map contains parcels prepared for the inventory of real property within Orange County, and is compiled from recorded deed, plats, and other public records and data. Users of this map are hereby notified that the aforementioned public primary information sources should be consulted for verification of the information contained on this map. The county and its mapping companies assume no legal responsibility for the information on this map.

April 2, 2018

SIZE: 59.233 A **BUILDING COUNT:** PIN: 9789247373 \$355,400 UNIVERSITY OF N C DEED REF: 000/000 LAND VALUE: OWNER 1: \$0 BLDG_VALUE: RATECODE: 22 OWNER 2: USE VALUE: \$0 PROPERTY OFFICE DATE SOLD: 10/22/2002 ADDRESS 1:

TOTAL VALUE: \$355,400 ADDRESS 2:

YEAR BUILT:

BLDG SQFT: CITY: CHAPEL HILL

1:12,000 0.2 STATE, ZIP: 0.1 0.4 mi LEGAL DESC: S/W INT HWY 86 & ESTES DR 0.15 0.6 km

- MUNICIPAL SERVICES BUILDING 72,000 SQ. FT. X 3.5 FLOORS
- TOWN FUTURE BUILDING 28,000 SQ. FT. X 3.5 FLOORS
- UNIVERSITY FUTURE BUILDING 100,000 SQ. FT. X 3 FLOORS
- SURFACE PARKING

SITE SECTION 'B'

SITE SECTION 'A'

0

S

4

S

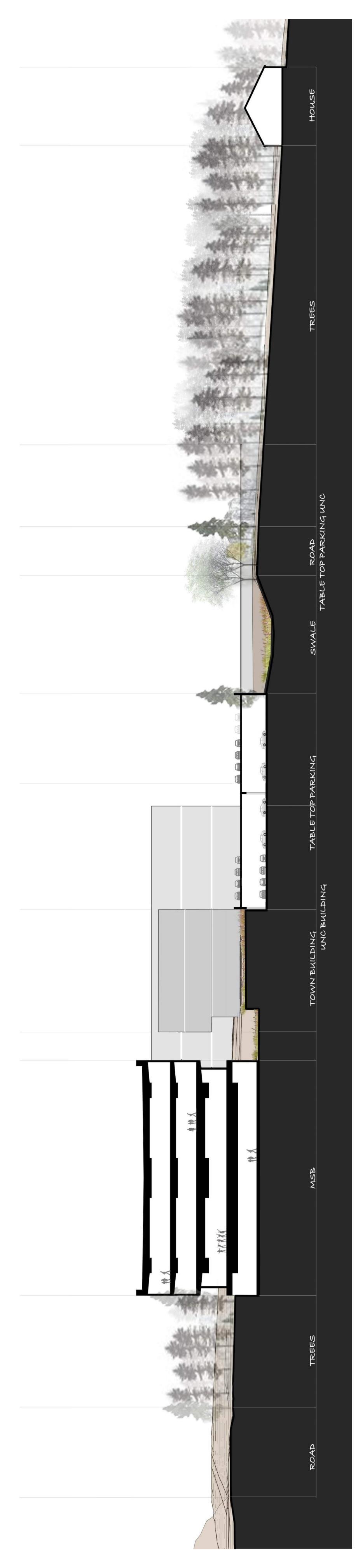
- TABLE TOP STRUCTURED PARKING
- STORM WATER CONTROL MEASURE SCM
- STREAM CENTERLINE
- 50 FT. STREAM BUFFER



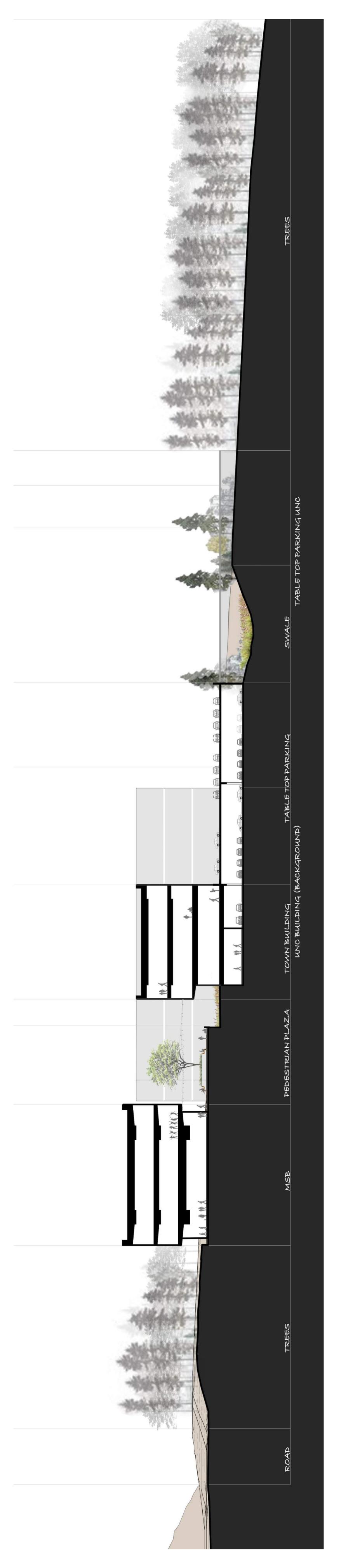








SITE SECTIONS0 3 . 0 1 . 2 0





SITE SECTIONS 0 3 . 0 1 . 2 0





Exhibit L - Site Development Permit Application



TOWN OF CHAPEL HILL Planning Department

405 Martin Luther King Jr. Blvd. Chapel Hill, NC 27514-5705

phone (919) 969-5066 fax (919) 969-2014 www.townofchapelhill.org

U-1 Site Development Permit Application – University Joint Operations DA

A Site Development Permit is required for the construction of specific buildings described in the Town and University Joint Operations Development Agreement. The Town must, within 15 working days of submission of a complete application, either approve, approve with conditions, or deny the permit. For additional information, please contact the Planning Department at (919) 969-5066 at planning@townofchapelhill.org.

	Accepted Dat	e:	Deadline:		
Section A: Project	Information				
Project Name:					
Application Num	ber:		Submittal I	Date:	
Parcel Identifier I	Number (PIN):				
Project Location:					
Project Description	on				
Section B: Contact Applicant Informa Name: Address:		orrespondence v	vill be mailed)		
City:			State:	Zip Code:	
Phone Number:			Email:		
The undersigned a with this application			the best of their knowledg	e and belief, all informat	ion supplied:
Applicant Signatu	ıre:			Date:	
University Autho	rization (print na	ıme):			
University Signat	ure:				

U-1 Site Development Permit Application

Section C: Floor Area

Pre-Application Floor Area:	
Additional Floor Area proposed with this project:	
Floor Area to be demolished with this project:	
NEW TOTAL FLOOR AREA:	

Section D: Vehicle Parking Spaces

Pre-Application Parking Spaces:	
Number of Parking Spaces to be added with this project:	
Number of Parking Spaces to be lost with this project:	
NEW TOTAL PARKING SPACES:	

Section E: Impervious Surface

Pre-Application Impervious Surface Area:	
Additional Impervious Surface Area proposed with this project:	
Impervious Surface to be removed with this project:	
NEW TOTAL IMPERVIOUS SURFACE:	

Section F: Impervious Surface

Pre-Application Impervious Surface Area:	
Additional Impervious Surface Area proposed with this project:	
Impervious Surface to be removed with this project:	
NEW TOTAL IMPERVIOUS SURFACE:	

U-1 Site Development Permit Submittal Requirements

contact the	Chap	ust accompany your application. For questions concerning applicability of the requirements, please all Hill Planning Department at (919) 969-5066 or at planning@townofchapelhill.org . Please provided the location of the submittal requirements (i.e. Sheet #, Attached).							
	1. Application fee (refer to fee schedule) 2. Digital Files – provide digital files of all plans and documents 3. Minor Modification Approval letter, if applicable								
		If-sized Site Plan Set (one copy)							
	5. Pla 6. Do	n Sets (full-sized) 8 copies folded cumentation of pre-submittal community meeting and 1000' notification. Is a pre-application meeting held? Yes No							
		egible and clearly drawn. All sheets should be signed and sealed by a Licensed Professional in the arolina. All plan set sheets should include the following:							
LaNoPro	-	rrow y Boundaries ngineering), denoted graphically and numerically							
Information (Sheet)	n for e	each of these categories should be included with application, when applicable.							
	7.	Area Map a) Identify location b) Dedicated open space, parks, greenways, c) Resource Conservation District, if applicable d) Property lines, zoning district boundaries, land uses, project names of site and surrounding properties, significant buildings, corporate limit lines e) Existing roads (public & private), rights-of-way, sidewalks, driveways, vehicular parking areas, bicycle parking, handicapped parking, street names							
	8.	 Existing Conditions a) Slopes, soils, environmental constraints, existing vegetation, and any existing land features b) Location of all existing buildings and uses c) Existing utilities & easements (location & sizes of water, sewer, electrical, & drainage lines) d) Nearest fire hydrants 							

f) Existing topography at minimum 2-foot intervals and finished grade of structure g) Natural drainage features & water bodies, floodways, floodplain, RCD, & Watershed

e) Nearest bus shelters and transit facilities

boundaries

Detailed Site Information Proposed building locations and footprints b) Access drives Pedestrian circulation (including connections to existing pedestrian networks) c) d) Location of proposed fire hydrants Location and dimension of all vehicle parking areas, entrances, exits, and drives Clearing limits 10. Grading Plan 11. Erosion and Sediment Control Erosion and sediment control plan (2 copies) for projects over 20,000 square feet of disturbance (COURTESY COPY) b) Copy of NC Division of Land Resources erosion and sediment control permit application for projects disturbing one acre or more (COURTESY COPY) 12. Utility Plan a) Show the location of utilities and any reserved access areas. b) Letters of approval from each applicable utility 13. Transportation Traffic Impact Study completed by Town's consultant (or exception). Transportation Management Plan (TMP) 14. Construction Management Plan a) To be consistent with the University's "Construction Management Guidelines" contained in the August 7, 2001 Addendum, to be approved by the Town Manager prior to commencement of construction, indicating measures to be taken during construction If any part of a University project lies within a public right-of-way on Town streets, all documents required for an Engineering Construction Permit (ECP) must be submitted as part of the Site Development Permit application process. An ECP must be obtained from the Town prior to start of work on the right-of-way. 15. Traffic Management Plan A traffic management/control plan shall be submitted which will provide for the safe and orderly movement of motorized vehicles, non-motorized vehicles, or pedestrians on any public streets (maintained and operated by the Town or the NCDOT) on which normal traffic flow will be disrupted (lane closures, street closures, sidewalk closures, etc.) during construction, including a detour plan as may be necessary.

corridors during construction.c) The traffic control plan, for public streets, shall be prepared in accordance with the latest version of Manual on Uniform Traffic Control Devices (MUTCD) and applicable NCDOT

A construction traffic routing map that identifies the local street network that will be impacted by construction traffic associated with the proposed addition. The routing plan shall minimize, and where practical, prohibit construction traffic on local streets. The plan shall also provide for the safe and continued use of bicycle, pedestrian, and greenway

- d) All signs shall be made of high retro-reflectivity material and, if night time closures are necessary, signs shall include beacons.
- e) A permit must be obtained from the Town Manager for lane closures, sidewalk closures, and street closures in the public right-of-way. The plan shall state that closures and night time work on public streets must be approved by the Town Manager. This form must be completed and submitted to the Town for approval along with the relevant traffic control plan. The Contractor must notify the Public Works, Police, Fire, and Transportation Departments, E911, and affected residents at least 3 business days prior to the commencement of approved night work and/or approved street closure.

16. Pedestrian Management Plan

- a) A pedestrian detour plan shall be submitted which will provide for the safe and orderly movement of pedestrians if any public sidewalks and/or designated pedestrian routes on public sidewalks and/or designated pedestrian routes on public rights-of-way and/or easements are disrupted or closed to normal pedestrian use.
- b) The pedestrian detour plan involving public right-of-way shall be prepared in accordance with the latest of the MUTCD and sealed by a professional engineer licensed in the State of North Carolina.
- c) All signs shall be made of high retro-reflectivity material and if a night time detour is required, signs shall include beacons.
- d) Detailed information on how the plan will provide for the safe and continued use of bicycle, pedestrian, and greenway corridors during construction.
- e) Detailed information on pedestrian security measures including the placement of security phones and lighting. The plan shall also include additional information on security for pedestrian bridges during evening hours.

17. Construction Traffic Management Plan

- a) The plan shall be submitted which will provide for the safe and orderly movement of construction traffic to and from the construction site and staging area. The plan must identify the location and size of staging areas and material storage areas which would affect construction traffic routes.
- b) The plan shall indicate location(s) where construction equipment will be parked (if offsite) and where construction personnel will park, including a routing plan for equipment and personnel going to and from the work site.
- c) The plan shall indicate graphically and describe how emergency vehicle access to and around the project site will be provided both during and after completion of construction
- d) The plan shall include measures to minimize construction traffic impacts on school bus traffic and access to schools in the vicinity of the project.
- e) Each construction site will include visible signage listing a telephone number and a University representative available to answer questions and respond to concerns about pedestrian safety and security. A detail of the proposed sign, including number of signs and general locations, shall be submitted with each Site Development Permit application.

18. Stormwater Management

a) A signed and sealed letter from a Professional Engineer, licensed in the State of North Carolina, certifying that the stormwater management measures associated with the Site Development Permit application meet or exceed the approved stormwater management performance standards for Carolina North. Stormwater management and treatment practices shall comply with all applicable federal and State regulations, and revisions thereof.

- b) Plans, signed and sealed by a Professional Engineer, licensed in the State of North Carolina, showing grading, regulatory floodplains, stream buffers and RCDs, plantings, erosion control, and stormwater runoff control best management practice(s) designs and details, in accordance with the performance criteria.
- c) A stormwater design report, signed and sealed by a Professional Engineer, licensed in the State of North Carolina, that includes the following:
 - (i) A narrative description of existing and proposed stormwater management measures
 - (ii) A summary table of pre-development and post-development impervious cover by drainage area
 - (iii) Plan showing the pre-development and post-development impervious cover
 - (iv) A table indicating the stormwater runoff volumes and peak discharge (rates) for the specified storms at the following three conditions: predevelopment; post-development without stormwater management; and post-development with stormwater management.
 - (v) Copies of all hydrologic and hydraulic calculations and routings
 - (vi) Maps indicating the existing and proposed drainage divides on the site and the drainage to each stormwater management measure
 - (vii) Inspection, operations, and maintenance plans for the proposed stormwater management structures (for information only)
- d) A note indicating the University is responsible for maintenance of stormwater facilities as agreed upon in the Carolina North Development Agreement for facilities on Universityowned property.

19. Noise

a) Every application for a Site Development Permit shall include a signed and sealed letter from a Professional Engineer, licensed in the State of North Carolina, and with demonstrable expertise in acoustical design and attenuation practices, certifying that any increase in measurable noises above existing pre-Development Plan noise levels on property outside the U-1 Zoning District will not exceed the levels allowed in the Town Noise Ordinance as established at the time each Site Development Permit application is approved by the Town. This noise restriction shall not apply to property outside of the U-1 Zoning District that is in the same ownership as property within the U-1 Zoning District.

20. Lighting

- a) Existing and proposed lighting fixture types and locations
- Isolux contour diagram and grid points with the measured and calculated pre-development and post-development foot-candles at grade on property where lighting impacts are expected.
- c) A description of how lighting meets the goals of 'dark skies' lighting and complies with the lighting standards of the Development Agreement.
- d) A demonstration that there is no increase in lighting foot-candle levels at the adjacent property line as a result of this project.
- e) Demonstration that increases in illumination on property outside of the Carolina North Project do not result in lighting levels in excess of 0.3 foot candles, measured at ground level.
- f) Confirmation that, for property outside of the Carolian North Project where existing ambient lighting levels are in excess of 0.3 foot-candles, there is no increase in measurable lighting levels.

21. Fire Protection and Safety

- a) Fire Flow Report sealed by a registered Professional Engineer, licensed in the State of North Carolina, demonstrating compliance with Town Standards.
- b) Locations of existing and proposed fire hydrants and Fire Department Connections (FDC).
 Hydrant spacing shall comply with the Town Design Manual and FDC locations shall be within 100 feet of hydrant.
- c) Any and all roads, driveways, or dedicated fire lanes used for fire department access shall be all-weather and designed to support fire apparatus weighing at least 80,000 lbs.
- d) Fire access roads shall have a minimum width of 20 feet with overhead clearance of 13 feet 6 inches for all structures not exceeding 30 feet in height and shall provide access to within 150 feet of all exterior portions of the building. Structures of 3 stories or exceeding 30 feet in height shall have an access road of at least 26 feet in width. Proximity to the building for aerial apparatus shall have at least one of the required access routes be located within 15 feet and a maximum of 30 feet from the building and shall be positioned parallel to one entire side of the building.
- e) All turns, radii, bridges, and depressions within the roadway shall be designed and constructed to be accessible by the largest fire apparatus operated by the Town of Chapel Hill.
- f) Provide details on signage, bollards, and/or markings used to prevent parking that would hinder access.
- g) If a Certificate of Occupancy is issued by the Town of Chapel Hill, fire protection devices, where required by the NC Fire Prevention Code, Town of chapel Hill Code (Chapter 7), and applicable NFPA codes will be required to be permitted, installed, tested, and approved.
- h) Fire apparatus access roads shall be installed and made serviceable prior to and during the time of construction, except when approved alternate means of protection are provided and approved by the AHJ.
- During construction, vehicle access for firefighting shall be provided. Temporary street signs shall be installed at each street intersection when construction allows the passage of vehicles. Signs shall be of an approved size, weather resistant, and maintained until replaced by permanent signs.
- j) Please include the following note on the submitted plans: During demolition and/or construction, all aspects of the NC Fire Prevention code shall be followed. The owner/developer shall designate one person to be the Fire Prevention Program Superintendent who shall be responsible for enforcing Chapter 14 of the NC Fire Prevention Code (Chapter 14) and the on-site fire prevention program and ensure that is carried out through completion of the project.
- k) When the building being constructed requires standpipes, a temporary standpipe connection will be constructed with ready Fire Department Access when the building is not more than 40' in height. Such standpipes shall provide usable connections adjacent to the stairs and shall continue with building progression always being not more than one floor below the highest floor of the building. NC FPC 2012 Section 1413.
- l) Fencing around project to include access gates with a 20 foot swing or slide motion. Any areas which will be inaccessible for firefighting or rescue operations shall be noted. Emergency access designation for apparatus shall be provided.
- m) An approved water supply for fire protection, either temporary or permanent, shall be made available as soon as combustible material arrives on the site. NC FPC 1412.

	n)	Energy	and	Conservation	Goals
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- a) Permit applications shall include energy performance models and the LEED checklist with project points to qualify at Silver level.
- b) Permit applications shall describe how potable water consumption and reclaimed strategies are incorporated into the project.
- c) Energy Management Plan including the following:
 - Description of how project will be 20% more energy efficient than ASHRAE standards
 - II. Description of utilization of sustainable forms of energy (Solar, Wind, Hydroelectric, and Biofuels)
 - III. Participation in NC GreenPower program
 - IV. Description of how project will ensure indoor air quality, adequate access to natural lighting, and allow for proposed utilization of sustainable energy
 - V. Description of how project will maintain commitment to energy efficiency and reduced carbon footprint over time
 - VI. Description of how the project's Transportation Management Plan will support efforts to reduce energy consumption as it affects the community

o) Landscape

- a) Plans indicating grading and limits of construction, existing and proposed tree canopy mix and percent coverage, and other proposed plantings.
- b) Identify planting materials, proposed irrigation and maintenance techniques for each type of landscape proposed.
- c) Applications should include any proposed trails, greenways or recreation facilities, and connections to adjacent related facilities to be constructed as part of the project and describe the design standards applied to those elements.

p) Exterior Elevations

a) An outline of each elevation of the building, including the finished grade line along the foundation (height of building measured from mean natural grade)

Elkin Hills Project Area

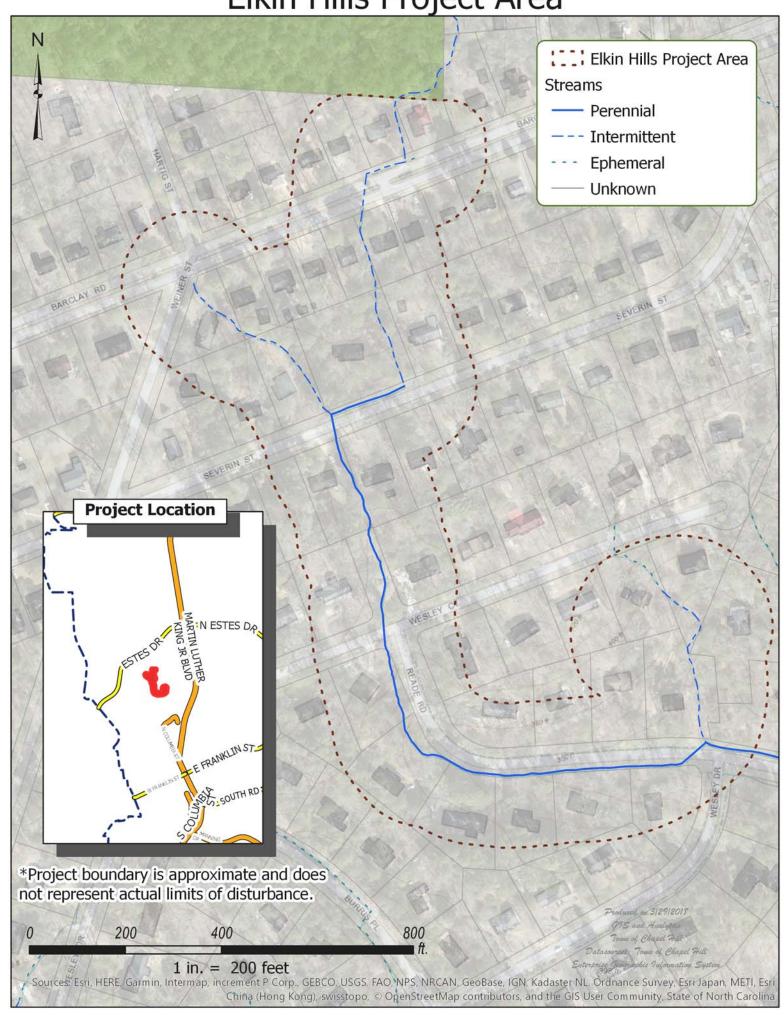


Exhibit N - Advisory Board Recommendations

PLANNING COMMISSION

The charge of the Planning Commission is to assist the Council in achieving the Town's Comprehensive Plan for orderly growth and development by analyzing, evaluating, and recommending responsible town policies, ordinances, and planning standards that manage land use and involving the community in long-range planning.

REVIEW AND COMMENT OF PLANNING COMMISSION TO THE TOWN COUNCIL

April 17, 2018

Municipal Services Center Development Agreement

The Planning Commission reviewed the Municipal Services Center draft development agreement and provided the following comments and questions regarding the development agreement:

- The Commission was interested in what type of permitting process the future University building would be subject to.
- The proposed land uses are concerning in that evening activities could be permitted that would affect adjacent properties. The Commission was interested in restricting the types of land uses and/or hours of activities.
- The Commission would like to see the future community engagement structure refined in the Development Agreement.
- The Commission supports the proposed buffer.
- The Commission supports the recommendations of the Transportation and Connectivity Advisory Board.
- The Commission supports the connectivity proposals.
- Change the phrasing regarding proposed albedo from "low" to "high"
- The University should be present during public discussion of this project.
- The Commission supported the use of permeable pavers.
- The Commission would like to see the parking deck evaluated for potential future repurposing.
- The Commission wanted to clarify that street parking should not be provided on Estes, although is comfortable with street parking on the internal drive aisles.
- The Commission asked that angled parking be proposed to reduce parking surface.

Prepared by: Aaron Frank, Planning and Development Services Staff

ENVIRONMENTAL STEWARDSHIP ADVISORY BOARD

The charge of the environmental stewardship advisory board will be to assist the Chapel Hill Town Council in strengthening environmentally responsible practices that protect, promote and nurture our community and the natural world through advice and program support.

REVIEW AND COMMENT OF ENVIRONMENTAL STEWARDSHIP ADVISORY TO THE TOWN COUNCIL

April 10, 2018

Municipal Services Center Development Agreement & Rezoning

The Environmental Stewardship Advisory Board reviewed the Municipal Services Center draft development agreement and rezoning and provided the following comments and questions regarding the development agreement:

- The development should strive to meet AIA 2030 goals, and the Land Use Management Ordinance may be amended to require this.
- The development should strive for nearly net zero runoff, while providing stormwater runoff to the wetland areas on the site.
- Three to four building stories provides optimal opportunities to construct an environmentally efficient building.
- The access road should be designed to the north or south of existing location to reduce encroachment within the Resource Conservation District.
- The Board asked if specific access or driveway requirements exist for police vehicles.
- The Board asked if an agreement could be reached to reduce audible presence of police sirens.
- The Board asked for quantifiable outcomes in stormwater management, and that stormwater runoff achieve a measureable reduction from existing conditions.
- The Board asked that the applicant agree to native plantings. Additional preference for drought tolerance was stated following the meeting
- The Board asked that the applicant ensure that no light trespass be permitted.
- The Board asked that a box culvert be provided.
- The Board asked that the stream determinations be provided to them.

Prepared by: John Richardson, Community Resilience Officer, Staff Liaison to ESAB

TRANSPORTATION AND CONNECTIVITY ADVISORY BOARD

To assist the Chapel Hill Town Council in creating an inclusive connected community by recommending, advocating and planning for comprehensive, safe, effective and sustainable multi-modal transportation and connectivity

Municipal Services Building Development Agreement Comments on the proposed standards and concept plan March 27, 2018

- The Board wants to see this site maximize access to transit. The Hartig St. path connection will help people access the A route bus stop on Barclay. The connection will provide a direct bike route downhill to downtown Chapel Hill using the greenway systems and local roads without getting on Estes Dr.
- They're interested in making sure the site has good connectivity from Hartig path connection through the parking lot to the other buildings. This means providing safe places to walk and bike—installing sidewalks and crosswalks.
- The Hartig St. path connection needs to be ADA compliant, 10 feet, and paved.
- Board members like the idea that the cross section on Estes Drive Extension will match the design that will be constructed on Estes Drive (MLK –Caswell). They would like to see the traffic signal at MLK and Estes to give preemption to bicyclists and pedestrians.
- If traffic signals are not warranted on Estes Drive Extension, the Board recommends crosswalks using Rectangular Rapid Flashing Beacons or other safe pedestrian crossing treatments.
- The Board would like staff to take a look at bus pull off on Estes Drive Extension. There is a concern that if buses stop in the travel lane that cars will try to pass the bus using the center turn lane.
- The Board is interested in seeing good lighting along the sidewalk. Cars speed on Estes Drive and with the curvy road it currently hard to see people walking or biking in the dark.
- The Board would like to see electric vehicle charging spaces provided and conduit for future expansion.
- The Board would like staff to evaluate how the parking deck can be constructed to be reused for other purposes in the future as transportation technology changes.
- The Board recommends that the travel lanes on Estes Drive Extension do not exceed 11 ft wide.
- The Board would like to make sure Police and Fire weigh in on the site and circulation to make sure it meets the needs for emergency vehicles.

COMMUNITY DESIGN COMMISSION REVIEW COMMENTS MUNICIPAL SERVICES CENTER March 27, 2018

The Commission offered the following comments to the applicant:

- In response to a citizen comment, the Commission was interested in the real estate assessment that would show the impact of the development on neighborhood property values.
- The buildings appear buffered from Estes Drive. Why not move the building toward the street? The applicant replied that it was proposed to be close to the street to allow additional buffer from the Elkin Hills neighborhood. The illustrative site plan made it appear the building was separated from the street by vegetation.
- Wanted information on bus stops and wanted to see stops with shelters.
- The CDC should review/approve the Building Elevations our own Town projects, not just a Courtesy Review.
- The new police building should include daylighting to make it bright inside.
- It appeared that community members chose images that were urban in appearance.
- Arrange the buildings to create site lines (terminated vistas) and defined common areas
- Look at adding trees between the sidewalk and street
- Look at adding on-street parking

Prepared By: Dixon Pitt, Chair, Community Design Commission Kay Pearlstein, Staff

MEMORANDUM

TO: Mayor and Town Council

FROM: Stefan Klakovich, Chair, Stormwater Management Utility Advisory Board

SUBJECT: Municipal Services Center (Estes Dr. Ext.) Development Agreement –

Stormwater Advisory Board recommendations

DATE: April 25, 2018

The Stormwater Management Utility Advisory Board (Board) appreciates this opportunity to provide recommendations to the Mayor and Town Council members for their consideration during the Council's deliberations on the development agreement for the proposed Municipal Services Center on Estes Drive Extension.

Following a presentation at its April 24, 2018 meeting by Mary Jane Nirdlinger and Dennis Pitts, the Board discussed and approved an initial set of recommendations listed below.

1. Peak Discharge Rate Limits to include the 50-year storm

The Stormwater Advisory Board recommends that the 50-year 24-hour storm event be included to the peak discharge rate limits requirements. This is in addition to the 1-year, 2-year, and 25-year 24-hour storm events. This is consistent with the peak discharge rate requirements contained in the Carolina North Development Agreement

Further, the Board recommends that increase in impervious surfaces associated with the Estes Dr. Ext. road improvements be included in the peak discharge rate calculations.

2. Flood Storage Study

In response to the concerns of increased flooding from the Elkin Hills residents, the Board recommends that a study be conducted for this catchment area in order to identify potential flood storage sites that could provide a greater community benefit.

The Board appreciates the improvements already made to the plans in response to input from residents and advisory boards. This includes the change to the "loop road" by realigning the stream crossing (closer to perpendicular), which shortened its length and reduced its width.

The Board encourages the Town Council to consider other strategies to lessen the stormwater impacts to adjacent properties and reduce the amount of impervious surfaces such as green roofs and deleting the median along the east driveway.

Motion made by Mr. Bevington; Mr. Hearn seconded the motion. It passed unanimously.

TOWN OF CHAPEL HILL MUNICIPAL SERVICES CAMPUS

TRAFFIC IMPACT STUDY



Prepared for:

The Town of Chapel Hill
Public Works Department - Engineering

Prepared by: HNTB North Carolina, PC

343 East Six Forks Road Suite 200 Raleigh, NC 27609

NCBELS License #: C-1554

February 2018



TOWN OF CHAPEL HILL MUNICIPAL SERVICES CAMPUS

TRAFFIC IMPACT STUDY



Prepared for:

The Town of Chapel Hill Public Works Department - Engineering

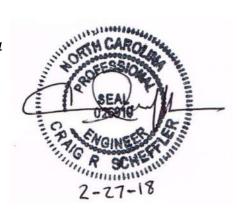
Prepared by:

HNTB North Carolina, PC

343 East Six Forks Road Suite 200 Raleigh, NC 27609

NCBELS License #: C-1554

February 2018





Town of Chapel Hill: Traffic Impact Study Town of Chapel Hill Municipal Services Campus - Proposed Institutional Development

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I. EXISTING CONDITIONS

A. Project Overview

A new municipal services campus for the Town of Chapel Hill is being proposed along the Estes Drive Extension near its intersection with Seawell School Road. The project proposes to create multiple buildings for Town services and potential uses for future office space for the University of North Carolina – Chapel Hill (UNC). **Figure 1** (found in *Appendix A*) shows the general location of the site. Phase 1 of the project is anticipated to be fully complete by 2020. This report analyzes the Phase 1 build-out scenario for the year 2021 (one year after anticipated completion of Phase 1), the no-build scenario for 2021, as well as 2017 existing year traffic conditions. Future phased development of the site will require updates to this Phase 1 traffic impact study when that development occurs.

The proposed site concept plan shows a provision for two full movement access driveways that connect to the Estes Drive Extension. No other vehicular access connections are proposed. The site driveways are proposed to have internal connectivity with on-site parking areas. **Figure 2** displays the preliminary concept plan of the Town of Chapel Hill Municipal Services Campus and nearby land uses and roadways. The site is expected to provide approximately 168 parking spaces on surface lots in its first phase.

B. Site Location and Study Area

This report analyzes and presents the transportation impacts that the Town of Chapel Hill Municipal Services Campus will have on the following intersections in the project study area:

- Estes Drive Extension and Seawell School Road
- Estes Drive Extension and UNC Student RR Lot / Future West Site Driveway
- Estes Drive Extension and Potential Future Carolina North Access / Future East Site Driveway
- Estes Drive Extension and Airport Drive
- Estes Drive Extension / Estes Drive and NC 86 (Martin Luther King, Jr. Boulevard)
- NC 86 (Martin Luther King, Jr. Boulevard) and Airport Drive

The impacts of the proposed site at the study area intersections will be evaluated during the AM, noon, and PM peak hours of an average weekday. The following study is based on background traffic for the existing year, 2017, the year following the estimated Phase 1 site build-out (2020), as well as the estimated Phase 1 site-generated traffic produced by the Town Services Campus.

There are several Town-approved or anticipated future developments in, or just beyond, the immediate project study area that were considered to be constructed by 2021 and may generate additional background traffic. An area-wide ambient future traffic growth percentage of 1.0 percent per year was applied to the existing volumes, based on historical average annual daily traffic (AADT) growth rate data provided by the Town of Chapel Hill and NCDOT, and consistent with recent traffic impact studies near the project study area.

C. Site Description

The Town Municipal Campus site is currently owned by UNC and contains a large amount of undeveloped, wooded land. It borders Carolina North property to the north and single-family residential developments to the west and south. It also borders UNC property to the east. Additional residential subdivisions, commercial and institutional developments are present along the NC 86 (Martin Luther King, Jr. Boulevard) corridor in the project study area.

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The site has frontage along the Estes Drive Extension which will provide all vehicular access. The proposed site concept, shown in **Figure 2**, indicates all parking for Phase 1 will be accommodated on-site, through the use of surface parking facilities. The two proposed site driveways will include internal connections to parking areas adjacent to the proposed campus buildings. The proposed driveways are located to form future four-legged intersections with the existing UNC Parkand-Ride facility and a future connection to Carolina North development.



D. Existing and Proposed Uses in Vicinity of Site

The land uses and development in the study area are primarily residential and institutional, with some commercial areas located along Martin Luther King, Jr. Boulevard. The Existing Land Use Plan shown in the 2021 *Town of Chapel Hill Comprehensive Plan* and adopted June 25, 2012, indicates that the proposed site is designated as "Institutional". The Future Land Use Plan, that is also a part of the Town Comprehensive Plan, indicates that the parcel would be considered as "University". The Comprehensive Plan also indicates that this parcel is a "Future Focus Discussion Area – S. MLK Jr. Boulevard / Homestead Road to Estes Drive". The parcel is currently zoned "OI-2" – delineating it as "office and institutional use".

E. Existing and Committed Surface Transportation Network

Roadways

The Town of Chapel Hill Municipal Services Campus project study area features several major and minor arterial roadways serving areas throughout the Town of Chapel Hill and points beyond, as well as a number of collector and local access streets. **Table 1** summarizes pertinent information on the study area roadway facilities.

Table 1. Existing Study Area Roadways

Road Name	Functional Classification*	Study Area Cross-Section	2015 AADT	Speed Limit	Sidewalk	On-Street Parking
N.C. 86 (Martin Luther King, Jr. Blvd)	Other Principal Arterial	5 lane undivided with TWLTL	29,000	35	Υ	N
Estes Drive	Minor Arterial	2 lane undivided	16,000	35	Υ	N
Estes Drive Extension	Minor Arterial	2 lane undivided	13,000	35	Ν	Ν
Seawell School Road	Minor Collector	2 lane undivided / 3 lane undivided with TWLTL	4,100	35	Υ	N
Airport Drive	Local	2 lane undivided	N/A	25	S	N

S – Some Sidewalk/Parking Present TWLTL – Two-Way Left-turn Lane

HNTB

^{* -} As defined on the NCDOT Functional Classification web page http://ncdot.maps.arcgis.com/home/webmap/viewer.html

AADT data was taken from 2015 AADT mapping produced by the NCDOT Traffic Survey Unit. **Figure 3** shows the existing lane configuration, traffic control, and speed limits for these study area roadways. Detailed descriptions of several of the major study area roadways are as follows:

- N.C. Highway 86 (Martin Luther King Jr, Blvd) is a principal arterial in the study area, serving areas from I-40 (via Martin Luther King Jr. Boulevard) to downtown Chapel Hill and the US 15-501 corridor to the south. In the study area vicinity, Martin Luther King, Jr. Boulevard is a five-lane undivided section (with two-way left-turn lane). There are multiple driveway access points along the roadway and several major street intersections. No on-street parking is permitted along N.C. 86 in the project study area. Several bus stops are located along the facility. The posted speed limit is 35 mph in the study area.
- Estes Drive / Estes Drive Extension is a minor arterial that connects areas of west and north Chapel Hill. In the study area, Estes Drive and Estes Drive Extension are undivided facilities with two-lane cross-sections near NC 86 and both have a 35 mph speed limit. Auxiliary turn-lanes are present at major intersections. Sidewalk is present on at least one side of the roadway along Estes Drive, but no sidewalk is present along Estes Drive Extension. Several bus stops are located along the facilities.
- Seawell School Road is a collector roadway that provides access to residential neighborhoods and multiple schools along the facility. It is primarily a two-lane facility, with some sections having a continuous center left-turn lane. Some sections of sidewalk are also present. The posted speed limit is 35 mph on Seawell School Road.
- **Airport Drive** is a two-lane local access roadway for UNC properties and residential neighborhoods just south of the Estes Drive Extension and west of NC 86.

Intersections

Table 2, below, summarizes all five existing study area intersections, traffic control features, and pedestrian amenities at each. Laneage details and intersection turn bay lengths are also detailed on **Figure 3**.

Traffic Ped Signal Signal Cross Intersection **Phases** Operation Control walk Signals Estes Drive Extension and Free-Run No No Signal Seawell School Road Estes Drive Extension and **TWSC** N/A N/A No No UNC Student RR Lot **TWSC** Estes Drive Extension and Airport Drive N/A N/A No No Estes Drive Extension / Estes Drive and 8 Coordinated Yes (1) Signal Yes (1) NC 86 (Martin Luther King, Jr. Boulevard) NC 86 (Martin Luther King, Jr. Boulevard) and **TWSC** N/A N/A No No Airport Drive

Table 2. Existing Study Area Intersection Details

TWSC = Two-Way Stop Controlled Intersection

The project study area along the Estes Drive Extension features a mixture of signalized and unsignalized intersections. The N.C. 86 (Martin Luther King, Jr. Boulevard) corridor features coordinated signal operation for weekday peak hours.

Bicycle Routes and Sidewalks

Specific bicycle facilities are present in the immediate study area, with sharrow markings in both directions along Martin Luther King, Jr. Boulevard south of Estes Drive Extension. No other bicycle facilities are currently present. Pedestrian sidewalk is found along both sides of Martin Luther King Jr. Boulevard through the study area. Additional connectivity exists along the Estes Drive corridor from Martin Luther King, Jr. Boulevard eastward on the south side of the road. Sidewalk is also present on one side of Seawell School Road north of the Estes Drive Extension. A crosswalk and pedestrian signal is present across Estes Drive at the Martin Luther King, Jr. Boulevard intersection. **Figure 4** displays a schematic of existing pedestrian and bicycle facilities in the project study area.

Transit Routes

Current Chapel Hill Transit (CHT) Routes A, G, HS, NU, NS and T serve the project study area along NC 86 (Martin Luther King, Jr. Boulevard) and the Estes Drive Extension with weekday bus service (T Route also provides Saturday service). Several bus stops, with a range of amenities (shelters, benches), are present in the study area. **Table 3** details the six current CHT routes serving the study area. Most buses run on 10, 20, 30, or hour headways during weekday peak service periods. Only the HS and NU Routes currently provide service directly to the proposed site along the Estes Drive Extension.

GoTriangle provides regional bus service to the immediate study area via the 420 Route that runs along NC 86 between Chapel Hill and Hillsborough. Service for this route occurs at 30 minute headways during peak weekday periods. GoTriangle also provides express bus service from Chapel Hill to Raleigh on the CRX Route that operates along NC 86 (Martin Luther King, Jr. Boulevard) in the study area on 30 minute headways during weekday peak hours.

Table 3. Current Study Area Weekday Transit Service

	Headways (minutes) AM PM Off Peak Peak Peak		nutes)							
Route				Study Area Stops	Destinations					
Chapel Hill Transit										
А	10-20	30	30	NC 86 Corridor	Downtown Chapel Hill UNC Campus/Hospitals Area					
G	35	50	45-50	NC 86 CorridorEstes Drive	Downtown Chapel Hill Glen Lennox/University Place					
HS	30	30**	30	Estes Drive ExtensionSeawell School Road	Rogers Road AreaHomestead RoadNC 86 Corridor					
NS	10	20	10	NC 86 Corridor	UNC Campus/Hospitals AreaEubanks Park and RideSouthern Village Park and Ride					
NU	20	20	20	 UNC RR Lot Airport Drive	UNC Main Campus UNC Hospitals					
Т	25-30	35	35	NC 86 Corridor	Tymberline Shopping CenterDowntown Chapel HillE. Chapel Hill HS/Cedar Falls Pk					

Source: CHT 2017 Fall Ride Guide





Table 3 (Continued). Current Study Area Weekday Transit Service

	Headways (minutes)		nutes)				
Route	AM PM Off Peak Peak Peak			Study Area Stops	Destinations		
GoTria	ngle						
420	30	30 N/A		NC 86 Corridor	Hillsborough Downtown Chapel Hill/UNC Campus		
CRX	CRX 15-35 15- 35 N/A		None (Express Service)	Downtown Chapel Hill/UNC CampusEubanks Park-and-RideRaleigh			

Source: http://www.gotriangle.org/maps-and-schedules

Figure 5 displays transit routes and bus stops that currently exist in the project study area. The potential for transit trips are accounted for in the Town of Chapel Hill Municipal Services Campus site in the following sections of this report, as the proximity and frequency of transit service directly near proposed site may account for a measurable portion of site trips.

Recommended/Committed Surface Transportation Improvement Projects

There is one committed/programmed NCDOT State Transportation Improvement Program (STIP) project – STIP EB-5886, which will construct a multi-use path, additional sidewalks and bicycle lanes along the Estes Drive Extension from N Greensboro Street to NC 86 (Martin Luther King, Jr. Blvd). Right-of-way acquisition is scheduled for 2020 and construction for 2021. This improvement was considered to be complete for the 2021 analysis year.

The Town of Chapel Hill, in cooperation with NCDOT, also has a transportation improvement project to construct sidewalk and bicycle lanes along Estes Drive east of NC 86 (Martin Luther King Jr. Blvd), which will also include laneage improvements to the Estes Drive/NC 86 intersection. This project was considered to be complete for the 2021 analysis year. **Figure 6** shows a schematic representation of the improvements from the January 2016 Public Hearing Map.

There are no private development-related projects to improve roadway facilities in the study area that are expected to be complete by 2021. Several development projects near the study area have recommended, as part of their traffic impact study reports, reoptimization of traffic signals along the NC 86 (Martin Luther King, Jr. Blvd) corridor. This was considered to be included as part of the Town project improvements at this intersection.

There are numerous additional recommended improvements to transportation facilities in Town of Chapel Hill Municipal Services Campus project study area that may occur as the Carolina North development progresses just to the north of the project study area. However, any additional improvements due to Carolina North were considered post-2021 Phase 1 analysis year for the purposes of this study.

F. Existing Traffic Conditions

Figure 7 shows the existing AM, noon, and PM peak hour traffic volumes for the study area intersections. The counts used to determine these volumes were conducted in February 2018 for all study area intersections during the weekday periods 7:00 - 9:00 AM, 11:30 AM – 1:30 PM, and 4:00 – 6:00 PM. This data, along with all turning movement count output is found in **Appendix B**. **Table 4** provides a detailed listing of each intersection count, peak hour, and count date.



4:55 - 5:55 PM

7:40 - 8:40 AM

12:05 - 1:05 PM

4:35 - 5:35 PM

7:40 - 8:40 AM

12:05 - 1:05 PM

4:35 - 5:35 PM

7:30 - 8:30 AM

12:15 - 1:15 PM

4:55 - 5:55 PM

7:35 - 8:35 AM

11:45 - 12:45 PM

4:45 - 5:45 PM

Date of

Count

11/14/17

11/14/17

11/14/17

11/14/17

11/14/17

Traffic Count Location

Seawell School Road

UNC Student RR Lot

and Airport Drive

Estes Drive Extension and

Estes Drive Extension and Airport Drive

Estes Drive Extension / Estes Drive and

NC 86 (Martin Luther King, Jr. Boulevard)

NC 86 (Martin Luther King, Jr. Boulevard)

Town of Chapel Hill Municipal Services Campus - Proposed Institutional Development

Period **Peak Hour** Counted **AM Peak** 7:40 - 8:40 AM Estes Drive Extension and Noon Peak 12:05 - 1:05 PM

PM Peak

AM Peak

Noon Peak

PM Peak

AM Peak

Noon Peak

PM Peak

AM Peak

Noon Peak

PM Peak

AM Peak

Noon Peak

PM Peak

Table 4. Traffic Count Information

Traffic count information shows traffic flows on N.C. 86 (Martin Luther King, Jr. Boulevard) were heavy during the AM and PM peak count periods, with southbound flows into downtown Chapel Hill heaviest in the AM peak and northbound return flows heaviest in the PM peak. Traffic on Estes Drive and the Estes Drive Extension was moderate to heavy during the peak commuting periods. Traffic flows were light to moderate on the remaining study area roadways that function as collector or local access streets.

Turning movement counts were also collected on November 14, 2017 in the field for the two existing driveways serving the Chapel Hill Police Station along NC 86 (Martin Luther King, Jr. Blvd) just south of the project study area. Turning movements entering and exiting the site were compiled for the peak hours corresponding to highest adjacent street traffic for use in trip generation estimation in Section II. C of this report. Raw count data for the driveways is also found in Appendix B.

II. FUTURE BUILD-OUT YEAR CONDITIONS

A. Future Ambient Area-Wide Traffic Growth Estimation

Based on information on average daily traffic collected by the Town of Chapel Hill and the NCDOT, a yearly ambient traffic growth rate of 1.0 percent per year was used for the short-term 2021 design year capacity analyses. This rate is based on previous and anticipated growth trends for this area from Town and NCDOT average daily traffic information from the period 2003-2015, and is generally consistent with recent traffic impact studies near the project study area. Figure 8 shows ambient area-wide growth traffic volume projections.

B. Approved Background Development Traffic Estimation

Per information from Town of Chapel Hill staff and the Town's Development Activity Report, five potential future developments that are either currently in the concept plan phase, approved, under construction. or are expected to be built out and fully operational by the 2021 design analysis year were studied for the

February 2018 6



inclusion of specific background traffic for this report. The five developments are listed in **Table 5**, along with their current status and impact to 2021 traffic volumes.

Development Name	Fall 2017 Status	Development Density	TIS Status	2021 Traffic Impact
Chapel Hill Retirement Residences	Approved, Not Constructed	100 Units	No - Exempt	Assume 100% built out – specific generator
Sawmill Condominiums	Concept Plan Phase	150 Units	No	Assume 100% built out – specific generator
North Estes Mixed-Use Center	Concept Plan Phase	100k SF Office 100k SF Retail	Yes – RS&H (2012) – Older Concept Plan	Assume Build-Out Post 2021
Women's Health & Wellness Center	Approved, Not Constructed	25k SF Redevelopment	No	Assume Included in Ambient Growth Projections
Carolina North	Approved, No Major Anticipated Near-Term	Phase 1	Yes – VHB (2009)	Assume Any Impact Between 2017 and 2021 Included in Ambient Growth Projections

Table 5. Study Area Background Development Status

Figure 9 shows the relative location of the background developments listed in **Table 5**. Total estimated background traffic volumes for the two background generators assumed to be complete by the 2021 analysis year in the project study area are shown in **Figure 10**. **Figure 11** shows the total background peak hour traffic volumes estimated for the 2021 Without Site analysis scenario. These volumes include the projected ambient area-wide traffic growth and the specific background generator estimates displayed in **Figure 10**.

Appendix C displays individual background traffic generator peak hour volumes estimates projected across the project study area. Traffic assignment from the two specific generator developments was estimated based on development density information and/or trip generation data provided for the Chapel Hill Retirement Residences and the Sawmill Condominiums projects. Background traffic assignment in the project study area was determined by using current turning movement peak hour volumes and engineering judgment.

C. Proposed Project Traffic

i. Trip Generation

Projected trips for the proposed government services facility expansion were generated based on the *ITE Trip Generation Manual* (Institute of Transportation Engineers, 9th Edition, 2013) and supplemented with field traffic count data for the existing Chapel Hill Police Station on NC 86 (Martin Luther King, Jr Blvd). Trip generation methodologies for estimated trips utilize the number of dwelling units and average rate methodology (per NCDOT recommendations) as trip-generating variables. **Table 6** shows the number of vehicular trips generated by existing Town of Chapel Hill Municipal Services Campus during the weekday AM, noon, and PM peak hours of adjacent streets, based on the generation methodologies described above. A peak hour truck percentage of two percent was estimated for all site-generated traffic.





Table 6
Phase 1 Weekday Peak Hour Vehicle Trip Generation Summary

ITE	December	Donoity		Daily		Α	M Pea	k	N	oon Pea	ak	F	PM Peak	(
LUC	Description	Density	Enter	Exit	Total	Enter	Exit	Total	Enter	Exit	Total	Enter	Exit	Total
710	General Office Building	48,467 SF	379	379	758	94	13	107	59**	62**	121**	23	110	133
N/A	Police Station – Field Data Adjusted	24,346 SF	146	146	292	32	3	35	28	22	50	15	40	55
Tot	tal Phase 1	72,813 SF	525	525	1,050	126	16	142	87	84	171	38	150	188

^{** -} No ITE Data Available for Noon Peak Hour, Data is Average of AM and PM Results

ii.) Adjustments to Trip Generation Rates

Raw ITE trip generation estimates for daily and peak hour trips are typically adjusted for the following factors to reduce raw trip generation estimates to actual estimated vehicular trips produced by the Town of Chapel Hill Municipal Services Campus development.

a.) Internal Capture

The land uses proposed for Town of Chapel Hill Municipal Services Campus development would not exhibit the potential for internally captured trips for any on-site uses. No additional modifications or reductions were made to trip generation results to account for internal capture.

b.) Modal Split

The study area is well served by several CHT and Triangle Transit fixed bus routes with frequent existing service with connectivity to trip attractions in downtown Chapel Hill and the UNC Main Campus, although most routes are not currently easily accessible from the site parcel. Likewise, pedestrian and bicycle facilities exist in the study area but lack connectivity to the site parcel itself currently. To be conservative, no quantitative reductions in vehicular trips was made using these modes. However, it is recognized that some peak period trip-making will occur with the availability and potential future connectivity provided for non-motorized transportation.

c.) Pass-by Trips

No pass-by trips were accounted for in this study, since the proposed Town of Chapel Hill Municipal Services Campus is not a typical pass-by trip generator.

d.) Trip Generation Budget

Current plans for Town of Chapel Hill Municipal Services Campus are for the project to be built in multiple phases, but only Phase 1 is included in this analysis. Additional phases are not explicitly defined in terms of development densities or schedules, so this analysis will need to be updated when plans for those future phases are complete.

iii.) Trip Distribution

Trip distribution for site-related traffic was based existing daily and peak hour traffic patterns to determine the directional peak hour characteristics of traffic to and from the site from the major study area thoroughfares. No local trips to/from lower volume collector and residential streets were estimated, though the possibility exists a small portion of trip-making may occur to/from these local streets. Basic distribution estimates for site traffic flow utilized existing peak hour turning movement counts and overall comparison to local and regional trip attractors. Distribution estimates for the two



site driveways were based on assumptions of utilization of the closest driveway and adjacent proposed parking facilities to the Phase 1 site building. **Figure 12** presents the projected trip distribution traffic percentages for the proposed site in 2021.

iv.) Trip Assignment

Figure 13 shows the corresponding Phase 1 site traffic volumes distributed on the 2021 study area network. Total volumes into and out of the site correspond to total external vehicular trips generated, based on the trip generation methodology developed previously.

D. Future Traffic Forecasts with the Proposed Development

Figure 14 displays the 2021 Build-out+1 year projected study area traffic volumes with site traffic added. These traffic volumes represent the aggregate traffic growth over existing traffic volumes for a) ambient traffic growth, b) specific background development traffic assignments from those developments, and c) estimated site traffic assignment for Phase 1 of the Town of Chapel Hill Municipal Services Campus. **Appendix D** contains all the peak hour scenario volume development spreadsheets used in the estimation of 2021 traffic volumes for both the with and without site scenarios.

III. IMPACT ANALYSES

A. Peak Hour Intersection Level of Service Analysis

i.) Methodology

Evaluation of traffic operations on suburban arterial, collector, and local roadway facilities is most effective through the determination of level of service (LOS) criteria. The concept of level of service correlates qualitative aspects of traffic flow to quantitative terms. This enables transportation professionals to take the qualitative issues, such as congestion and substandard geometrics, and translate them into measurable quantities, such as operating speeds and vehicular delays. The 2010 *Highway Capacity Manual (HCM 2010)* characterizes level of service by letter designations A through F. Level of service A represents ideal low-volume traffic operations, and level of service F represents over-saturated high-volume traffic operations. Level of service is measured differently for various roadway facilities, but in general, level of service letter designations are described in **Table 7**.

The minimum acceptable peak hour intersection level of service established for this project is LOS D for signalized intersections or LOS E for critical movements at unsignalized intersections, or no increase in delay for signalized intersections operating below LOS D or unsignalized intersection critical movements operating below LOS E without the inclusion of site traffic. The following four conditions were evaluated:

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Condition 1 - Existing Traffic

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Condition 2 - 2021 Traffic without Site Traffic

Condition 3 - 2021 Traffic with Phase 1 Site Traffic Volumes Added

Condition 4 - 2021 Traffic with Phase 1 Site Traffic and Improvements

HNTB

Table 7. Level of Service (LOS) Characteristics

Le	vel of Service Description	Per Vehicle Delay at Signal	Per Vehicle Delay at Stop Sign
LO	SA		
>	Free flow	4400000	4400000
>	Freedom to select desired speed and to maneuver is extremely high	< 10.0 sec	< 10.0 sec
>	General level of comfort and convenience for motorists is excellent		
LO	SB		
>	Stable flow	10.0 - 20.0	10.0 – 15.0
>	Other vehicles in the traffic stream become noticeable	sec	sec
\triangleright	Reduction in freedom to maneuver from LOS A		
LO	SC		
\triangleright	Stable flow	20.0 – 35.0	15.0 – 25.0
\triangleright	Maneuverability and operating speed are significantly affected by	20.0 - 33.0 sec	15.0 – 25.0 SeC
	other vehicles	360	360
\triangleright	General level of comfort and convenience declines noticeably		
LO	S D		
\triangleright	High density but stable flow	35.0 - 55.0	25.0 – 35.0
>	Speed/freedom to maneuver are very restricted	sec	sec
\triangleright	General level of comfort / convenience is poor	300	300
>	Small increases in traffic will generally cause operational problems		
_	SE		
	Unstable flow		
	Speed reduced to lower but relatively uniform value	55.0 - 80.0	35.0 - 50.0
>	Volumes at or near capacity level	sec	sec
>	Comfort and convenience are extremely poor		000
>	Small flow increases or minor traffic stream disturbances will cause		
	breakdowns		
	<u>S</u> F		
>	Forced or breakdown flow		
>	Volumes exceed roadway capacity	> 80.0 sec	> 50.0 sec
>	Formation of unstable queues		
\triangleright	Stoppages for long periods of time because of traffic congestion		

The *Synchro Professional Version 9* operations analysis software was used to analyze peak hour conditions at signalized intersections. Synchro was also used to analyze peak hour conditions at unsignalized intersections, through the use of its HCM 2010 two-way stop controlled output function. The methodology of evaluating each condition for signalized intersections is presented below:

- Condition 1 Use current Town of Chapel Hill data for the cycle length, splits and offsets of individual signalized intersections and report LOS and delay values from Synchro.
- Conditions 2 and 3 Reoptimize the cycle lengths and splits of individual intersections in Synchro, if existing timing data does not provide adequate overall intersection LOS. Adjust cycle lengths, splits, and offsets, if necessary, if the signal is currently operating in a coordinated system. The optimized signal timing information will be held constant for both Conditions, to provide a means to compare effects of the proposed site traffic. No changes to free run traffic signal inputs were made for Conditions 2 and 3.





 Condition 4 – Optimize coordinated traffic signals for effects of recommended mitigation strategies that change existing/committed changes to lane geometrics. Evaluate the potential for different signal phasing schemes (left-turn lag phases, for example). Retain existing split minimums and any pedestrian timing values. Recommendations, if warranted, will be made to obtain at least LOS D for the intersection as a whole.

The net effect of this process is that direct comparisons, by movement, of delay and LOS between each of the three conditions are impossible because splits and cycle lengths can and do change between conditions. The pertinent statistic of this analysis is the *overall intersection level of service* and delay. Improvements to deficient intersections in Condition 3 were made by first attempting to adjust signal operations via changes in cycle lengths, splits and/or with acceptable adjustments to signal phasing. If that did not produce satisfactory results for all intersections, geometric improvements to improve intersection capacity were considered for the deficient intersections. **Appendix E** contains the Synchro signalized intersection output for all four conditions (where applicable).

Unsignalized intersections were analyzed using HCM methodologies. Their results were evaluated on a per-movement basis, since HCM methods do not produce an overall intersection level of service for unsignalized intersections. Thus, intersections with deficient (LOS F) movements in Condition 2 would need to be evaluated for improvements in Condition 3. This methodology differs from signalized intersections, where one or more movements at an intersection may be deficient in Condition 2, but as long as the overall intersection level of service does not fall below LOS D, no intersection improvements may be deemed necessary. *Appendix F* contains the Synchro 2010 HCM unsignalized output for all stop-controlled intersections under study.

ii.) 2017 Existing Conditions Results

Table 8 presents the results for the existing year traffic conditions as compiled from field data. The table lists LOS and delay values for those movements that are in existence at this time. Currently, all study area signalized intersections operate at an overall acceptable level of service for all of the analyzed 2017 peak hours, with the exception of NC 86 and Estes Drive, which operates at a LOS E in the 2017 PM peak hour. The stop-controlled approaches along Airport Drive with its intersections with Estes Drive Extension and NC 86 also operate at a deficient LOS F in at least one peak hour in 2017.

iii.) 2021 No-Build Scenario (Condition 2) Results

Table 9 presents the results for the 2021 analysis year estimated traffic conditions without the impacts of site-related traffic. This analysis includes ambient growth, and data for the future background site developments.

During Condition 2 - 2021 Without Site Traffic, delays marginally increase for most study area intersections/critical movements, with the same deficient LOS F stop-controlled approaches anticipated for Airport Drive intersections with Estes Drive Extension and NC 86. There were specific geometric improvements to the NC 86 and Estes Drive intersection area that are committed by the Town/NCDOT project to improve the Estes Drive corridor to the east. For traffic capacity analysis inputs, these are primarily auxiliary lane improvements to the intersection along with signal phasing and timing upgrades. These upgrades provide minor beneficial effects in terms of LOS and delay improvements, though they do mitigate the effect of additional projected background traffic to some extent. The NC 86 and Estes Drive intersection still is expected to operate at an overall LOS E in the 2021 PM peak hour in this scenario.



Table 8. Capacity Analysis Results for Study Area Intersections
Condition 1 – 2017 Existing Traffic

Intersections/Movements		LOS			e Vehicula onds/veh	
	AM	Noon	PM	AM	Noon	PM
Estes Drive Extension and Seawell School Road	В	Α	В	15.0	9.9	17.5
SB LT SB RT EB LT EB THRU WB THRU WB RT	C C A B B A	B B A B A	C D A A C A	23.5 25.3 9.3 11.6 19.5 3.9	18,7 19,6 4.5 5.4 12.9 3.3	30.3 37.4 8.3 6.4 23.6 3.1
Estes Drive Extension and UNC Student RR Lot	N/A	N/A	N/A	N/A	N/A	N/A
EB LT SB LT SB RT	A C A	A C A	A D B	8.1 22.8 0.0	8.2 16.7 0.0	9.4 29.9 14.3
Estes Drive Extension and Airport Drive	N/A	N/A	N/A	N/A	N/A	N/A
WB LT NB LT-RT	ВС	A B	A F	10.3 24.8	8.2 14.9	8.5 50.7
Estes Drive Extension / Estes Drive and NC 86 (Martin Luther King, Jr. Boulevard)	D	D	E	44.1	39.7	61.8
EB LT EB THRU EB RT WB LT WB THRU WB RT NB LT NB THRU-RT SB LT SB THRU-RT	0 E 0 0 E 0 0 0 0	E E D D F D C C C B	F E D F F C D F C	51.3 78.0 36.3 47.4 55.5 20.2 40.7 42.7 40.1 37.4	59.3 79.4 47.3 52.5 91.9 43.1 23.2 26.5 25.2 18.7	109.2 57.2 38.2 42.5 126.3 96.4 32.1 45.4 98.7 31.1
NC 86 (Martin Luther King, Jr. Boulevard) and Airport Drive	N/A	N/A	N/A	N/A	N/A	N/A
EB LT EB RT NB LT	F C B	D B A	F В А	58.1 16.6 13.2	28.6 11.2 9.5	67.4 11.8 9.9

N/A => Not Applicable, i.e. movement is non-existent or overall intersection values are not reported for unsignalized intersections **BOLD/ITALICS** – Movement or overall intersection is over Town TIS Guidelines threshold capacity



Table 9. Capacity Analysis Results for Study Area Intersections Condition 2 - 2021 Traffic Without Site

Intersections/Movements		LOS			e Vehicula onds/veh	
	AM	Noon	PM	AM	Noon	РМ
Estes Drive Extension and Seawell School Road	В	В	В	15.5	10.1	18.6
SB LT SB RT EB LT EB THRU WB THRU WB RT	C C A B B A	B C A B A	C D A A C A	24.6 26.4 9.8 12.1 19.6 3.8	19.3 20.3 4.5 5.4 12.9 3.2	31.7 39.5 9.2 6.5 24.3 3.0
Estes Drive Extension and UNC Student RR Lot	N/A	N/A	N/A	N/A	N/A	N/A
EB LT SB LT SB RT	A D A	A C A	A D B	8.1 32.2 0.0	8.3 17.5 0.0	9.5 32.6 14.7
Estes Drive Extension and Airport Drive	N/A	N/A	N/A	N/A	N/A	N/A
WB LT NB LT-RT	B D	A C	A F	10.5 27.0	8.3 15.5	8.6 65.3
Estes Drive Extension / Estes Drive and NC 86 (Martin Luther King, Jr. Boulevard)	D	D	E	42.7	40.8	59.9
EB LT EB THRU EB RT WB LT WB LT WB THRU WB RT NB LT NB THRU NB THRU NB RT SB LT SB THRU SB RT NC 86 (Martin Luther King, Jr. Boulevard)	E E C E F C C D B C D A	E E C E C D D B C C B	F E D F F D D E B F C B	71.7 64.9 22.3 70.4 80.1 30.5 23.2 40.8 15.7 27.5 37.8 5.4	78.5 55.7 33.3 77.1 76.7 25.8 38.5 40.2 15.9 33.6 25.7 10.7	107.6 56.9 38.4 92.2 106.1 54.3 44.5 65.1 17.6 90.5 33.6 12.9
EB LT EB RT NB LT	F С В	D B A	F В В	68.4 17.7 14.0	32.4 11.4 9.7	87.5 12.1 10.2

N/A => Not Applicable, i.e. movement is non-existent or overall intersection values are not reported for unsignalized intersections BOLD/ITALICS - Movement or overall intersection is over Town TIS Guidelines threshold capacity **BLUE** = New or Modified Movements Committed in No Build Scenario

Iv.) 2021 Build Scenario (Condition 3) Results

Table 10 presents results for 2021 Build-out+1 year estimated traffic conditions, including impacts of site-related traffic. In general, the addition of site-related traffic will marginally increase delays at existing intersections and is not expected to cause additional intersections or critical intersection stopcontrolled movements to drop to deficient levels in the 2021 analysis year.

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Table 10. Capacity Analysis Results for Study Area Intersections Condition 3 – 2021 Traffic With Site (Phase 1)

Intersections/Movements		LOS		Average Vehicular Delay (seconds/vehicle)			
	AM	Noon	PM	AM	Noon	РМ	
Estes Drive Extension and Seawell School Road	В	В	В	15.6	10.1	18.7	
SB LT SB RT EB LT EB THRU WB THRU WB RT	C C A B A	B C A A B	C D A A C A	25.1 26.6 9.7 12.2 19.5 3.8	19.5 20.4 4.5 5.4 13.0 3.2	32.3 40.1 9.4 6.5 24.3 3.1	
Estes Drive Extension and West Site Driveway / UNC Student RR Lot	N/A	N/A	N/A	N/A	N/A	N/A	
EB LT WB LT NB LT-THRU-RT SB LT SB THRU-RT	A B C E A	A A B C	A A C E B	8.1 10.2 23.6 46.2 0.0	8.3 8.4 13.8 21.8 0.0	9.5 8.6 20.2 47.0 14.8	
Estes Drive Extension and East Site Driveway	N/A	N/A	N/A	N/A	N/A	N/A	
WB LT NB LT NB RT	B E C	A C B	A E B	10.9 43.3 17.1	8.6 22.7 12.0	8.9 39.9 14.9	
Estes Drive Extension and Airport Drive	N/A	N/A	N/A	N/A	N/A	N/A	
WB LT NB LT-RT	B E	A C	A F	10.6 42.6	8.5 20.2	9.1 134.3	
Estes Drive Extension / Estes Drive and NC 86 (Martin Luther King, Jr. Boulevard)	D	D	E	43.1	42.3	62.5	
EB LT EB THRU EB RT WB LT WB THRU WB RT NB LT NB THRU NB RT SB LT SB THRU SB RT NC 86 (Martin Luther King, Jr. Boulevard) and Airport Drive	E E C E F C C D B C D A	E D C E E C D D B D C B	F E D F F D E B F C B	71.7 64.8 21.9 68.9 84.4 29.6 27.4 41.5 15.8 28.2 39.2 5.8	78.9 53.3 32.5 77.1 76.2 24.6 41.4 42.6 17.1 37.6 29.1 12.2 N/A	130.5 57.8 39.8 92.2 107.8 53.4 46.8 66.8 17.8 90.9 34.0 13.2 N/A	
EB LT EB RT NB LT	F C B	E B A	F В В	81.9 17.9 14.8	37.6 11.8 10.0	106.1 12.9 10.5	

N/A => Not Applicable, i.e. movement is non-existent or overall intersection values are not reported for unsignalized intersections **BOLD/ITALICS** – Movement or overall intersection is over Town TIS Guidelines threshold capacity **BLUE** = New or Modified Movements Committed in Build Scenario





v.) 2021 Build + Mitigation Scenario (Condition 4) Results

Table 11 presents results for 2021 Build-out+1 year estimated traffic conditions, including impacts of site-related traffic and safety-related recommended mitigation improvements along the Estes Drive Extension. These improvements include the development of a three-lane undivided cross-section from the existing UNC RR Lot intersection to NC 86 (Martin Luther King, Jr Blvd), which would provide a continuous two-way left-turn lane at each existing or proposed intersection. In addition, an additional eastbound left-turn lane was tested at the NC 86 intersection with Estes Drive/Estes Drive Extension. The Town has committed to including this improvement in the Estes Drive pedestrian and bicycle improvements project.

Table 11. Capacity Analysis Results for Study Area Intersections Condition 4 – 2021 Traffic With Site & Mitigation

Intersections/Movements	LOS			Average Vehicular Delay (seconds/vehicle)		
		Noon	PM	AM	Noon	PM
Estes Drive Extension and West Site Driveway / UNC Student RR Lot	N/A	N/A	N/A	N/A	N/A	N/A
EB LT WB LT NB LT-THRU-RT SB LT SB THRU-RT	A B C E A	A A B C	A A C E B	8.1 10.2 23.6 45.7 0.0	8.3 8.4 13.8 21.7 0.0	9.5 8.6 20.1 46.5 14.8
Estes Drive Extension and East Site Driveway	N/A	N/A	N/A	N/A	N/A	N/A
WB LT NB LT NB RT	B C C	A C B	A C B	10.9 20.7 17.1	8.6 15.5 12.0	8.9 20.1 14.9
Estes Drive Extension and Airport Drive	N/A	N/A	N/A	N/A	N/A	N/A
WB LT NB LT-RT	B C	A C	A D	10.6 22.6	8.5 15.1	9.1 30.9
Estes Drive Extension / Estes Drive and NC 86 (Martin Luther King, Jr. Boulevard)	D	D	D	40.1	39.7	54.0
EB LT EB THRU EB RT WB LT WB THRU WB RT NB LT NB THRU NB RT SB LT SB THRU SB RT	E E C E E C C D B C D A	E E D E E C C C B C C B	F E D F F D D E B E C B	56.6 68.0 23.3 72.8 72.6 28.1 24.7 38.3 14.9 25.7 36.2 6.9	74.2 66.6 39.6 75.3 75.9 30.8 30.7 31.9 11.8 31.4 23.6 12.4	98.0 68.8 45.9 92.2 93.3 45.5 40.4 55.5 15.2 73.5 28.3 14.9

N/A => Not Applicable, i.e. movement is non-existent or overall intersection values are not reported for unsignalized intersections **BLUE** = New or Modified Movements Analyzed in Mitigation Scenario

B. Access Analysis

Vehicular site access is to be accommodated at two proposed full movement access driveways connecting to Estes Drive Extension. The western site driveway is about 800 feet to the east of the Estes



Town of Chapel Hill: Traffic Impact Study



Town of Chapel Hill Municipal Services Campus - Proposed Institutional Development

Drive Extension signalized intersection with Seawell School Road. As conceptually shown in **Figure 2**, the driveways have single inbound lanes. The western driveway is assumed to have one outbound lane, with the eastern (main) driveway assumed to have two outbound lanes. The driveway connections to the Estes Drive Extension have throat lengths of approximately 150 feet (eastern) and 500 feet (western) prior to internal parking lot connections. Throat lengths are acceptable, based on 50 foot minimum throat length standards found on Page 69 of the 2017 *Town of Chapel Hill Public Works Design Manual*.

Driveway distances along Estes Drive Extension from the signalized intersection at Seawell School Road is approximately 800 feet as noted above, and is acceptable, based on recommendations of 100 foot minimum corner clearance as set forth in the 2003 *NCDOT Policy on Street and Driveway Access to North Carolina Highways* and the 100 foot minimum along collector streets specified in the Town Design Manual. The distance between the proposed driveway connections is approximately 600 feet and would also be acceptable, based on the recommended 50 foot spacing between driveways along collector roadways found in Table 3.2 – Street Standards in the Town Design Manual.

Access for pedestrians and bicycles is not adequate in the project study area. Sidewalk is present along the NC 86 corridor and along sections of Seawell School Road and Estes Drive to the east of NC 86. Crosswalk exists across the Estes Drive intersection with the NC 86 signalized intersection in only one quadrant. No specific bicycle amenities are present along the Estes Drive Extension, but bicycle "sharrow" lanes are present along NC 86 south of the Estes Drive Extension. With the completion of the pedestrian and bicycle projects along Estes Drive and the Estes Drive Extension, pedestrian and bicycle access to the site will improve significantly, as continuous facilities for both modes will be constructed along over two miles of the Estes Drive/Estes Drive Extension corridor.

C. Signal Warrant Analysis

Based on projected 2021 traffic volumes and proposed access plans, the unsignalized Site Driveway intersections with the Estes Drive Extension would not warrant the installation of a traffic signal, based on the methodology found in the 2009 Manual on Uniform Traffic Control Devices (MUTCD).

The stop-controlled approaches at the intersections of Estes Drive Extension and Airport Drive and NC 86 and Airport Drive are expected to operate at a LOS F in the 2021 analysis year with site traffic added. 2021 peak hour volumes and existing geometrics at these locations were analyzed for meeting the Peak Hour Warrant in the 2009 MUTCD and HCS Warrants software package. The results shown in *Appendix G* indicate that the Estes Drive and Airport Drive intersection would warrant the installation of a traffic signal, but the NC 86 and Airport Drive intersection would not, based on only the Peak Hour Warrant. Satisfaction of additional warrants would be needed to justify the implementation of a traffic signal at these locations.

D. Sight Distance Analysis

In general, sight distance issues entering/exiting the existing Town of Chapel Hill Municipal Services Campus driveways would be minimal, considering the fact that Estes Drive Extension has no horizontal curvature in the vicinity of this existing access location and vertical curvature at this location is minimal, giving entering and exiting traffic adequate sight distance in both directions.

E. Crash Analysis

Data from the NCDOT Traffic Safety Unit TEAAS crash software database was extracted for the five year period from 11/1/2012 to 10/31/2017 for the segment of the Estes Drive Extension from Seawell School Road to NC 86 (Martin Luther King, Jr. Boulevard). Raw crash data can be found in **Appendix H**.



Estes Drive Extension Corridor

There were 25 crashes reported along the Estes Drive Extension study area corridor between Seawell School Road and NC 86 over the five year period. In this 0.78 mile segment, crash types varied with no predominate crash type. Several left-turn, run-off road and rear end crash types were noted. Spatial distribution of crashes along the corridor from the segment strip map indicates that a considerable number of crashes (16) occurred in the vicinity of the NC 86 (Martin Luther King, Jr. Blvd) intersection. The remaining crashes were distributed near other intersections along the rest of the segment, with four crashes occurring in the vicinity of the Airport Drive intersection and three crashes near Seawell School Road. One fatality crash (motorcyclist) occurred at the intersection with Seawell School Road.

Table 12 presents a comparison between the Estes Drive Extension study area crash rates and the latest North Carolina statewide rates for the period 2013-2015 (compiled by NCDOT Traffic Safety Unit). Overall, the crash rates along Estes Drive Extension in the project study area are lower than statewide averages for similar facilities (two-lane undivided) in every reported category, except for fatal crashes.

Table 12. Study Area Crash Rate Comparison – Estes Drive Extension Corridor

	Crashes Per 100 Million Vehicle Miles				
Statistic	Estes Drive Extension	NC Statewide Average*			
	Seawell School Rd to NC 86 (MLK Jr. Blvd)	2-Lane Undivided			
Total Crash Rate	141.55	247.39			
Fatal Crash Rate	5.66	1.18			
Non-Fatal (Injury) Crash Rate	33.97	76.16			
Night Crash Rate	50.96	65.51			
Wet Crash Rate	45.30	46.04			

^{* -} Data for Urban Secondary Routes

F. Other Transportation-Related Analyses

Other transportation-related analyses relevant to the 2001 Town of Chapel Hill Guidelines for the preparation of Traffic Impact Studies were completed as appropriate. The following topics listed in **Table 13** are germane to the scope of this study.

G. Special Analysis/Issues Related to Project

Based on discussions with Town of Chapel Hill staff, there are no special issues or analyses beyond the ones already discussed for this proposed site.

Table 13. Other Transportation-Related Analyses

Analysis	Comment
Turn Lane Storage Requirements	Storage bay lengths at study area intersections were analyzed using Synchro and HCS 95 th percentile (max) queue length estimates for the 2021 Build Scenario. At the intersection of Estes Drive/Estes Drive Extension and NC 86 (Martin Luther King, Jr. Blvd), projected 95 th percentile queue lengths may exceed future delineated storage bay lengths as the intersection is still near/at capacity even with the assumed committed improvements to auxiliary turn lanes. Dual eastbound left-turn lanes, which would require 300 feet of dual left-turn lane storage are recommended to serve this movement and provide some improved overall intersection capacity which would benefit other movements that are projected to exceed turn bay storage lengths.
Appropriateness of Acceleration/ Deceleration Lanes	The site concept plan shows no specifics related to acceleration/deceleration lanes. It is recommended that a westbound left-turn lane be constructed along Estes Drive Extension at the proposed site driveway to remove turning traffic movements from the westbound through traffic flow along Estes Drive Extension. Site traffic volumes heading eastbound to the site are expected to be low and would not require a right-turn deceleration lane. No other specific acceleration/deceleration lane issues were analyzed in the project study area.
Pedestrian and Bicycle Analysis	Existing pedestrian access and connectivity currently lacking along the Estes Drive Extension corridor adjacent to the site. With the completion of Town and NCDOT projects along Estes Drive and the Estes Drive Extension, the proposed site will have fully connected, safe and efficient pedestrian and bicycle access along those corridors. Signalized crosswalks and pedestrian signals are recommended for the NCDOT STIP project at the Seawell School Road/Estes Drive Extension intersection.
Public Transportation Analysis	Public transportation service to the study area, and to the proposed site is available, with bus stops and multiple local and regional bus routes along NC 86 in both directions and two local routes serving the Estes Drive Extension proximate to the site. It is recommended that a bus stop be constructed along the site frontage near the East Site Driveway.

IV. MITIGATION MEASURES / RECOMMENDATIONS

A. Planned Improvements

The Town of Chapel Hill in coordination with NCDOT has a pedestrian and bicycle improvement project for Estes Drive east of NC 86 (Martin Luther King, Jr. Blvd) that includes auxiliary lane improvements and pedestrian crosswalk/signal heads at the Estes Drive/NC 86 intersection. This project is expected to be complete by the 2021 site build-out+1 analysis year. As a result of the mitigation analysis for this traffic impact study, the Town is including the provision of an additional eastbound left-turn lane at the NC 86 and Estes Drive intersection into the design of the project. See **Figure 15** for details.

NCDOT also has a programmed pedestrian and bicycle facility enhancement project (STIP EB-5886) that will construct sidewalks, multi-use paths and bicycle lanes along the Estes Drive Extension facility between N. Greensboro Street in Carrboro to NC 86. This project is scheduled for construction in 2021 and it was also assumed to be complete by the 2021 analysis year in this study.





B. Background Committed Improvements

There are no specific geometric improvements to the study area roadway intersections related to background private development projects that are expected to be completed between 2017 and 2021. Several traffic impact studies for development projects in and near the study area recommended signal timing reoptimization for signalized intersections along the NC 86 (Martin Luther King, Jr. Blvd) corridor by their respective build-out years. It is assumed that signal timing reoptimization will occur for the NC 86 intersection with Estes Drive/Estes Drive Extension, due to the geometric and signal phasing upgrades expected to occur as part of the Estes Drive Pedestrian/Bicycle project.

As previously discussed, additional recommended improvements to transportation facilities in Town of Chapel Hill Municipal Services Campus project study area may occur as the Carolina North development progresses. However, any additional improvements due to Carolina North were considered post-2021 Phase 1 analysis year for the purposes of this study. The Carolina North traffic impact study will need to be revised and updated as definitive phased construction occurs during the course of the project.

C. Applicant Committed Improvements

Based on the preliminary site plans and supporting development information provided, there are no specific transportation-related improvements proposed external to the Town of Chapel Hill Municipal Services Campus site. The two proposed site driveways and initial laneage assumptions are schematically shown in **Figure 15**, based on the preliminary concept plans shown in **Figure 2**.

D. Necessary Improvements - Due to Site Impact

Based on traffic capacity analyses for the 2021 design year, and analyses of existing study area turning bay storage lengths and site access, the following improvements are recommended as being necessary for adequate transportation network operations due specifically to site transportation impact (see improvements in **Figure 16** highlighted in green).

- 1) Widen Estes Drive Extension along the length of site frontage to provide a consistent three-lane cross-section with exclusive westbound left-turns lane into the site at the two proposed site driveway intersections. This improvement improves operations for stop-controlled movements at both intersections and improves overall safety by removing the left-turn movements from the through traffic streams along the Estes Drive Extension.
- 2) Provide a bus stop and amenities for transit riders along the frontage of the proposed Municipal Services Campus.

E. Necessary Improvements – Regardless of Site Development

Based on traffic capacity analyses for the 2021 design year, and analyses of existing study area turning bay storage lengths and site access, the following improvements are recommended as being necessary for adequate transportation network operations due existing traffic congestion issues or issues arising from background traffic growth whether or not the Town Municipal Services Campus is constructed (see improvements in **Figure 16** highlighted in blue).

1) Widen Estes Drive Extension between the proposed site frontage and the NC 86 (Martin Luther King Jr. Blvd.) intersection to a consistent three-lane cross-section. This provides operational and safety improvements for the Airport Drive intersection with the Estes Drive Extension and should be considered in the design of the NCDOT pedestrian and bicycle improvement project along the Estes Drive Extension corridor.

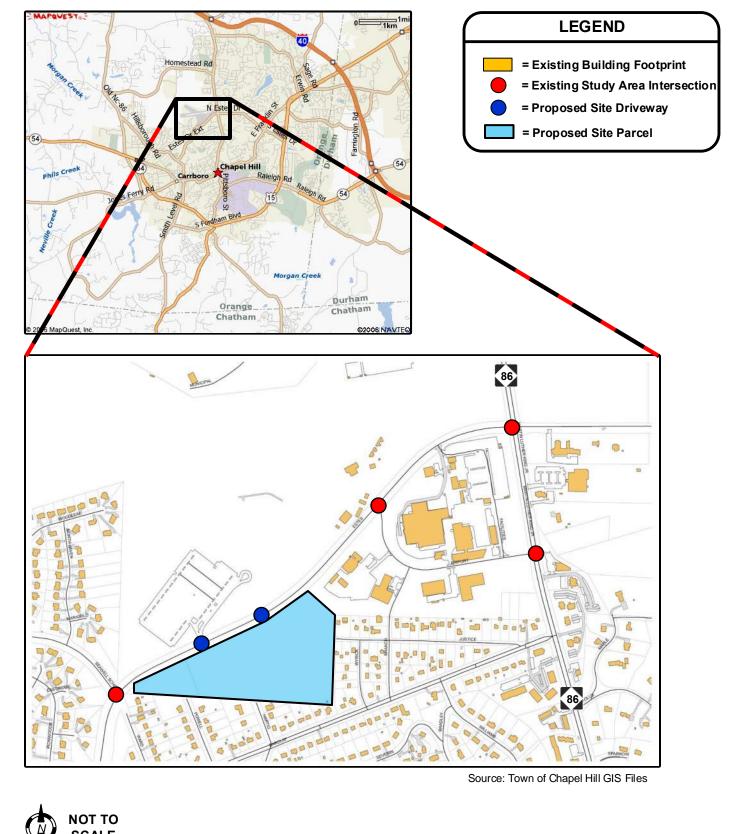




Though the Estes Drive Extension intersection with Airport Drive meets MUTCD Peak Hour Signal Warrants, it is not recommended that a signal be installed at this location if the proposed recommendations to widen Estes Drive Extension to a three-lane cross-section are constructed. No additional improvements are recommended at the Airport Drive intersection with NC 86, though 2021 peak hour capacity analyses indicate a LOS F for stop-controlled left-turn movements for eastbound Airport Drive at this location. Provision of additional improvements at this intersection, along with signalizing the Airport Drive/Estes Drive Extension intersection may encourage additional cut-through traffic along the Airport Drive facility.



Appendix A – Figures







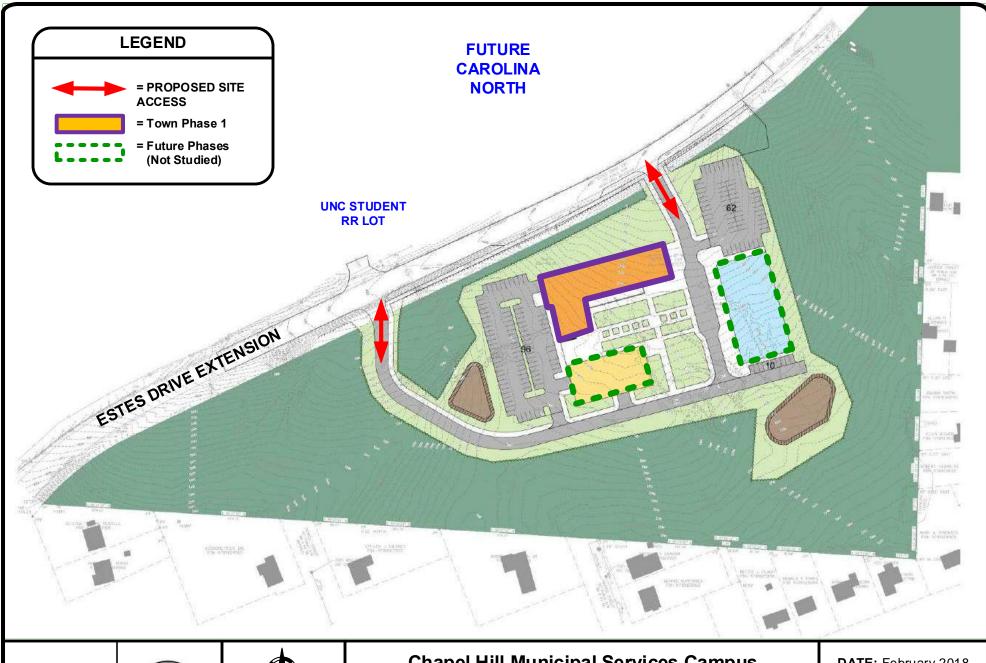


Chapel Hill Municipal Services Campus Traffic Impact Study

PROJECT STUDY AREA

DATE: February 2018

FIGURE 1









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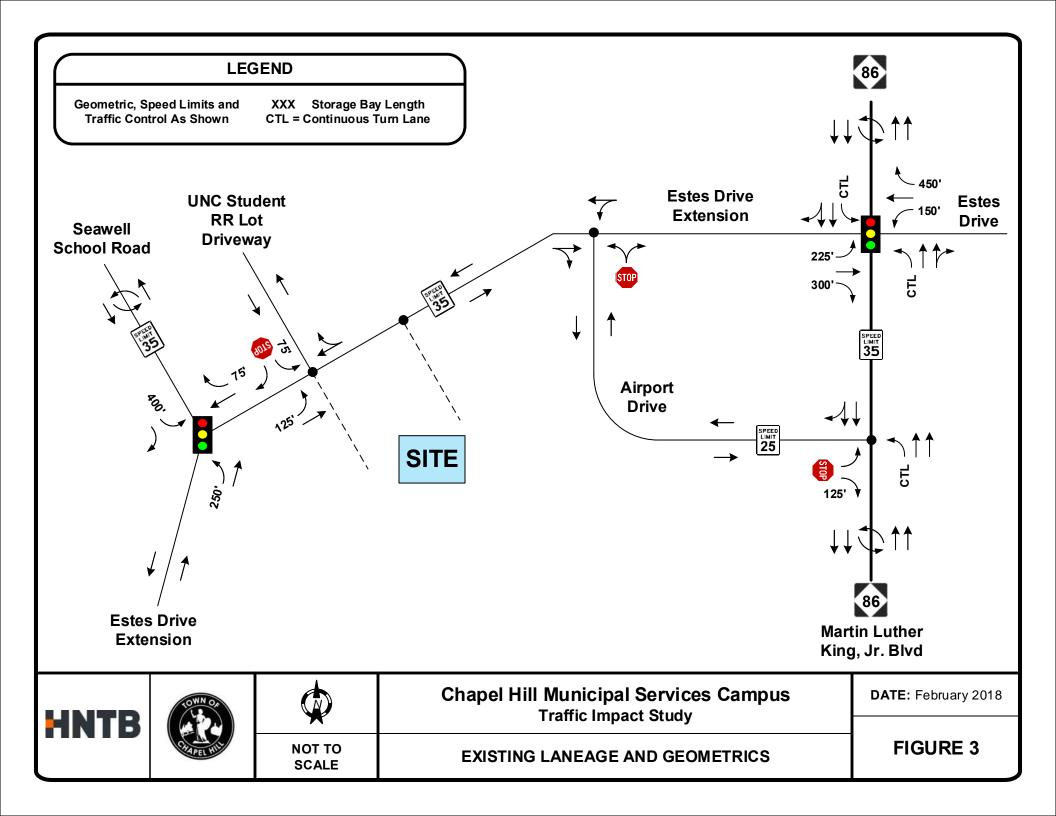
SCALE

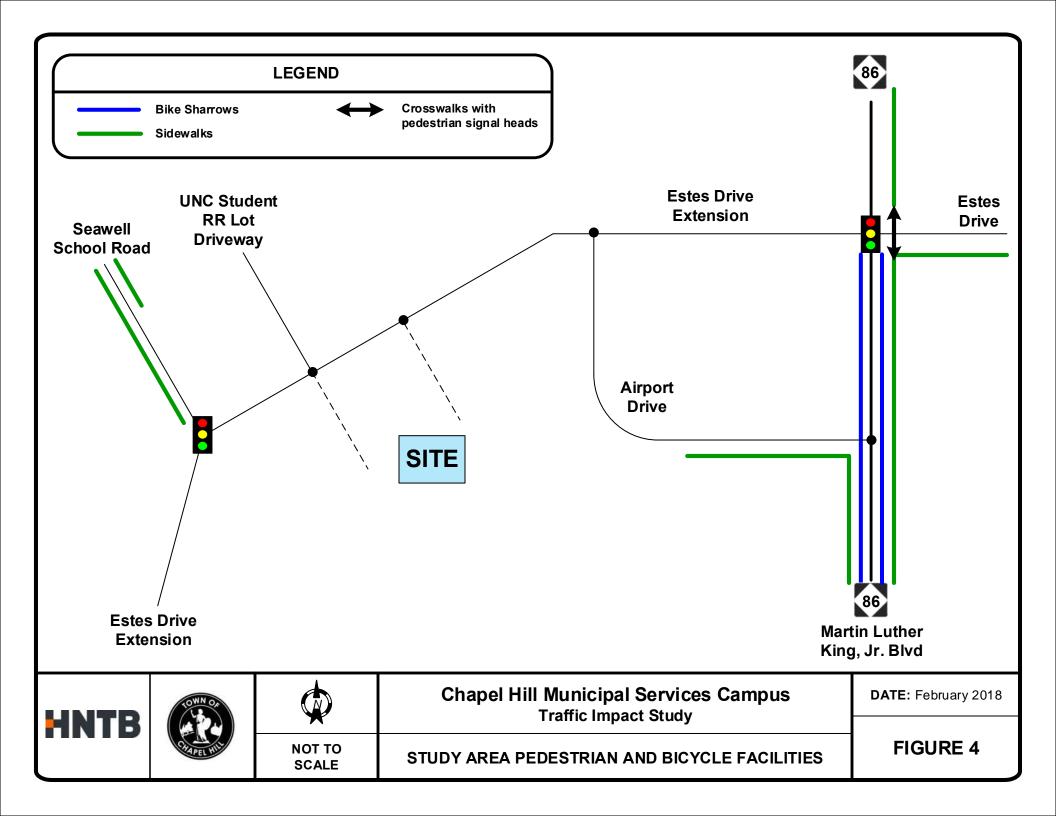
Chapel Hill Municipal Services Campus
Traffic Impact Study

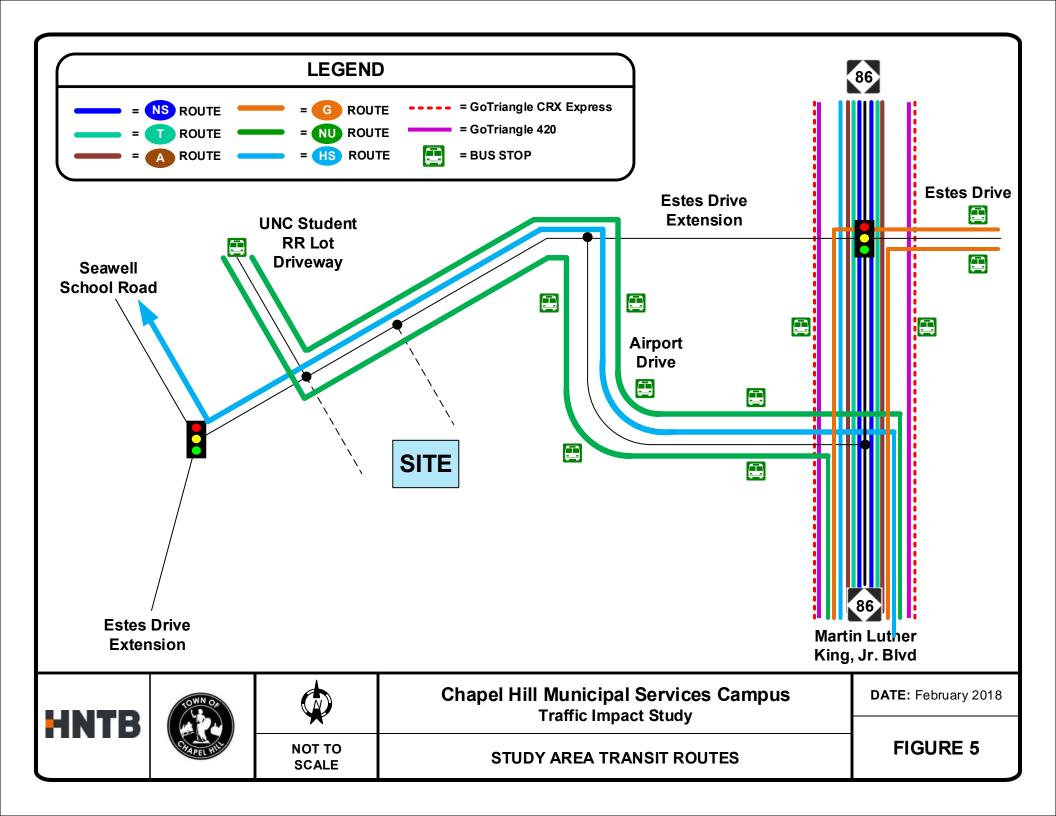
PRELIMINARY SITE PLAN

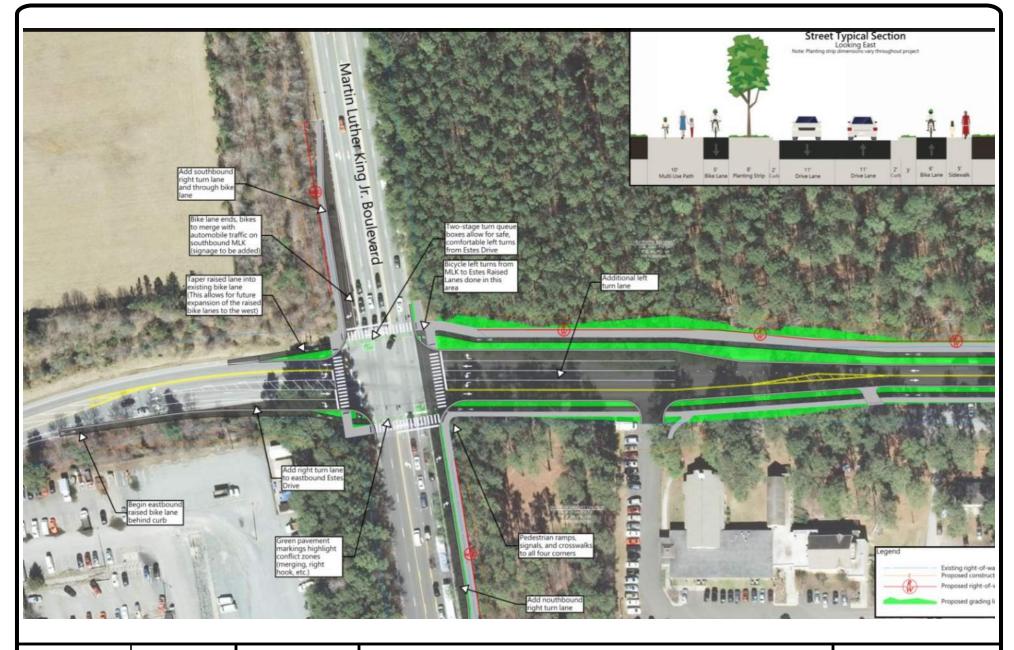
DATE: February 2018

FIGURE 2













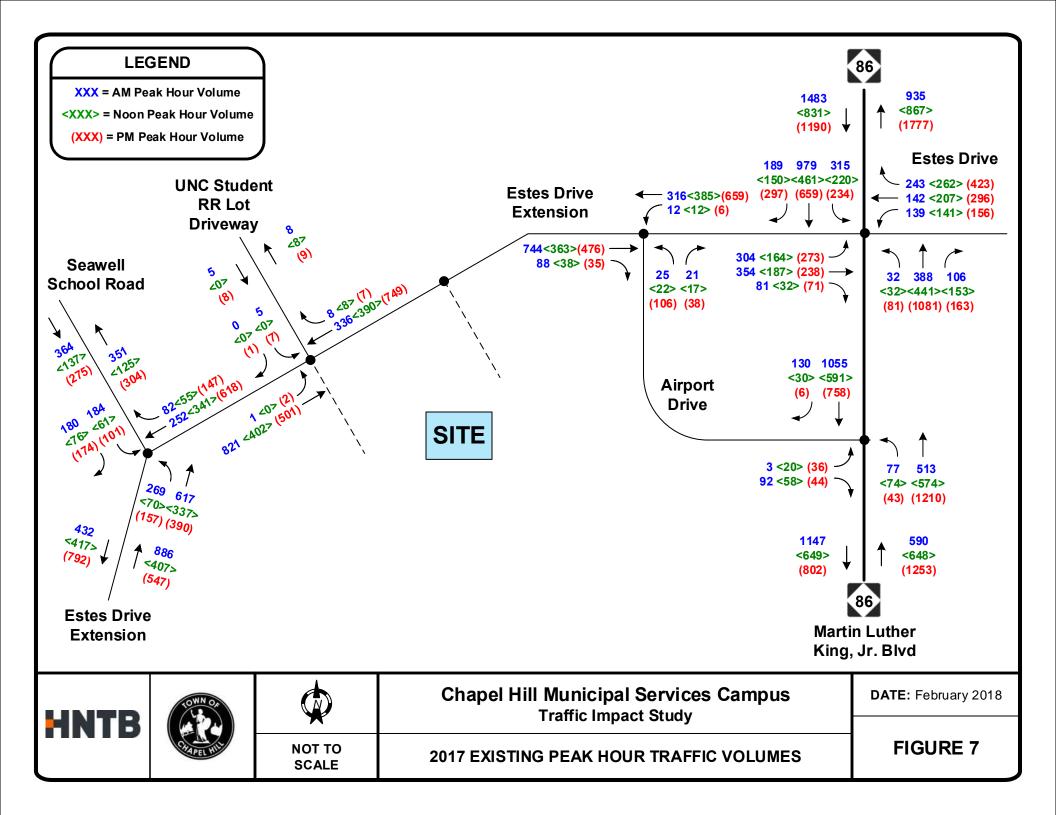


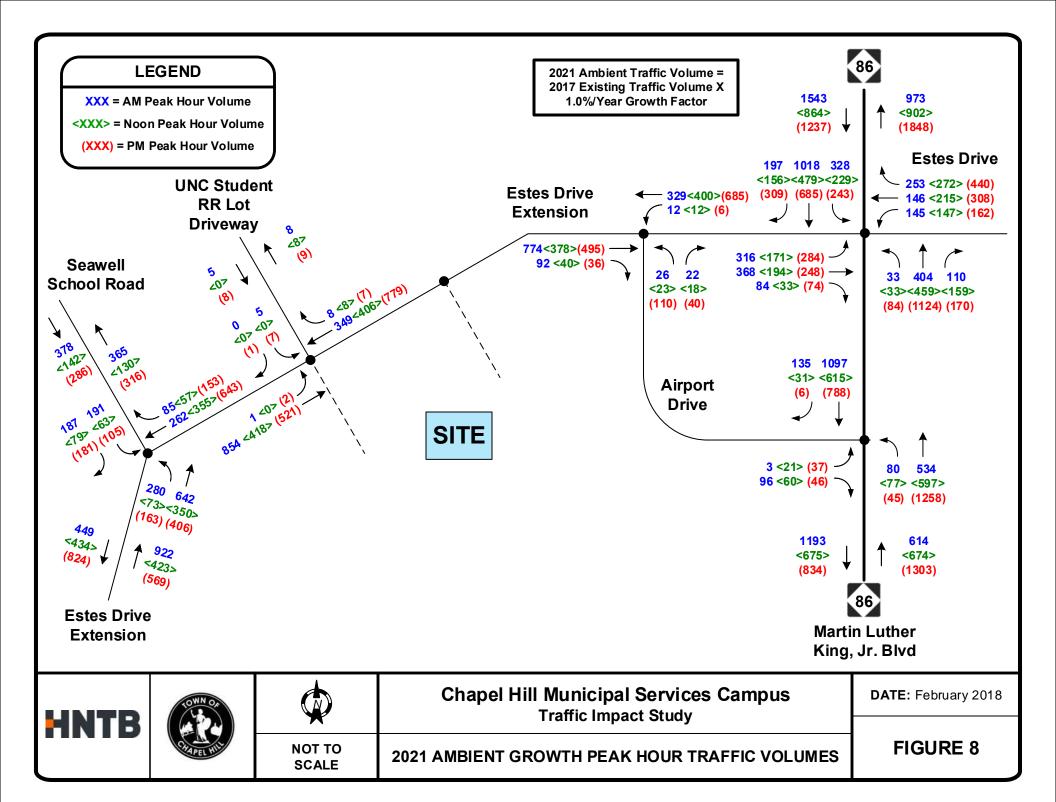
NOT TO SCALE Chapel Hill Municipal Services Campus
Traffic Impact Study

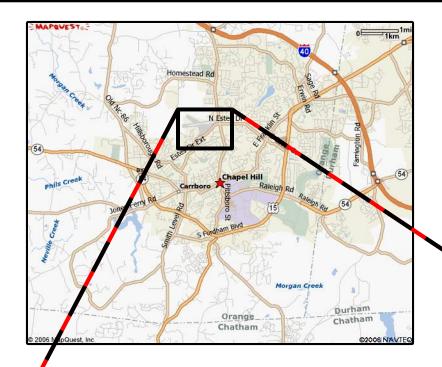
TOWN OF CHAPEL HILL – ESTES DRIVE IMPROVEMENTS SCHEMATIC DETAIL

DATE: February 2018

FIGURE 6

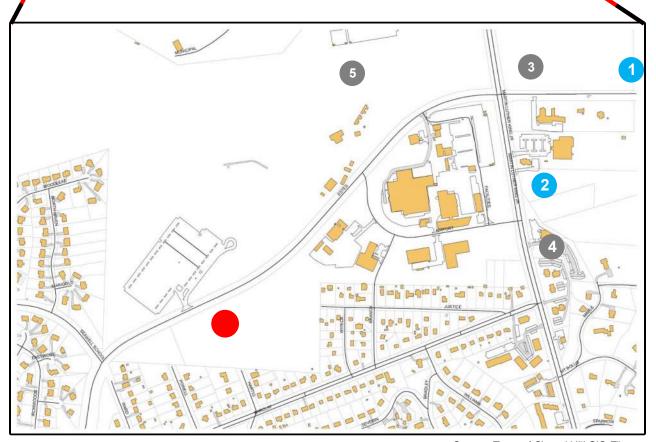






LEGEND

- Chapel Hill Retirement Residence
- 2 Sawmill Condominiums
- North Estes Mixed-Use Center
- Womens Health & Wellness Center
- 5 Carolina North
- Chapel Hill Municipal Services Campus Site
 - = Not Specifically Studied as
 Background Generator





Source: Town of Chapel Hill GIS Files



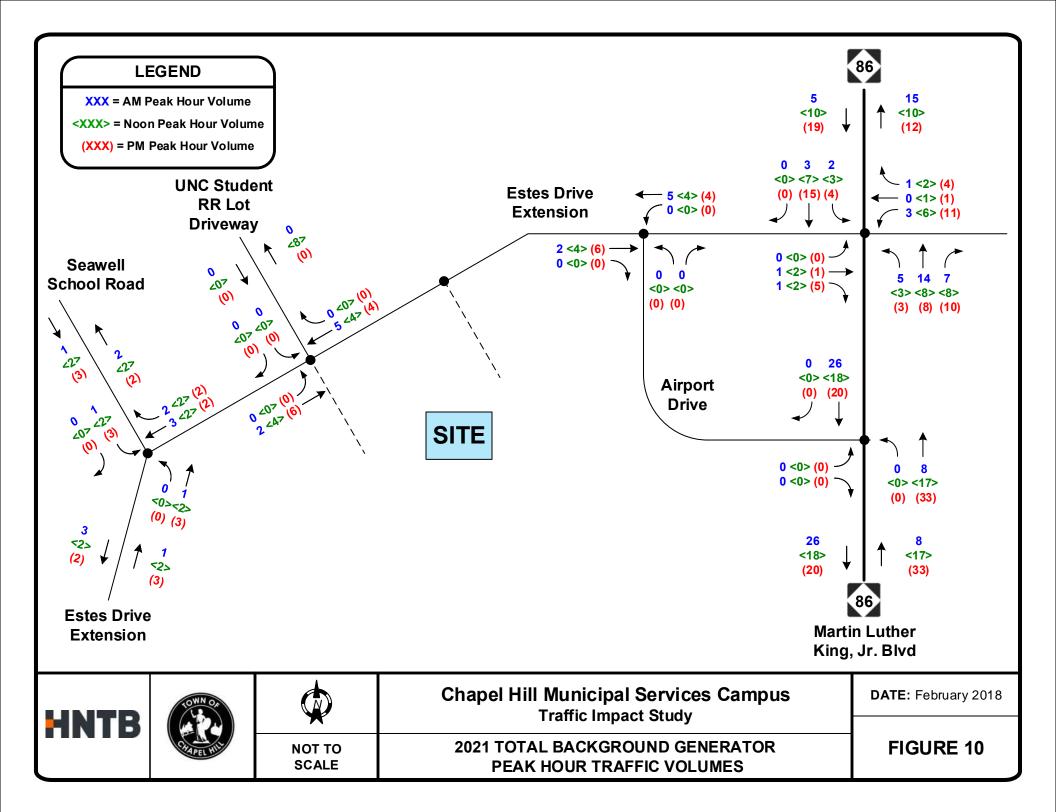


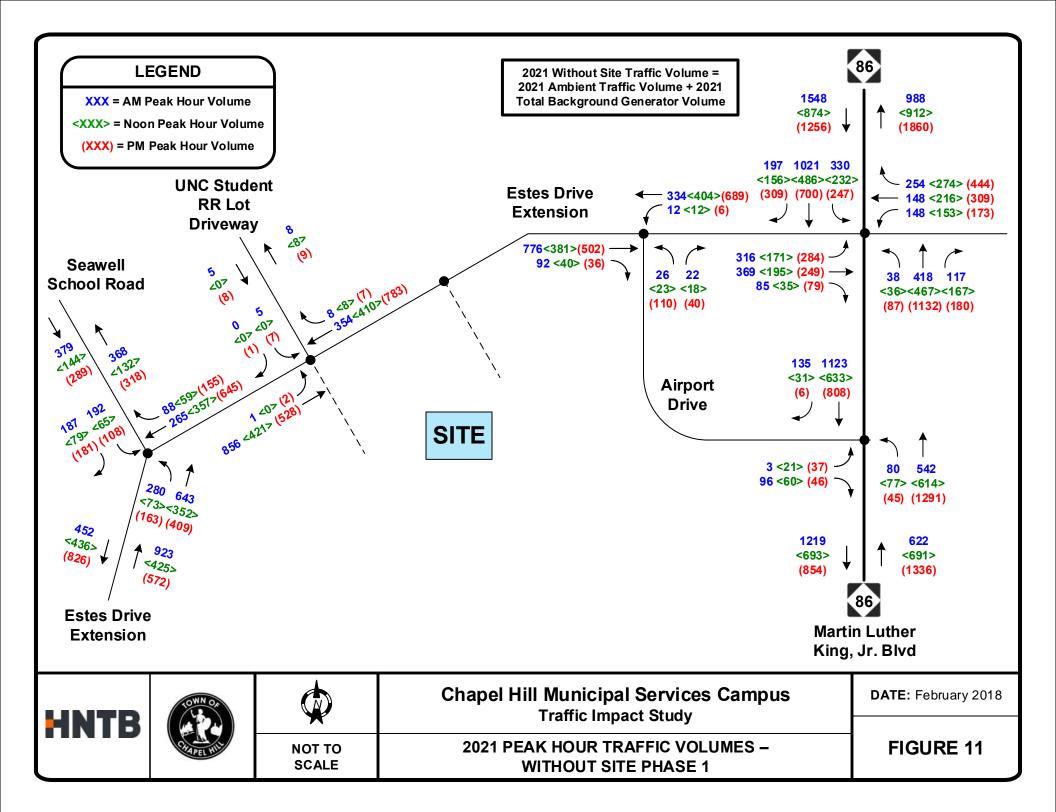
Chapel Hill Municipal Services Campus
Traffic Impact Study

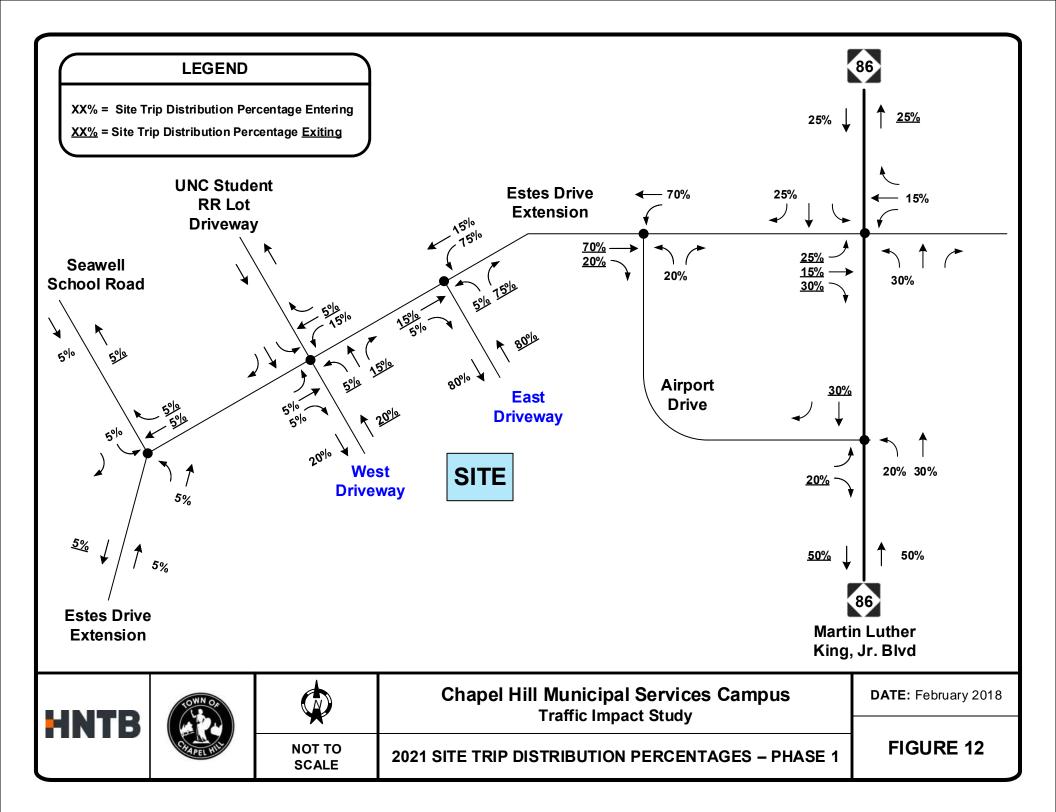
BACKGROUND DEVELOPMENT LOCATIONS

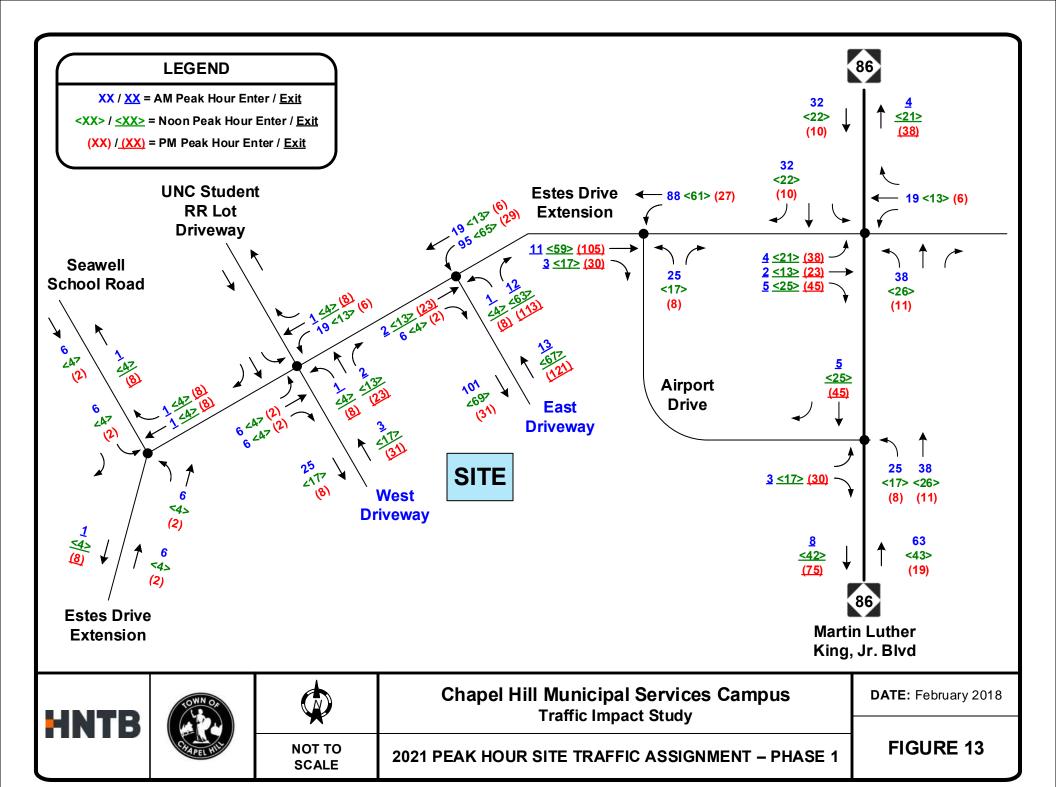
DATE: February 2018

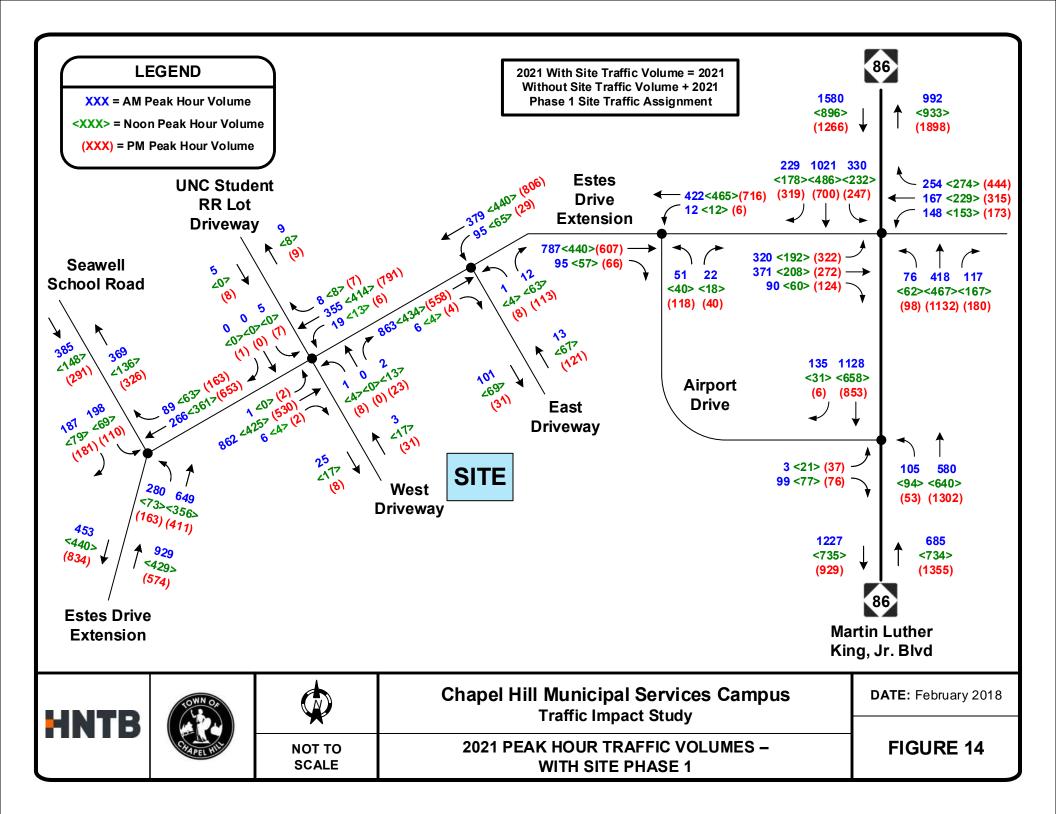
FIGURE 9

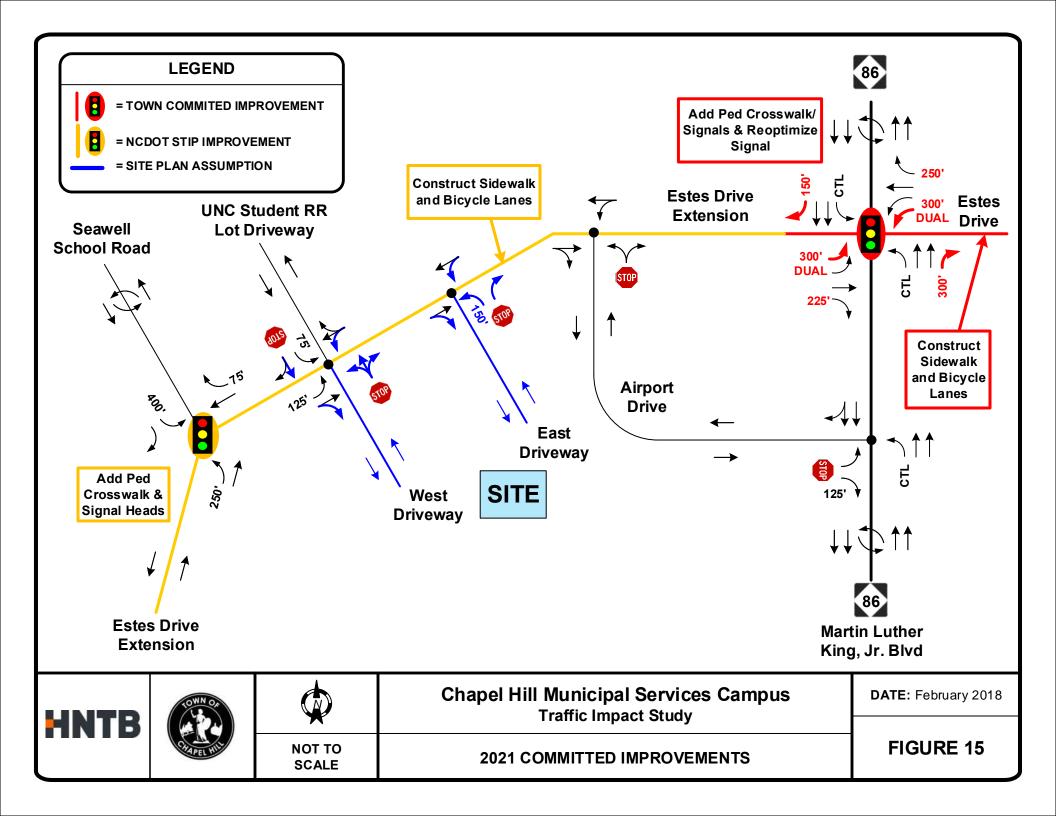


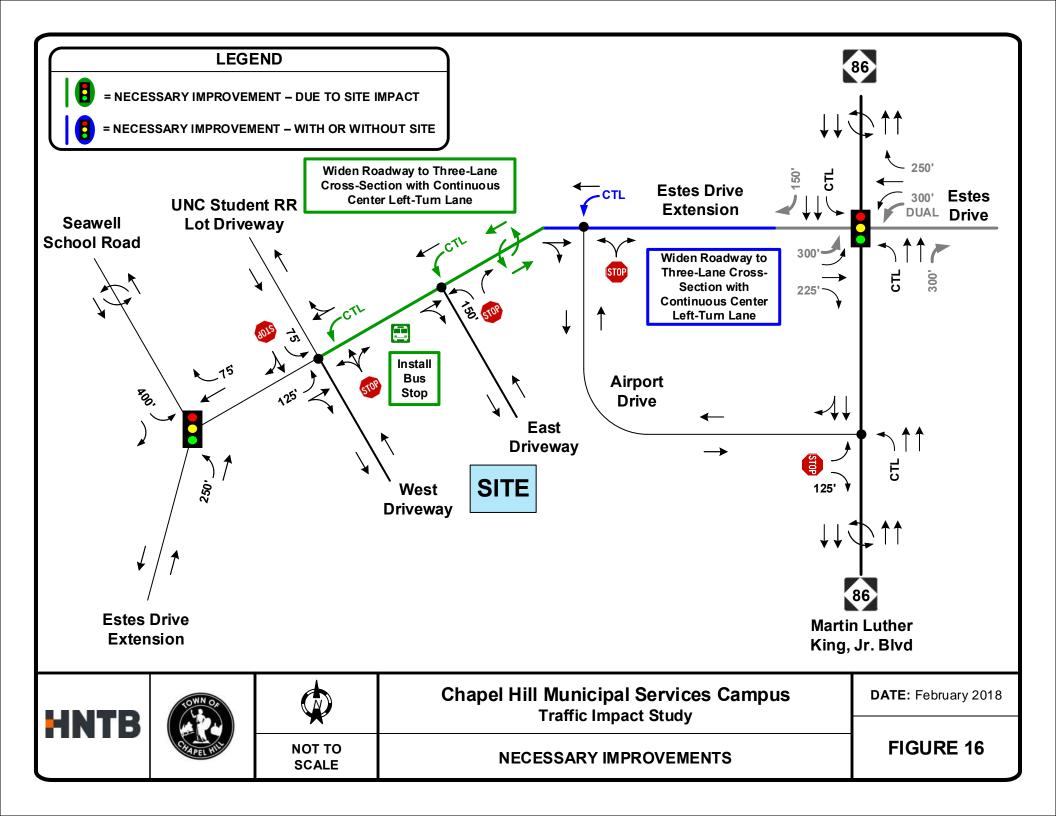




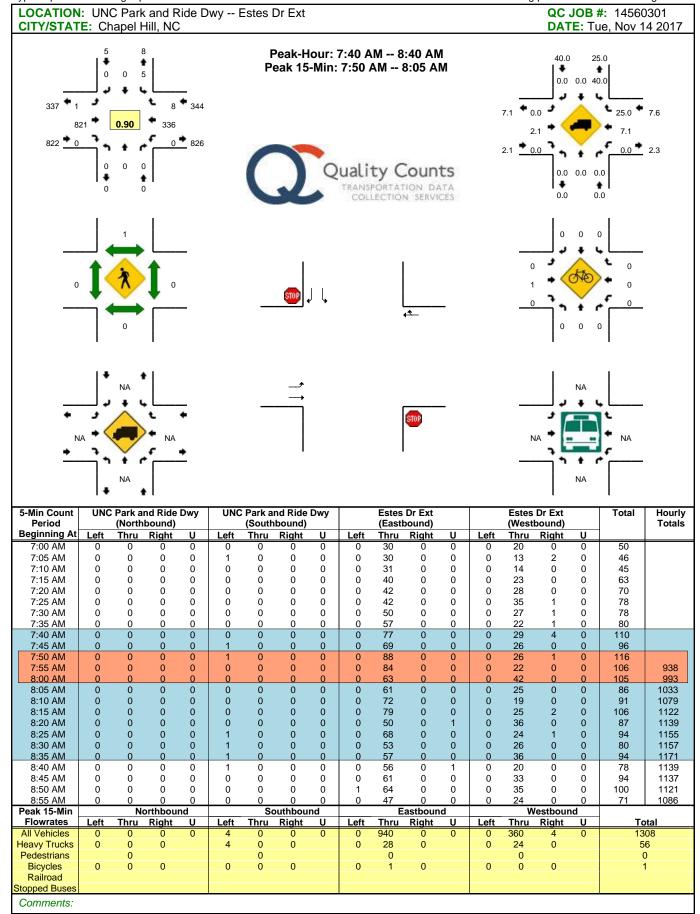


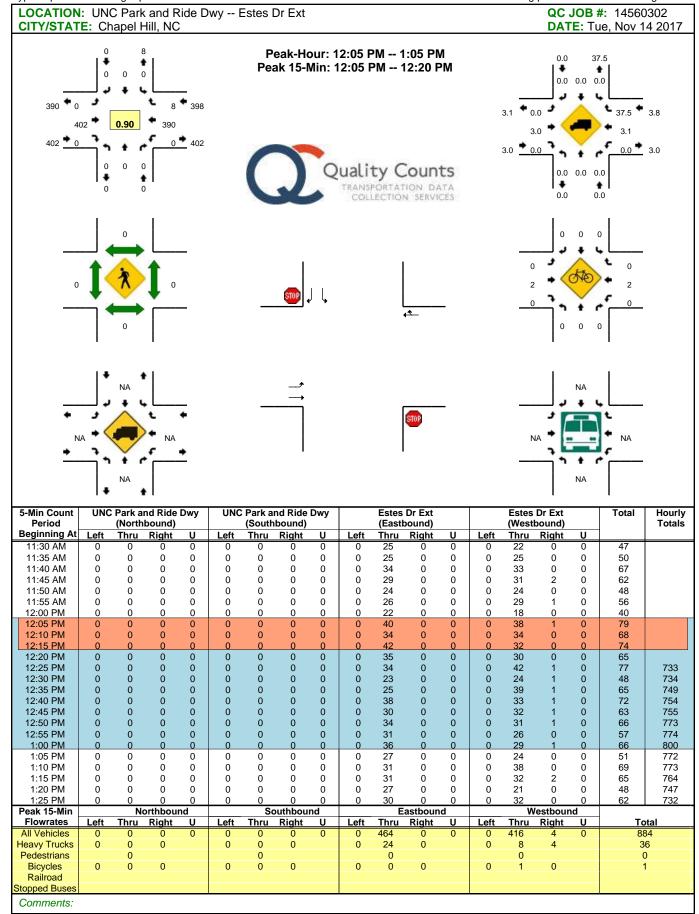


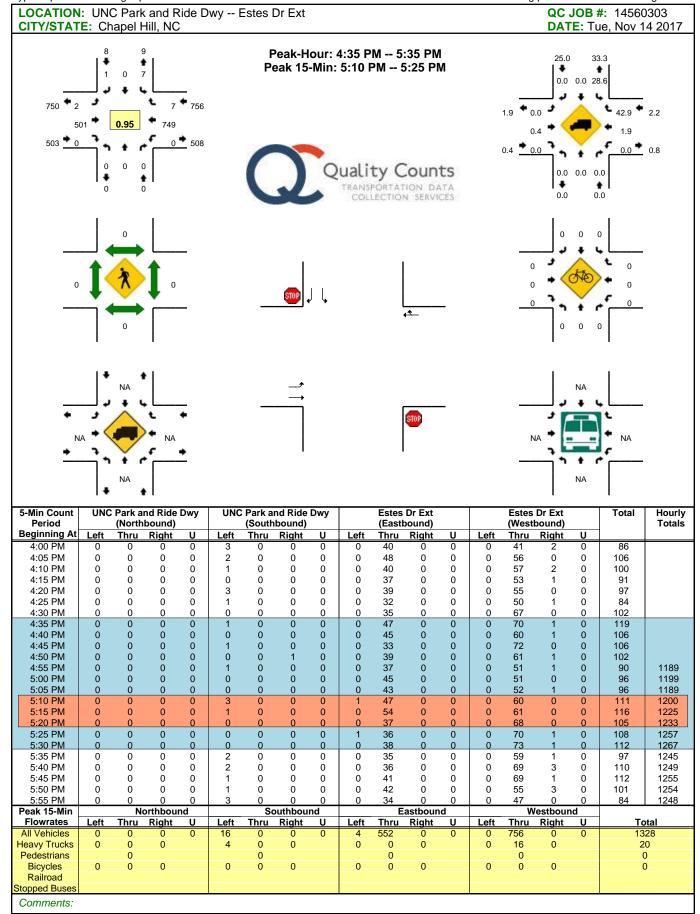


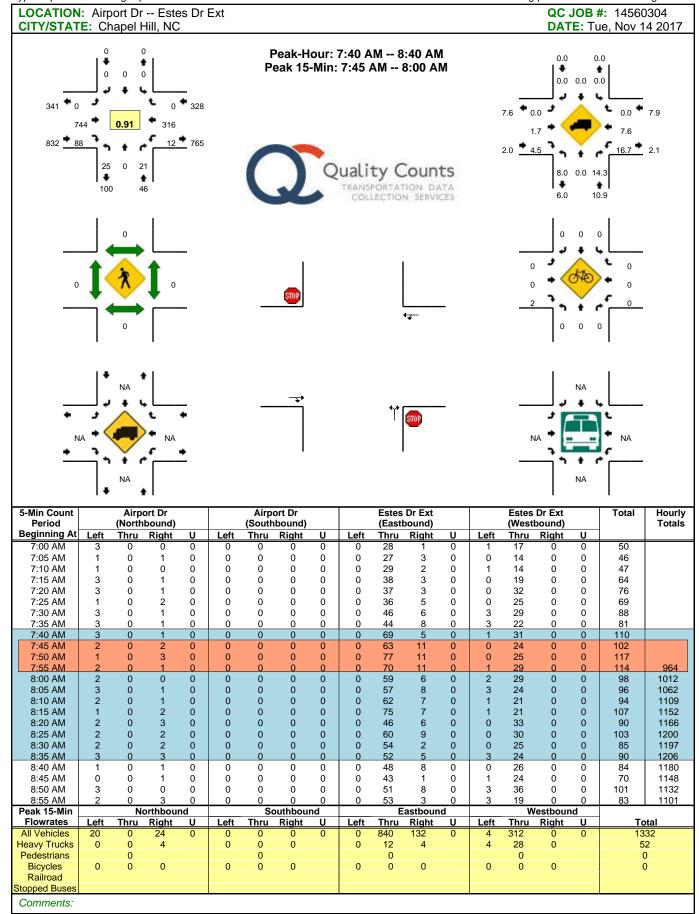


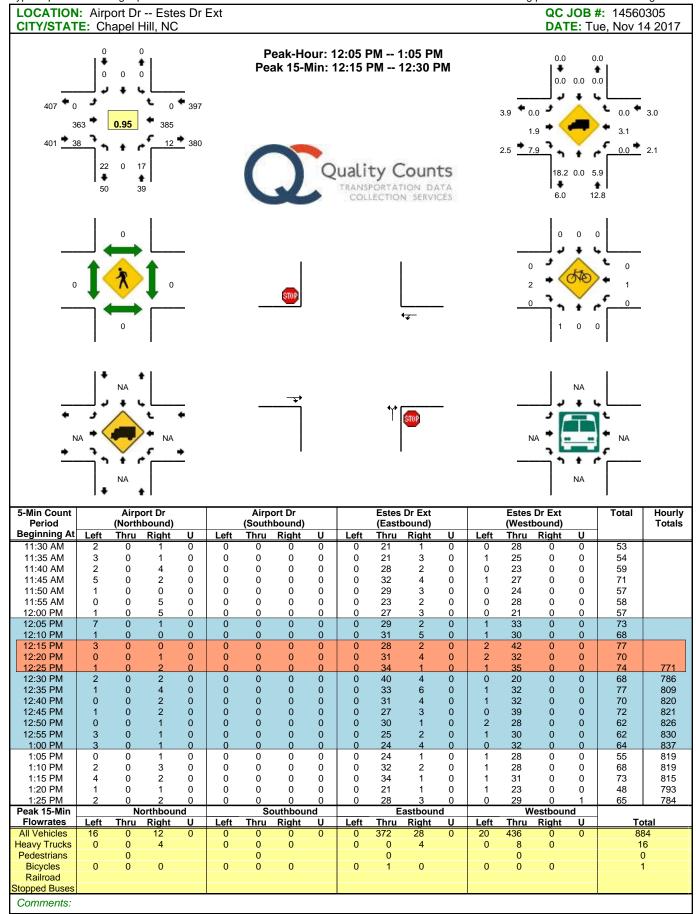
Appendix B – Traffic Count Data

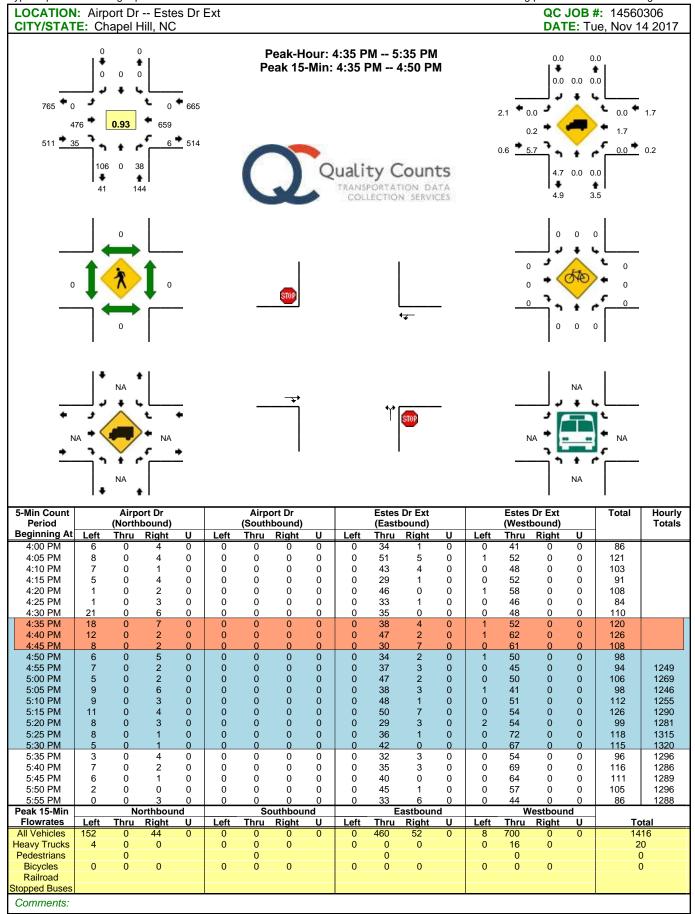


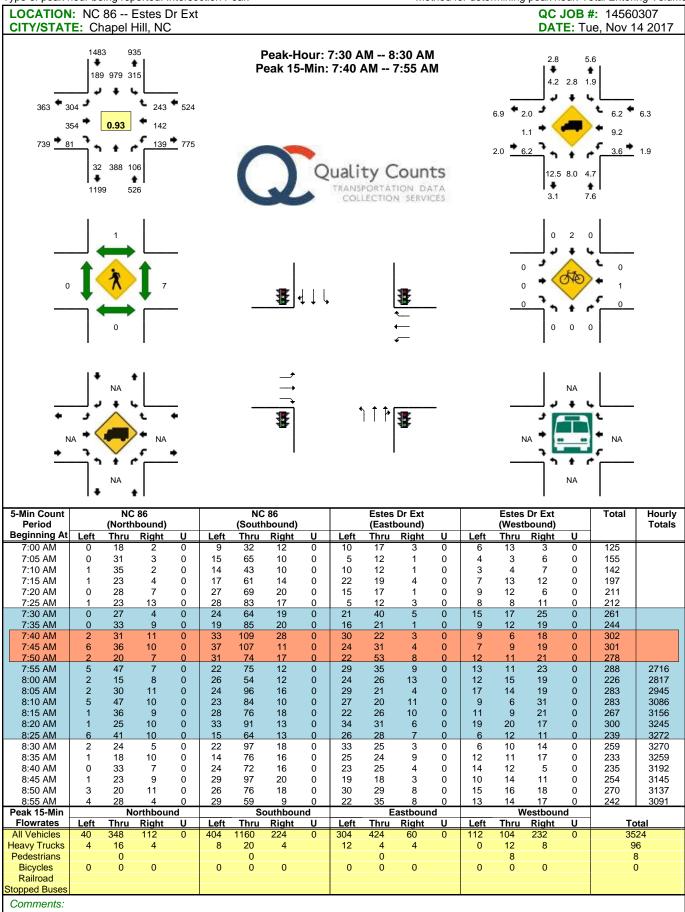


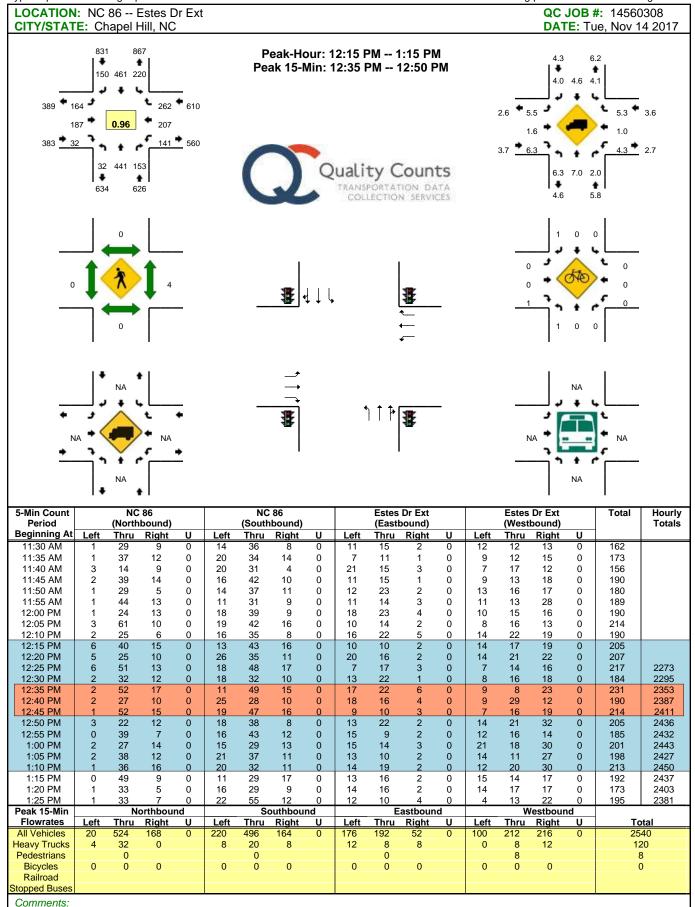


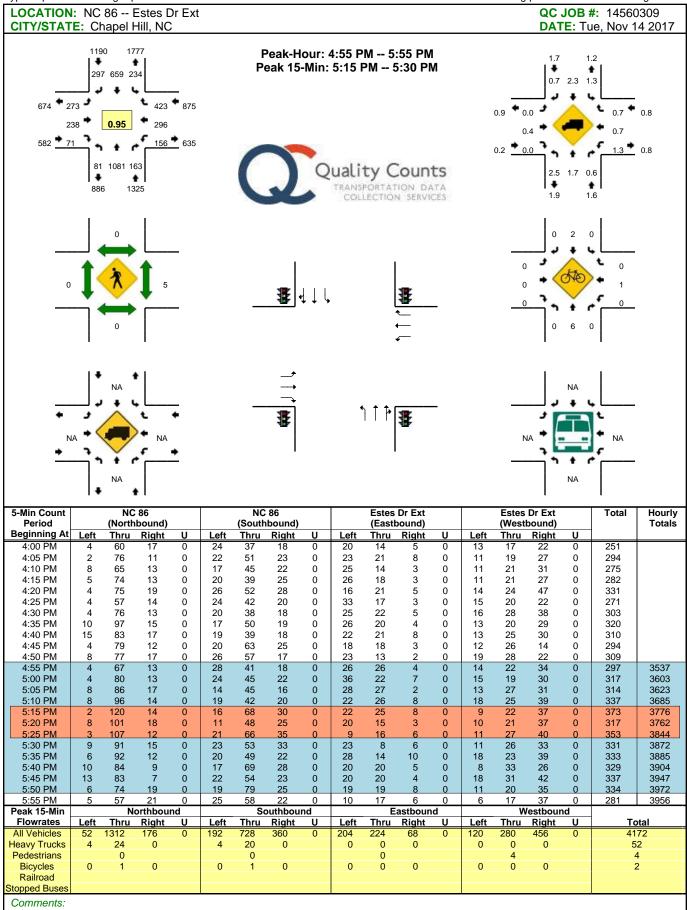


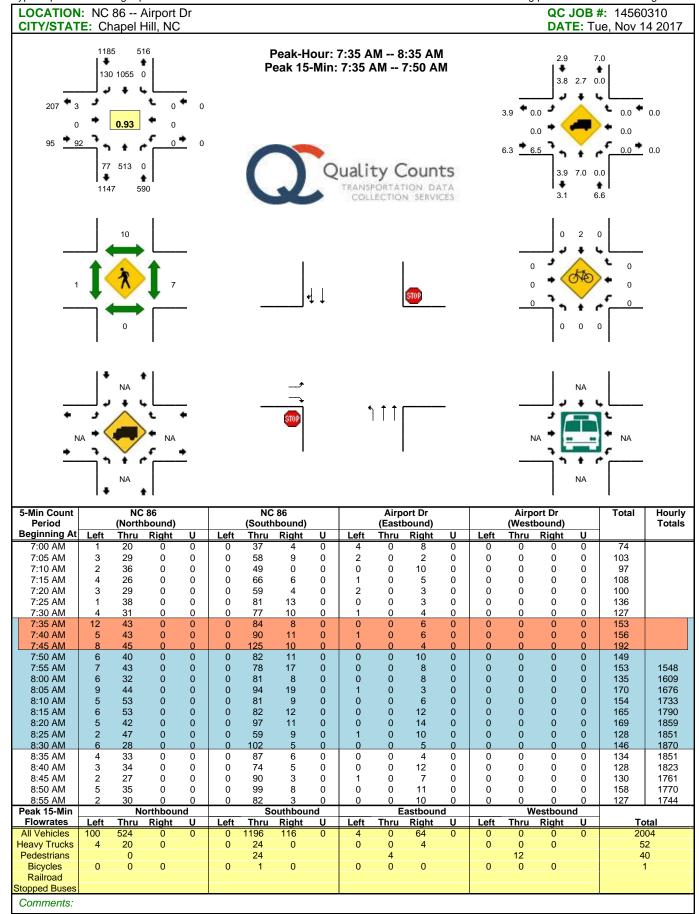


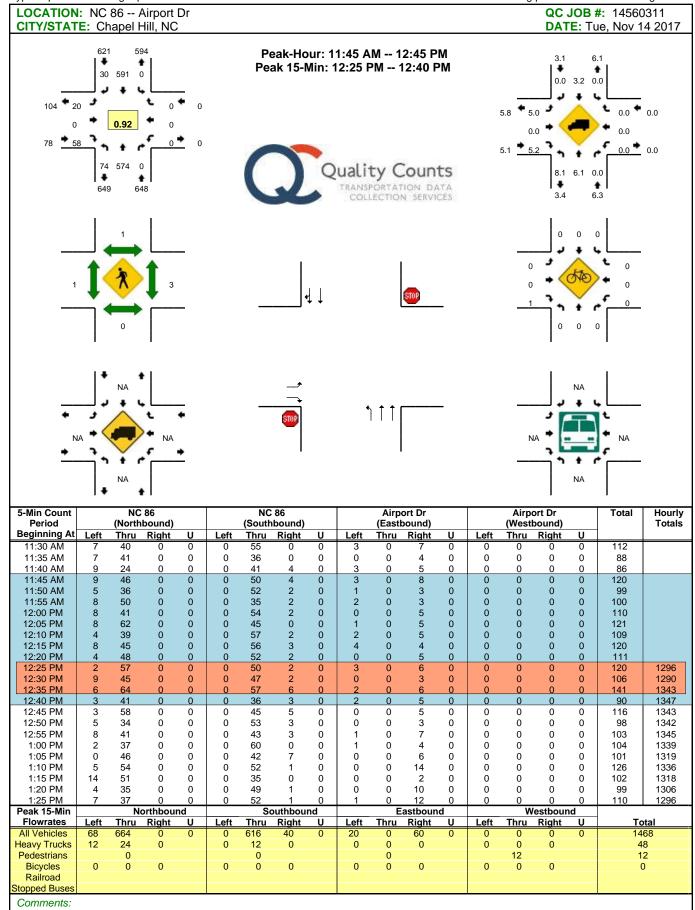


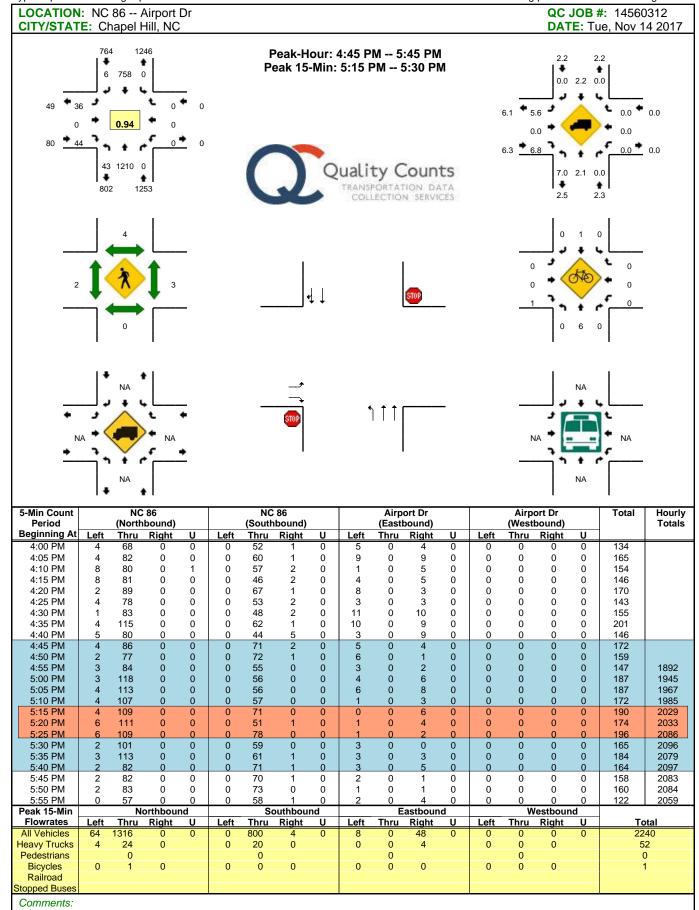


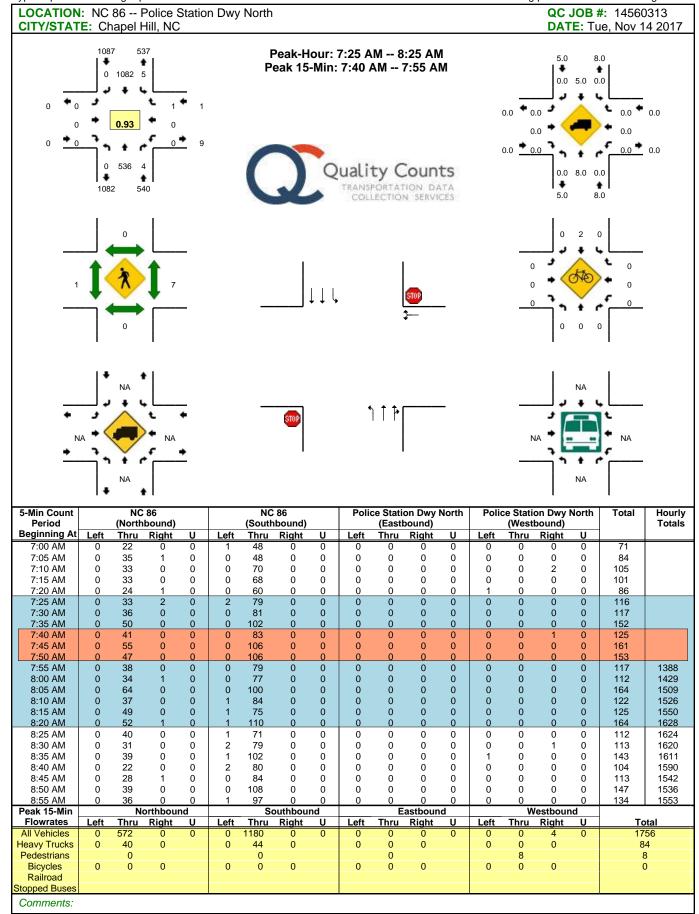


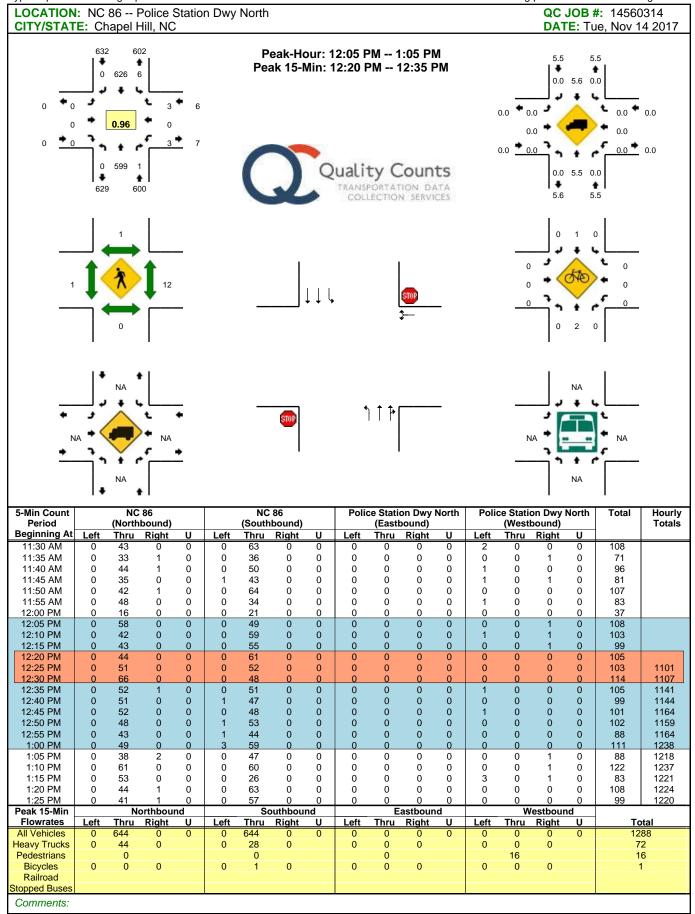


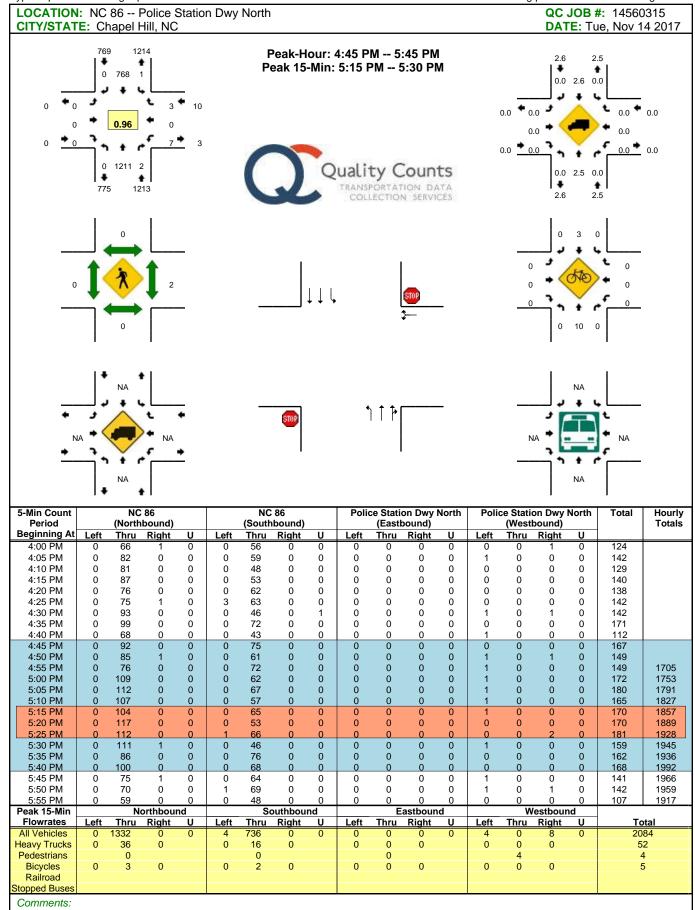


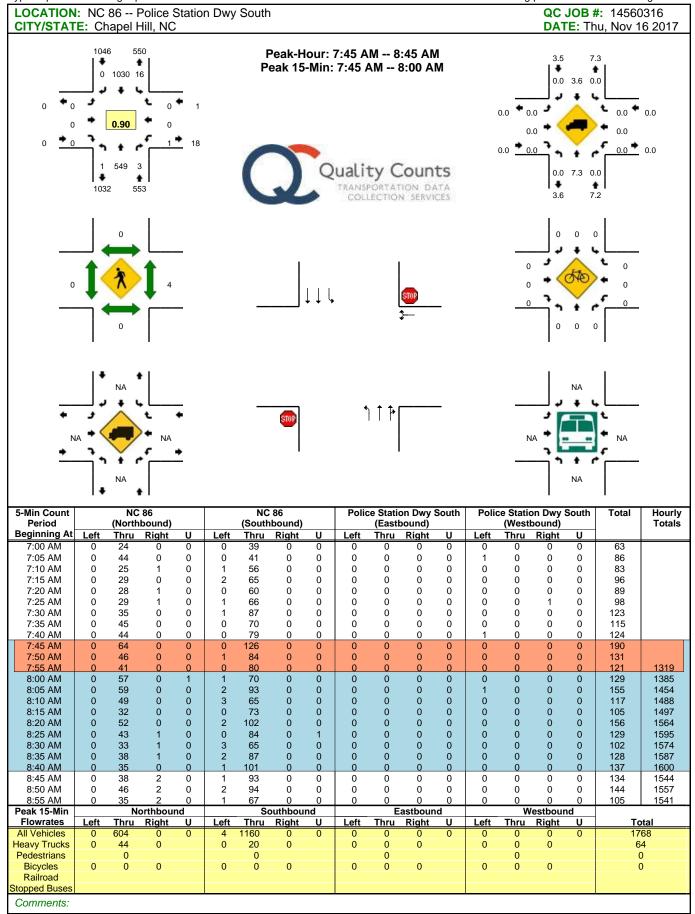


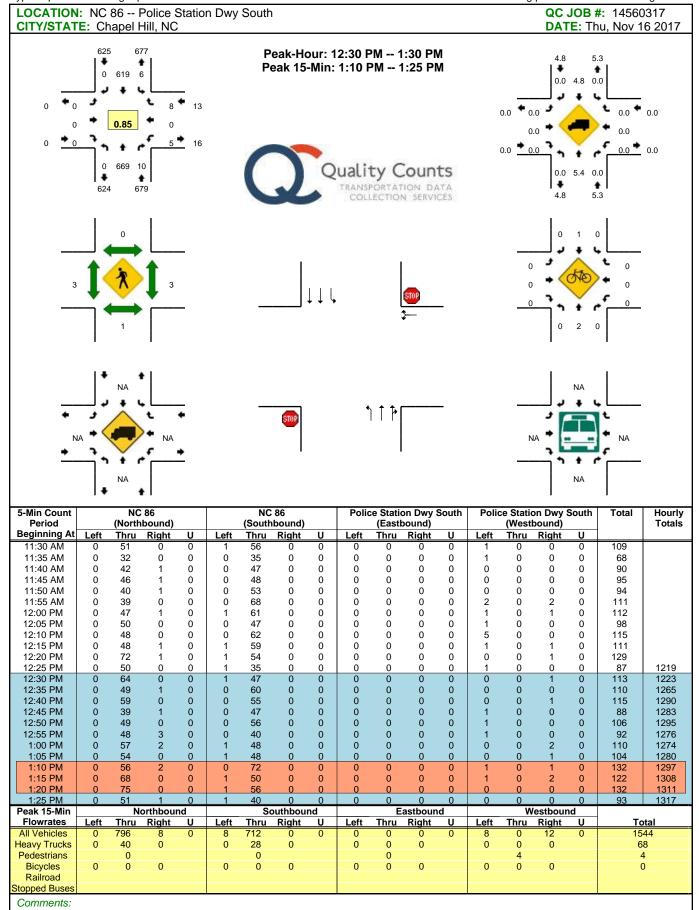


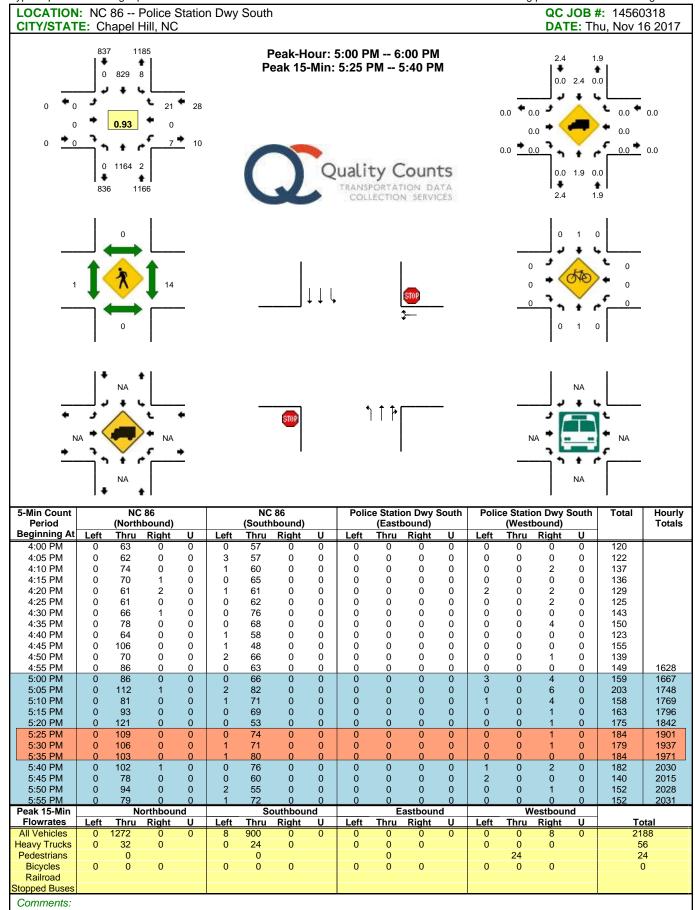


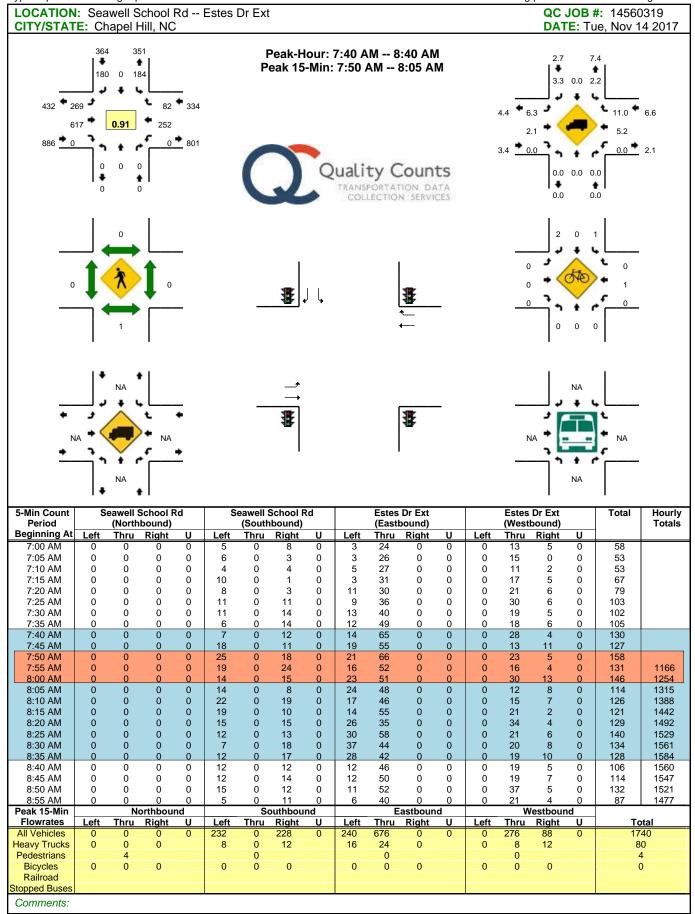


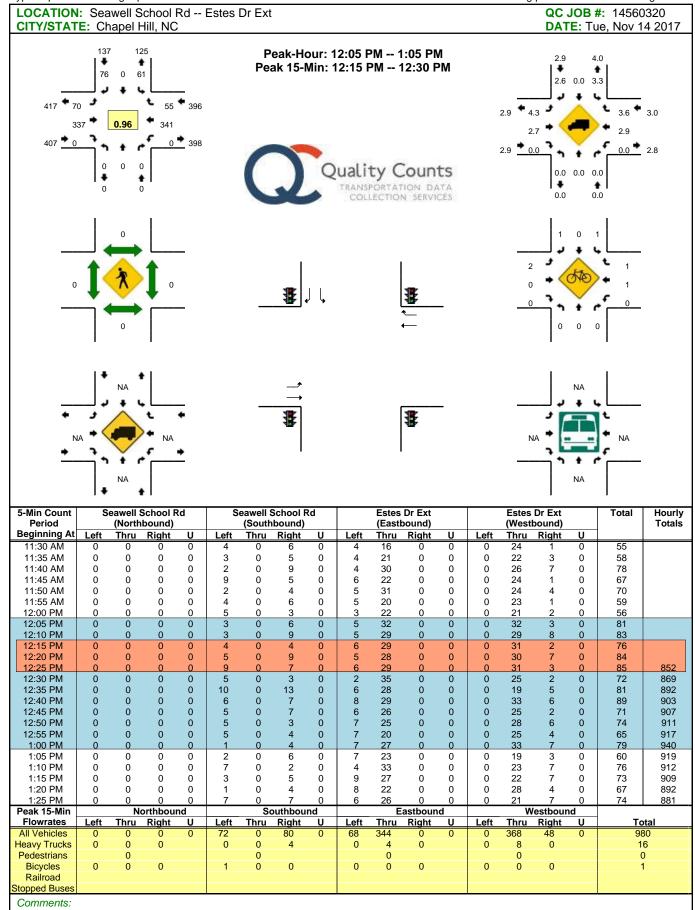


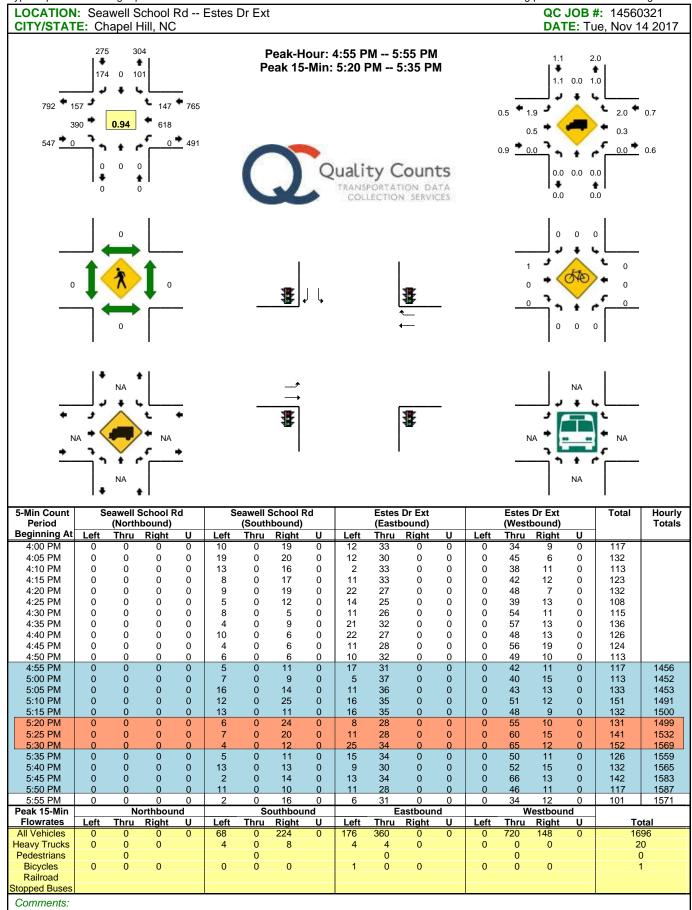




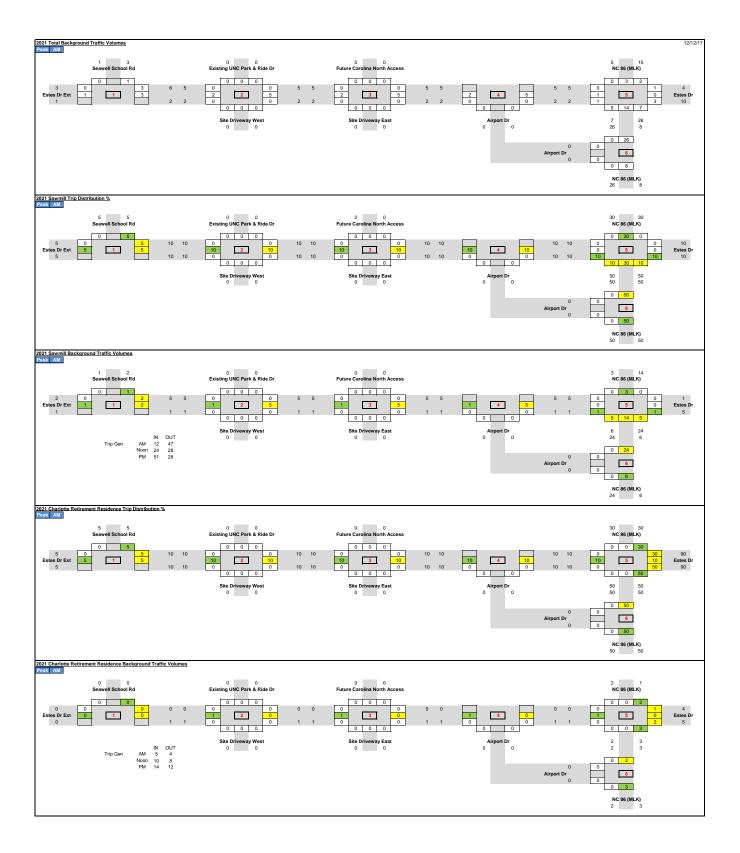


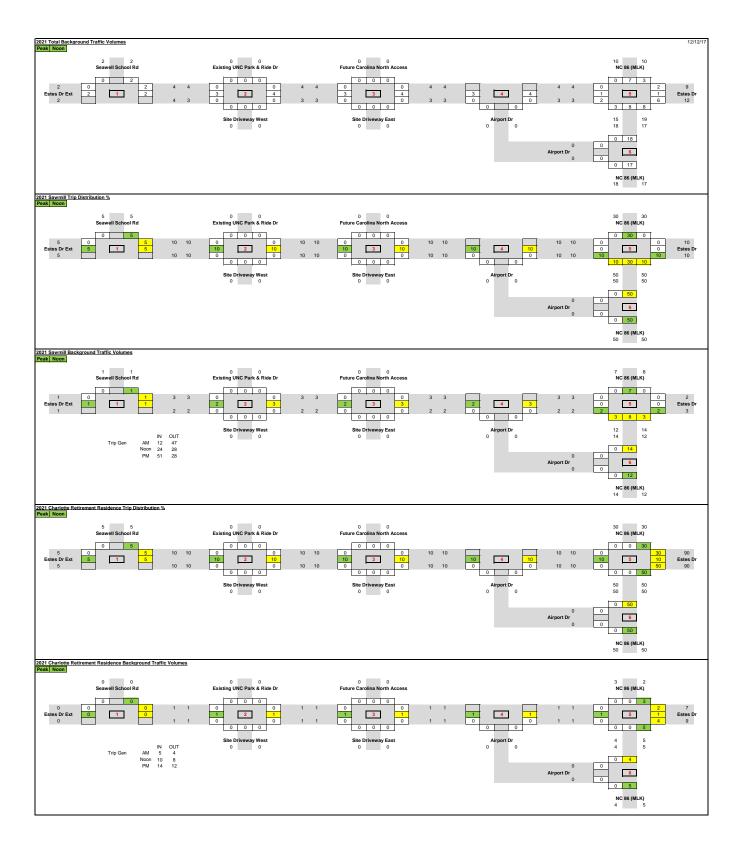


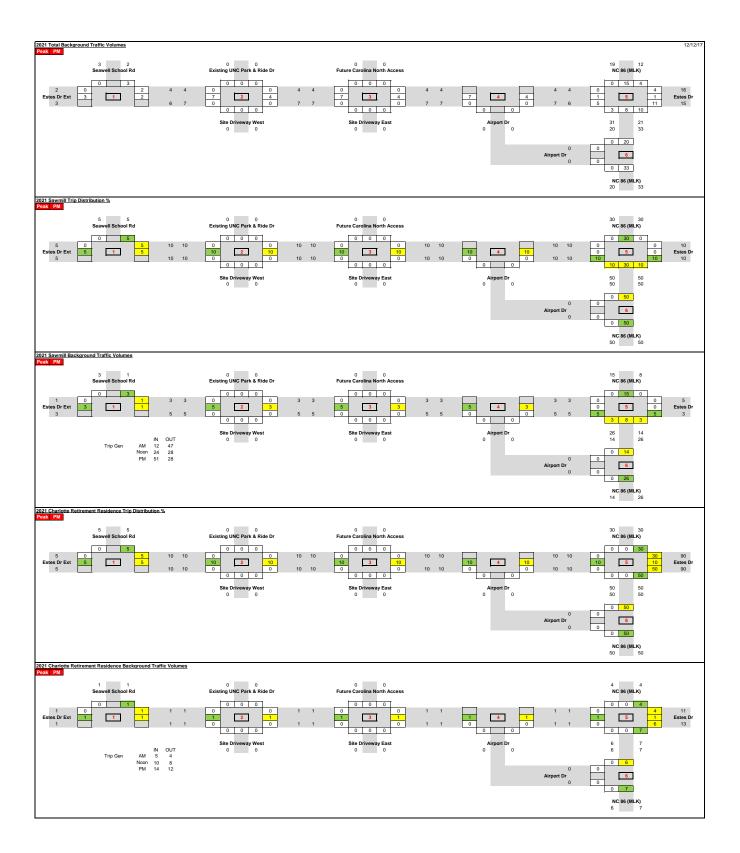




Appendix C – Background Generator Information







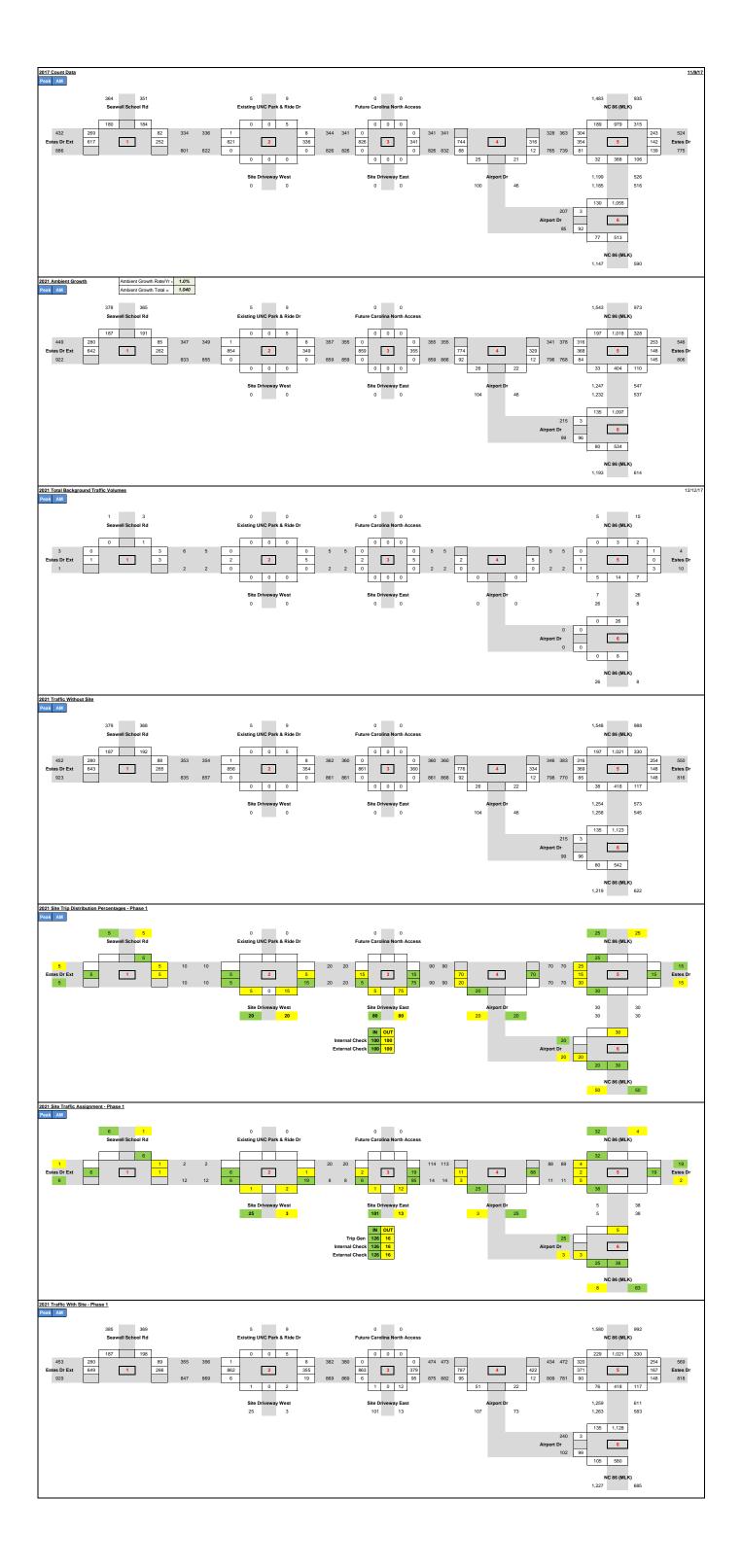
Appendix D - Traffic Volume Development Spreadsheets

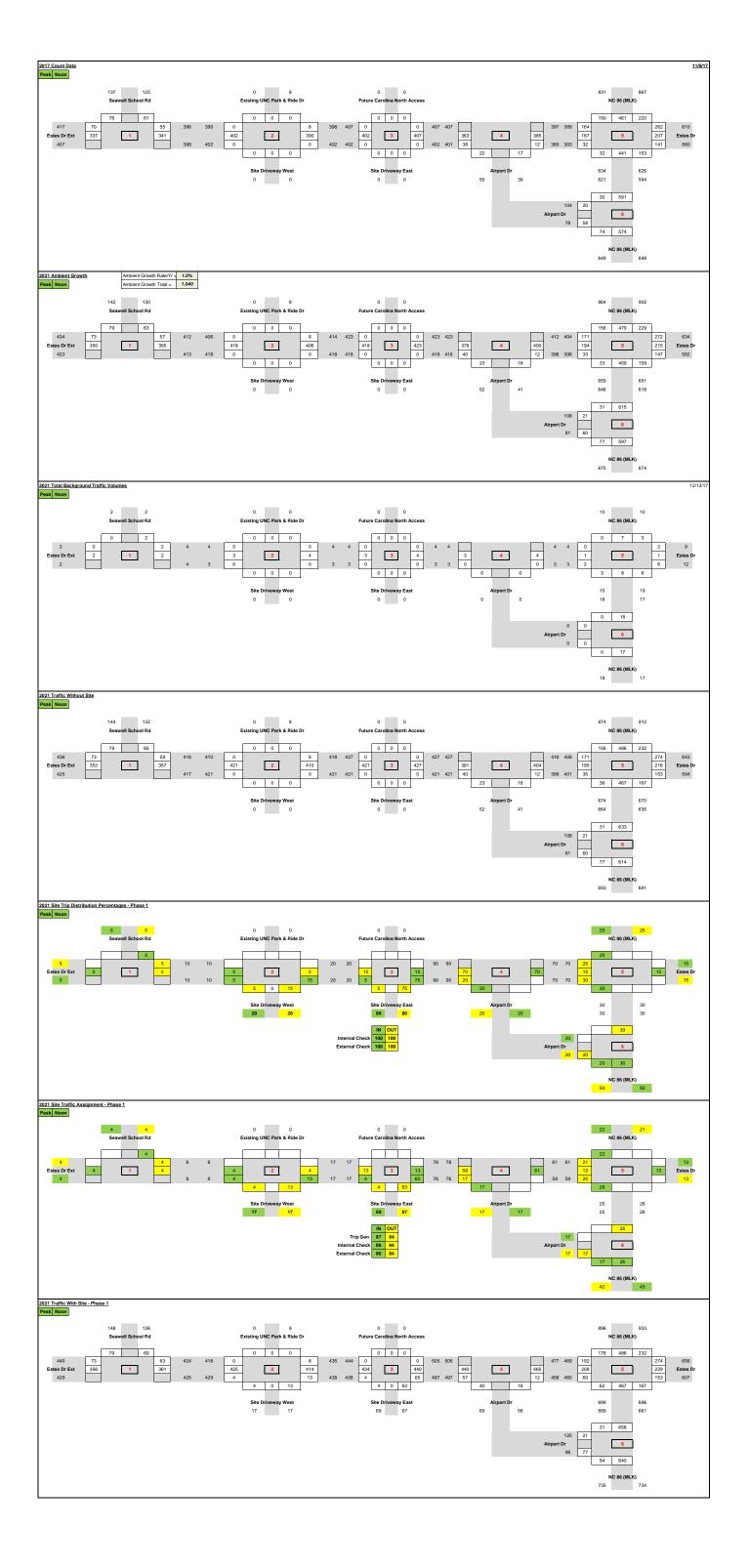
Chapel Hill Municipal Campus Trip Generation Results

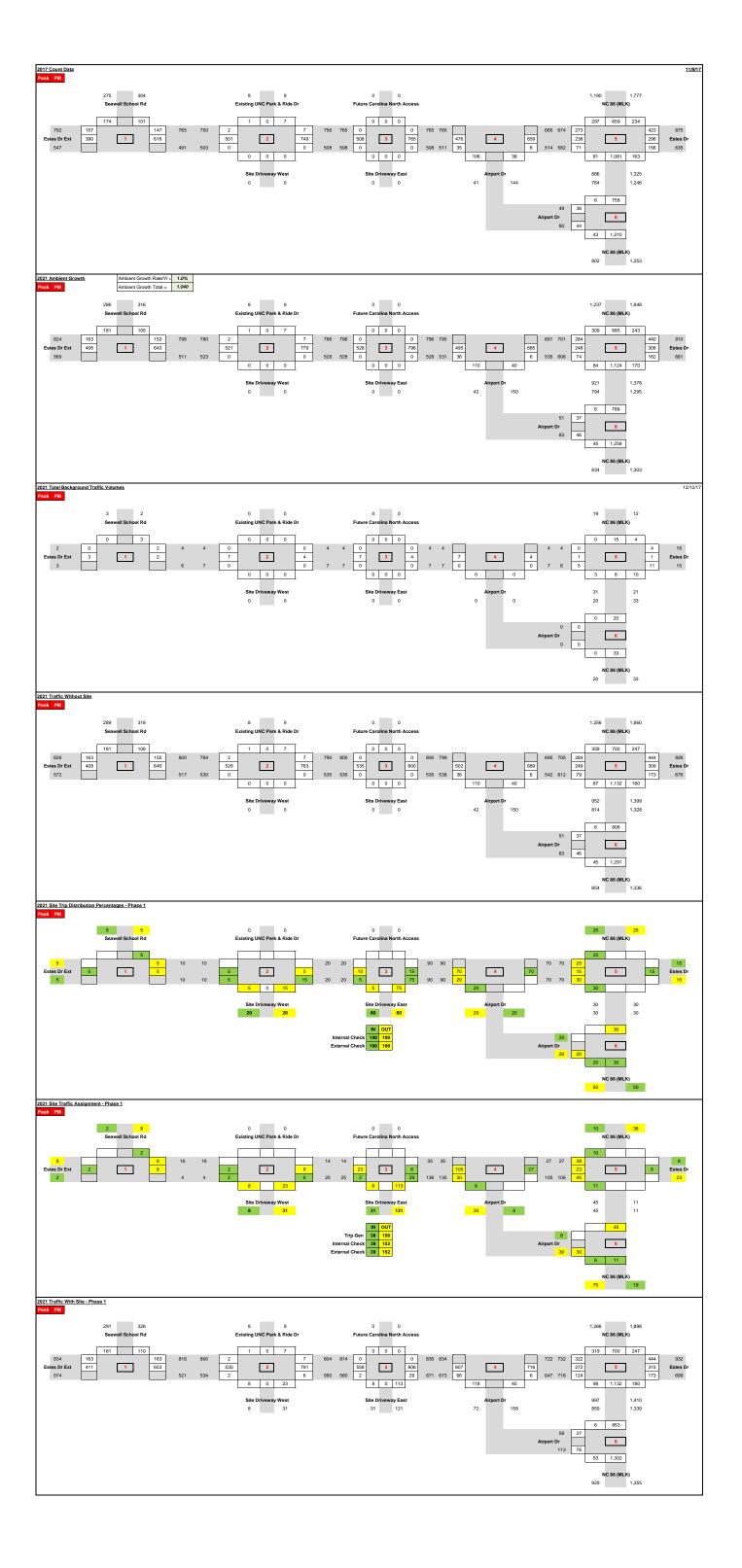
Phase 1	Phase 1			Daily		AM Peak				Noon Peak	(PM Peak		
ITE LUC	Description	Density	Enter	Exit	Total	Enter	Exit	Total	Enter	Exit	Total	Enter	Exit	Total
710	General Office Building	48467 SF	379	379	758	94	13	107	59	62	121	23	110	133
•	Police Station - Field Data Adjusted	24346 SF	146	146	292	32	3	35	28	22	50	15	40	55
	Total Phase 1	72813 SF	525	525	1050	126	16	142	87	84	171	38	150	188

				Daily			AM Peak			Noon Peak			PM Peak	
Existing	Description	Density	Enter	Exit	Total	Enter	Exit	Total	Enter	Exit	Total	Enter	Exit	Total
	Police Station - Field Data	24000 SF				30	3	33	27	21	48	14	38	52
		5% Inc				32	3	35	28	22	50	15	40	55

Daily Estimate for Police Station	AM Ratio	Noon Ratio	PM Ratio	Weighted
	0.33	0.41	0.41	0.38







Appendix E – Synchro Signalized Capacity Analysis Output

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1: Estes Drive Extension & Seawell School Road

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Lane Group	SEL	SER	NEL	NET	SWT	SWR	
Actuated g/C Ratio	0.23	0.23	0.58	0.58	0.31	0.63	
v/c Ratio	0.49	0.54	0.49	0.64	0.50	0.09	
Control Delay	23.5	25.3	9.3	11.6	19.5	3.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	23.5	25.3	9.3	11.6	19.5	3.9	
LOS	С	С	Α	В	В	Α	
Approach Delay	24.4			10.9	15.7		
Approach LOS	С			В	В		
Queue Length 50th (ft)	54	53	40	120	70	9	
Queue Length 95th (ft)	127	128	100	283	150	20	
Internal Link Dist (ft)	782			565	745		
Turn Bay Length (ft)	400		250			75	
Base Capacity (vph)	667	597	608	1823	1728	1199	
Starvation Cap Reductn	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.30	0.33	0.49	0.37	0.16	0.08	
Intersection Summary							
Area Type:	Other						
Cycle Length: 100							
Actuated Cycle Length: 54	1.6						
Natural Cycle: 50							
Control Type: Actuated-U	ncoordinated						
Maximum v/c Ratio: 0.64							
Intersection Signal Delay:					tersection		
Intersection Capacity Utiliz	zation 51.0%			IC	U Level o	of Service	e A
Analysis Period (min) 15							

Splits and Phases: 1: Estes Drive Extension & Seawell School Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ች	†	7	ች	†	7	ሻ	ΦÞ		ሻ	↑ ↑	
Traffic Volume (vph)	304	354	81	139	142	243	32	388	106	315	979	189
Future Volume (vph)	304	354	81	139	142	243	32	388	106	315	979	189
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)	1000	0%	1000	1000	1%	1000	1000	1%	1000	1000	-2%	1000
Storage Length (ft)	225	070	300	150	170	450	0	170	0	0	270	0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	1.00	1.00	1.00	1.00	1.00	0.99	1.00	0.99	0.50	0.99	0.00	0.50
Frt	1.00		0.850			0.850		0.968		0.55	0.976	
Flt Protected	0.950		0.000	0.950		0.000	0.950	0.500		0.950	0.570	
Satd. Flow (prot)	1770	1863	1583	1694	1783	1516	1678	3223	0	1770	3455	0
Flt Permitted	0.444	1000	1000	0.156	1700	1010	0.097	3223	U	0.455	0400	U
Satd. Flow (perm)	827	1863	1583	278	1783	1497	171	3223	0	840	3455	0
Right Turn on Red	021	1003	No	210	1703	No	171	3223	No	040	3433	No
Satd. Flow (RTOR)			INO			INO			INU			INU
Link Speed (mph)		35			35			35			35	
,		1360			1121			1058			1059	
Link Distance (ft)		26.5			21.8			20.6			20.6	
Travel Time (s)	1	20.5			21.0	1		20.0	7	7	20.0	
Confl. Peds. (#/hr)	-	0.02	0.02	0.02	0.02	1	0.02	0.02		-	0.02	0.02
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	2%	2%	6%	6%	6%	7%	7%	7%	3%	3%	3%
Adj. Flow (vph)	327	381	87	149	153	261	34	417	114	339	1053	203
Shared Lane Traffic (%)	207	201	87	140	150	261	34	E21	٥	220	1056	0
Lane Group Flow (vph)	327	381		149	153			531	0	339	1256	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane	4.00	4.00	4.00	4.04	4.04	4.04	4.04	4.04	4.04	0.00	0.00	0.00
Headway Factor	1.00	1.00	1.00	1.01	1.01	1.01	1.01	1.01	1.01	0.99	0.99	0.99
Turning Speed (mph)	15	NI A	9	15	N I A	9	15	NI A	9	15	NI A	9
Turn Type	pm+pt	NA		pm+pt	NA	pm+ov	pm+pt	NA		pm+pt	NA	
Protected Phases	7	4	5	3	8	1	5	2		1	6	
Permitted Phases	4	4	4	8	0	8	2	0		6	0	
Detector Phase	7	4	5	3	8	1	5	2		1	6	
Switch Phase	7.0	7.0	7.0	7.0	7.0	7.0	7.0	40.0		7.0	40.0	
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	12.0		7.0	12.0	
Minimum Split (s)	12.0	13.0	13.0	13.0	13.0	13.0	13.0	34.0		13.0	18.0	
Total Split (s)	25.0	40.0	20.0	25.0	40.0	35.0	20.0	50.0		35.0	65.0	
Total Split (%)	16.7%	26.7%	13.3%	16.7%	26.7%	23.3%	13.3%	33.3%		23.3%	43.3%	
Yellow Time (s)	3.0	3.8	3.0	3.0	3.8	3.0	3.0	4.3		3.0	4.3	
All-Red Time (s)	1.9	1.7	2.8	2.4	1.7	2.8	2.8	1.7		2.8	1.7	
Lost Time Adjust (s)	0.1	-0.5	-0.8	-0.4	-0.5	-0.8	-0.8	-1.0		-0.8	-1.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lead		Lag	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Recall Mode	None	C-Max		None	C-Max							
Act Effct Green (s)	53.4	34.6	48.5	44.7	29.7	59.7	50.3	50.3		71.5	71.5	
Actuated g/C Ratio	0.36	0.23	0.32	0.30	0.20	0.40	0.34	0.34		0.48	0.48	
v/c Ratio	0.78	0.89	0.17	0.67	0.43	0.44	0.23	0.49		0.58	0.76	
Control Delay	51.3	78.0	36.3	47.4	55.5	20.2	40.7	42.7		40.1	37.4	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	51.3	78.0	36.3	47.4	55.5	20.2	40.7	42.7		40.1	37.4	
LOS	D	Ε	D	D	Е	С	D	D		D	D	
Approach Delay		62.5			37.0			42.6			38.0	
Approach LOS		Е			D			D			D	
Queue Length 50th (ft)	242	354	60	98	130	113	23	223		218	538	
Queue Length 95th (ft)	327	#541	104	149	198	156	52	291		323	677	
Internal Link Dist (ft)		1280			1041			978			979	
Turn Bay Length (ft)	225		300	150		450						
Base Capacity (vph)	419	449	576	280	416	599	208	1080		586	1645	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.78	0.85	0.15	0.53	0.37	0.44	0.16	0.49		0.58	0.76	

Intersection Summary

Area Type: Other

Cycle Length: 150

Actuated Cycle Length: 150

Offset: 91 (61%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

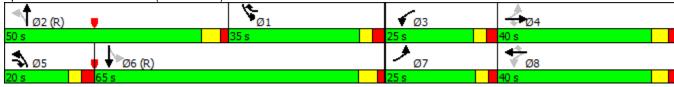
Maximum v/c Ratio: 0.89

Intersection Signal Delay: 44.1 Intersection LOS: D
Intersection Capacity Utilization 83.8% ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lanes, Volumes, Timings 1: Estes Drive Extension & Seawell School Road

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Lane Group	SEL	SER	NEL	NET	SWT	SWR
Actuated g/C Ratio	0.20	0.20	0.63	0.65	0.50	0.78
v/c Ratio	0.18	0.24	0.11	0.29	0.38	0.05
Control Delay	18.7	19.6	4.5	5.4	12.9	3.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.7	19.6	4.5	5.4	12.9	3.3
LOS	В	В	Α	Α	В	Α
Approach Delay	19.2			5.2	11.6	
Approach LOS	В			Α	В	
Queue Length 50th (ft)	14	18	6	37	73	5
Queue Length 95th (ft)	46	55	19	79	155	14
Internal Link Dist (ft)	782			565	745	
Turn Bay Length (ft)	400		250			75
Base Capacity (vph)	874	781	703	1826	1860	1453
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.07	0.10	0.10	0.19	0.19	0.04
Intersection Summary						
Area Type:	Other					
Cycle Length: 100						
Actuated Cycle Length: 43	3					
Natural Cycle: 50						
Control Type: Actuated-Ur	ncoordinated					
Maximum v/c Ratio: 0.38						
Intersection Signal Delay:					tersection	
Intersection Capacity Utiliz	zation 42.1%			IC	U Level o	of Service
Analysis Period (min) 15						

Splits and Phases: 1: Estes Drive Extension & Seawell School Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ኻ	†	7	ች	†	7	ሻ	† 1>		ች	↑ ↑	
Traffic Volume (vph)	164	187	32	141	207	262	32	441	153	220	461	150
Future Volume (vph)	164	187	32	141	207	262	32	441	153	220	461	150
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)	1000	0%	1000	1000	1%	1000	1000	1%	1000	1000	-2%	1000
Storage Length (ft)	225	0,0	300	150	1,0	450	0	.,,	0	0	_,,	0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (ft)	25			25		•	25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor								0.99		1.00		
Frt			0.850			0.850		0.961			0.963	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1736	1827	1553	1727	1818	1545	1694	3230	0	1753	3376	0
Flt Permitted	0.228			0.348			0.322			0.412		
Satd. Flow (perm)	417	1827	1553	633	1818	1545	574	3230	0	757	3376	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		1360			1121			1058			1059	
Travel Time (s)		26.5			21.8			20.6			20.6	
Confl. Peds. (#/hr)									4	4		
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	6%	6%	6%	4%	4%	4%
Adj. Flow (vph)	171	195	33	147	216	273	33	459	159	229	480	156
Shared Lane Traffic (%)												
Lane Group Flow (vph)	171	195	33	147	216	273	33	618	0	229	636	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	•		12			12	Ū		12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.01	1.01	1.01	1.01	1.01	1.01	0.99	0.99	0.99
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA		pm+pt	NA	
Protected Phases	7	4	5	3	8	1	5	2		1	6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	5	3	8	1	5	2		1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	12.0		7.0	12.0	
Minimum Split (s)	12.0	13.0	13.0	13.0	13.0	13.0	13.0	34.0		13.0	18.0	
Total Split (s)	25.0	30.0	25.0	25.0	30.0	25.0	25.0	80.0		25.0	80.0	
Total Split (%)	15.6%	18.8%	15.6%	15.6%	18.8%	15.6%	15.6%	50.0%		15.6%	50.0%	
Yellow Time (s)	3.0	3.8	3.0	3.0	3.8	3.0	3.0	4.3		3.0	4.3	
All-Red Time (s)	1.9	1.7	2.8	2.4	1.7	2.8	2.8	1.7		2.8	1.7	
Lost Time Adjust (s)	0.1	-0.5	-0.8	-0.4	-0.5	-0.8	-0.8	-1.0		-0.8	-1.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lead		Lag	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Recall Mode	None	C-Max		None	C-Max							
Act Effct Green (s)	41.3	24.0	37.4	39.0	22.9	42.9	79.9	79.9		94.0	94.0	
Actuated g/C Ratio	0.26	0.15	0.23	0.24	0.14	0.27	0.50	0.50		0.59	0.59	
v/c Ratio	0.69	0.71	0.09	0.56	0.83	0.66	0.10	0.38		0.40	0.32	
Control Delay	59.3	79.4	47.3	52.5	91.9	43.1	23.2	26.5		25.2	18.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	59.3	79.4	47.3	52.5	91.9	43.1	23.2	26.5		25.2	18.7	
LOS	Е	Ε	D	D	F	D	С	С		С	В	
Approach Delay		68.1			61.9			26.3			20.4	
Approach LOS		Ε			Е			С			С	
Queue Length 50th (ft)	140	193	27	118	219	186	18	215		119	186	
Queue Length 95th (ft)	207	289	58	180	#338	260	40	273		181	242	
Internal Link Dist (ft)		1280			1041			978			979	
Turn Bay Length (ft)	225		300	150		450						
Base Capacity (vph)	277	293	475	304	284	414	426	1612		569	1983	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.62	0.67	0.07	0.48	0.76	0.66	0.08	0.38		0.40	0.32	

Intersection Summary

Area Type: Other

Cycle Length: 160
Actuated Cycle Length: 160

Offset: 104 (65%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 75

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.83 Intersection Signal Delay: 39.7 Intersection Capacity Utilization 72.2%

Intersection LOS: D
ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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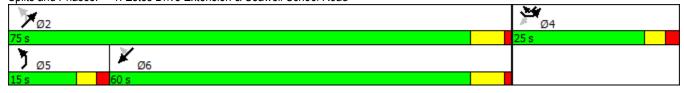
1: Estes Drive Extension & Seawell School Road

	₩.	1	7	×	×	*
Lane Group	SEL	SER	NEL	NET	SWT	SWR
v/c Ratio	0.31	0.59	0.43	0.34	0.77	0.14
Control Delay	30.3	37.4	8.3	6.4	23.6	3.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	30.3	37.4	8.3	6.4	23.6	3.1
LOS	С	D	Α	Α	С	Α
Approach Delay	34.8			7.0	19.6	
Approach LOS	С			Α	В	
Queue Length 50th (ft)	40	73	23	66	229	17
Queue Length 95th (ft)	102	170	53	134	397	30
Internal Link Dist (ft)	782			565	745	
Turn Bay Length (ft)	400		250			75
Base Capacity (vph)	533	477	411	1697	1494	1318
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.20	0.39	0.41	0.24	0.44	0.12
Intersection Summary						
Area Type:	Other					
Cycle Length: 100						
Actuated Cycle Length: 71						
Natural Cycle: 60						
Control Type: Actuated-Ur	ncoordinated					
Maximum v/c Ratio: 0.77						
Intersection Signal Delay:	17.9			In	tersectior	LOS: B

Splits and Phases: 1: Estes Drive Extension & Seawell School Road

Intersection Capacity Utilization 59.6%

Analysis Period (min) 15



ICU Level of Service B

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	<u></u>	7	ř	†	7	ň	↑ ↑		¥	↑ ↑	
Traffic Volume (vph)	273	238	71	156	296	423	81	1081	163	234	659	297
Future Volume (vph)	273	238	71	156	296	423	81	1081	163	234	659	297
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		0%			1%			1%			-2%	
Storage Length (ft)	225		300	150		450	0		0	0		0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor								1.00				
Frt			0.850			0.850		0.980			0.953	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1761	1853	1575	1761	3435	0	1787	3407	0
FIt Permitted	0.116			0.371			0.146			0.102		
Satd. Flow (perm)	216	1863	1583	688	1853	1575	271	3435	0	192	3407	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		1360			1121			1058			1059	
Travel Time (s)		26.5			21.8			20.6			20.6	
Confl. Peds. (#/hr)									5	5		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	287	251	75	164	312	445	85	1138	172	246	694	313
Shared Lane Traffic (%)												
Lane Group Flow (vph)	287	251	75	164	312	445	85	1310	0	246	1007	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	Ţ,		12			12	J		12	J
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.01	1.01	1.01	1.01	1.01	1.01	0.99	0.99	0.99
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA		pm+pt	NA	
Protected Phases	7	4	5	3	8	1	5	2		1	6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	5	3	8	1	5	2		1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	12.0		7.0	12.0	
Minimum Split (s)	12.0	13.0	13.0	13.0	13.0	13.0	13.0	34.0		13.0	18.0	
Total Split (s)	30.0	35.0	25.0	30.0	35.0	25.0	25.0	90.0		25.0	90.0	
Total Split (%)	16.7%	19.4%	13.9%	16.7%	19.4%	13.9%	13.9%	50.0%		13.9%	50.0%	
Yellow Time (s)	3.0	3.8	3.0	3.0	3.8	3.0	3.0	4.3		3.0	4.3	
All-Red Time (s)	1.9	1.7	2.8	2.4	1.7	2.8	2.8	1.7		2.8	1.7	
Lost Time Adjust (s)	0.1	-0.5	-0.8	-0.4	-0.5	-0.8	-0.8	-1.0		-0.8	-1.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lead		Lag	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Act Effct Green (s)	59.0	36.7	52.8	48.3	30.0	50.0	85.0	85.0		93.9	93.9	
Actuated g/C Ratio	0.33	0.20	0.29	0.27	0.17	0.28	0.47	0.47		0.52	0.52	
v/c Ratio	1.00	0.66	0.16	0.56	1.01	1.02	0.39	0.81		0.89	0.57	
Control Delay	108.3	75.9	49.1	52.5	126.3	96.4	31.7	45.4		94.8	31.1	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	108.3	75.9	49.1	52.5	126.3	96.4	31.7	45.4		94.8	31.1	
LOS	F	Е	D	D	F	F	С	D		F	С	
Approach Delay		87.8			98.7			44.6			43.6	
Approach LOS		F			F			D			D	
Queue Length 50th (ft)	292	276	66	143	~380	~396	55	688		191	418	
Queue Length 95th (ft)	#519	#423	117	212	#596	#691	93	785		#367	505	
Internal Link Dist (ft)		1280			1041			978			979	
Turn Bay Length (ft)	225		300	150		450						
Base Capacity (vph)	286	379	542	359	308	437	293	1622		277	1776	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	1.00	0.66	0.14	0.46	1.01	1.02	0.29	0.81		0.89	0.57	

Intersection Summary

Area Type: Other

Cycle Length: 180

Actuated Cycle Length: 180

Offset: 88 (49%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.02 Intersection Signal Delay: 62.5 Intersection Capacity Utilization 95.5%

Intersection LOS: E
ICU Level of Service F

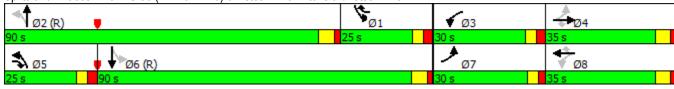
Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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1: Estes Drive Extension & Seawell School Road

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Lane Group	SEL	SER	NEL	NET	SWT	SWR
Actuated g/C Ratio	0.23	0.23	0.58	0.58	0.31	0.64
v/c Ratio	0.51	0.56	0.52	0.66	0.51	0.10
Control Delay	24.6	26.4	9.8	12.1	19.6	3.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	24.6	26.4	9.8	12.1	19.6	3.8
LOS	С	С	Α	В	В	Α
Approach Delay	25.5			11.4	15.6	
Approach LOS	С			В	В	
Queue Length 50th (ft)	58	57	42	131	75	10
Queue Length 95th (ft)	138	138	104	305	157	21
Internal Link Dist (ft)	782			565	745	
Turn Bay Length (ft)	400		250			75
Base Capacity (vph)	654	585	599	1809	1703	1196
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.32	0.35	0.51	0.39	0.17	0.08
Intersection Summary						
Area Type:	Other					
Cycle Length: 100						
Actuated Cycle Length: 55	5.9					
Natural Cycle: 50						
Control Type: Actuated-U	ncoordinated					
Maximum v/c Ratio: 0.66						
Intersection Signal Delay:					tersection	
Intersection Capacity Utili	zation 52.8%			IC	U Level o	of Service
Analysis Period (min) 15						

Splits and Phases: 1: Estes Drive Extension & Seawell School Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	1	7	1,1	1	7	ሻ	^	7	ሻ	^	7
Traffic Volume (vph)	316	369	85	148	148	254	38	418	117	330	1021	197
Future Volume (vph)	316	369	85	148	148	254	38	418	117	330	1021	197
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		0%			1%			1%			-2%	
Storage Length (ft)	225		225	300		250	0		225	0		150
Storage Lanes	1		1	2		1	1		1	1		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Ped Bike Factor	1.00					0.99			0.96	0.99		
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	3287	1783	1516	1678	3357	1502	1770	3540	1584
FIt Permitted	0.950			0.950			0.162			0.365		
Satd. Flow (perm)	1768	1863	1583	3287	1783	1496	286	3357	1444	675	3540	1584
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		1360			1121			1058			1059	
Travel Time (s)		26.5			21.8			20.6			20.6	
Confl. Peds. (#/hr)	1					1			7	7		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	2%	2%	6%	6%	6%	7%	7%	7%	3%	3%	3%
Adj. Flow (vph)	340	397	91	159	159	273	41	449	126	355	1098	212
Shared Lane Traffic (%)												
Lane Group Flow (vph)	340	397	91	159	159	273	41	449	126	355	1098	212
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24	•		12	•		12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.01	1.01	1.01	1.01	1.01	1.01	0.99	0.99	0.99
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov
Protected Phases	7	4	5	3	8	1	5	2	3	1	6	7
Permitted Phases			4			8	2		2	6		6
Detector Phase	7	4	5	3	8	1	5	2	3	1	6	7
Switch Phase												
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	12.0	7.0	7.0	12.0	7.0
Minimum Split (s)	14.0	14.0	15.0	15.0	14.0	15.0	15.0	35.0	15.0	15.0	19.0	14.0
Total Split (s)	45.0	54.0	15.0	17.0	26.0	34.0	15.0	45.0	17.0	34.0	64.0	45.0
Total Split (%)	30.0%	36.0%	10.0%	11.3%	17.3%	22.7%	10.0%	30.0%	11.3%	22.7%	42.7%	30.0%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lag	Lead	Lead	Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Recall Mode	None	C-Max	None	None	C-Max	None						
Act Effct Green (s)	34.8	39.4	48.8	14.3	18.9	43.1	61.6	52.1	66.4	81.3	66.8	101.6
Actuated g/C Ratio	0.23	0.26	0.33	0.10	0.13	0.29	0.41	0.35	0.44	0.54	0.45	0.68
v/c Ratio	0.83	0.81	0.18	0.51	0.71	0.63	0.20	0.39	0.20	0.65	0.70	0.20
Control Delay	71.7	64.9	22.3	70.4	80.1	30.5	23.2	40.8	15.7	27.5	37.8	5.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	71.7	64.9	22.3	70.4	80.1	30.5	23.2	40.8	15.7	27.5	37.8	5.4
LOS	Е	Е	С	Е	F	С	С	D	В	С	D	Α
Approach Delay		63.0			54.6			34.5			31.5	
Approach LOS		Ε			D			С			С	
Queue Length 50th (ft)	316	366	46	75	150	121	19	181	42	202	463	42
Queue Length 95th (ft)	423	453	73	119	231	147	43	254	82	298	584	63
Internal Link Dist (ft)		1280			1041			978			979	
Turn Bay Length (ft)	225		225	300		250			225			150
Base Capacity (vph)	472	608	521	318	249	482	211	1166	647	577	1576	1128
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.72	0.65	0.17	0.50	0.64	0.57	0.19	0.39	0.19	0.62	0.70	0.19

Intersection Summary

Area Type: Other

Cycle Length: 150

Actuated Cycle Length: 150

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

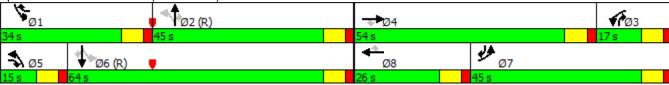
Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.83

Intersection Signal Delay: 42.7 Intersection LOS: D
Intersection Capacity Utilization 83.6% ICU Level of Service E

Analysis Period (min) 15



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1: Estes Drive Extension & Seawell School Road

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Lane Group	SEL	SER	NEL	NET	SWT	SWR
Actuated g/C Ratio	0.20	0.20	0.63	0.66	0.51	0.79
v/c Ratio	0.19	0.25	0.12	0.31	0.39	0.05
Control Delay	19.3	20.3	4.5	5.4	12.9	3.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	19.3	20.3	4.5	5.4	12.9	3.2
LOS	В	С	Α	Α	В	Α
Approach Delay	19.9			5.3	11.6	
Approach LOS	В			Α	В	
Queue Length 50th (ft)	16	19	7	39	78	5
Queue Length 95th (ft)	50	58	20	85	164	15
Internal Link Dist (ft)	782			565	745	
Turn Bay Length (ft)	400		250			75
Base Capacity (vph)	861	770	695	1826	1845	1451
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.08	0.11	0.11	0.20	0.20	0.04
Intersection Summary						
Area Type:	Other					
Cycle Length: 100						
Actuated Cycle Length: 43	3.9					
Natural Cycle: 50						
Control Type: Actuated-U	ncoordinated					
Maximum v/c Ratio: 0.39						
Intersection Signal Delay:					tersection	
Intersection Capacity Utiliz	zation 43.0%			IC	U Level o	of Service
Analysis Period (min) 15						

Splits and Phases: 1: Estes Drive Extension & Seawell School Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ች	†	7	ሻሻ	†	7	ሻ	^	#	ች	^	7
Traffic Volume (vph)	171	195	35	153	216	274	36	467	167	232	486	156
Future Volume (vph)	171	195	35	153	216	274	36	467	167	232	486	156
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		0%			1%			1%			-2%	
Storage Length (ft)	225		225	300		250	0		225	0		150
Storage Lanes	1		1	2		1	1		1	1		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Ped Bike Factor									0.97	0.99		
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1736	1827	1553	3350	1818	1545	1694	3389	1516	1753	3506	1568
Flt Permitted	0.950			0.950			0.335			0.475		
Satd. Flow (perm)	1736	1827	1553	3350	1818	1545	597	3389	1469	871	3506	1568
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		1360			1121			1058			1059	
Travel Time (s)		26.5			21.8			20.6			20.6	
Confl. Peds. (#/hr)									4	4		
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	6%	6%	6%	4%	4%	4%
Adj. Flow (vph)	178	203	36	159	225	285	38	486	174	242	506	163
Shared Lane Traffic (%)												
Lane Group Flow (vph)	178	203	36	159	225	285	38	486	174	242	506	163
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.01	1.01	1.01	1.01	1.01	1.01	0.99	0.99	0.99
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov
Protected Phases	7	4	5	3	8	1	5	2	3	1	6	7
Permitted Phases			4			8	2		2	6		6
Detector Phase	7	4	5	3	8	1	5	2	3	1	6	7
Switch Phase												
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	12.0	7.0	7.0	12.0	7.0
Minimum Split (s)	14.0	14.0	15.0	15.0	14.0	15.0	15.0	35.0	15.0	15.0	19.0	14.0
Total Split (s)	37.0	59.0	17.0	20.0	42.0	35.0	17.0	46.0	20.0	35.0	64.0	37.0
Total Split (%)	23.1%	36.9%	10.6%	12.5%	26.3%	21.9%	10.6%	28.8%	12.5%	21.9%	40.0%	23.1%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lead	Lead	Lag	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Recall Mode	None	C-Max	None	None	C-Max	None						
Act Effct Green (s)	23.6	36.7	52.0	13.9	27.0	57.0	59.3	59.3	73.3	81.8	81.8	110.5
Actuated g/C Ratio	0.15	0.23	0.32	0.09	0.17	0.36	0.37	0.37	0.46	0.51	0.51	0.69
v/c Ratio	0.70	0.48	0.07	0.55	0.73	0.52	0.13	0.39	0.26	0.40	0.28	0.15
Control Delay	78.5	55.7	33.3	77.1	76.7	25.8	38.5	40.2	15.9	33.6	25.7	10.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	78.5	55.7	33.3	77.1	76.7	25.8	38.5	40.2	15.9	33.6	25.7	10.7
LOS	Е	Ε	С	Е	Ε	С	D	D	В	С	С	В
Approach Delay		63.5			55.1			34.0			25.1	
Approach LOS		Ε			Ε			С			С	
Queue Length 50th (ft)	179	187	26	82	226	154	26	196	54	146	161	58
Queue Length 95th (ft)	254	240	47	123	307	166	61	287	113	258	251	110
Internal Link Dist (ft)		1280			1041			978			979	
Turn Bay Length (ft)	225		225	300		250			225			150
Base Capacity (vph)	347	616	522	314	420	550	304	1256	686	610	1792	1164
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.51	0.33	0.07	0.51	0.54	0.52	0.13	0.39	0.25	0.40	0.28	0.14

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 160

Offset: 104 (65%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

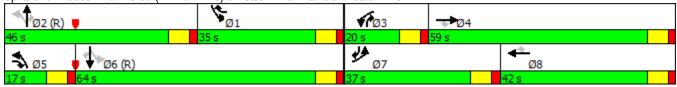
Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.73

Intersection Signal Delay: 40.8 Intersection LOS: D
Intersection Capacity Utilization 73.7% ICU Level of Service D

Analysis Period (min) 15



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1: Estes Drive Extension & Seawell School Road

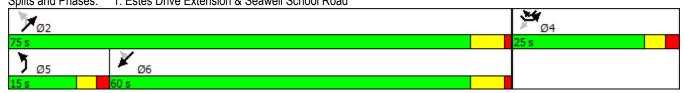
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Lane Group	SEL	SER	NEL	NET	SWT	SWR
v/c Ratio	0.33	0.62	0.46	0.36	0.79	0.14
Control Delay	31.7	39.5	9.2	6.5	24.3	3.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	31.7	39.5	9.2	6.5	24.3	3.0
LOS	С	D	Α	Α	С	Α
Approach Delay	36.6			7.3	20.2	
Approach LOS	D			Α	С	
Queue Length 50th (ft)	44	79	24	73	249	18
Queue Length 95th (ft)	112	183	56	141	421	31
Internal Link Dist (ft)	782			565	745	
Turn Bay Length (ft)	400		250			75
Base Capacity (vph)	515	461	391	1664	1454	1314
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.22	0.42	0.44	0.26	0.47	0.13
Intersection Summary						
Area Type:	Other					
Cycle Length: 100						
Actuated Cycle Length: 73	3.5					
Natural Cycle: 60						
Control Type: Actuated-U	ncoordinated					
Maximum v/c Ratio: 0.79						

Splits and Phases: 1: Estes Drive Extension & Seawell School Road

Intersection Signal Delay: 18.6

Analysis Period (min) 15

Intersection Capacity Utilization 61.5%



Intersection LOS: B

ICU Level of Service B

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ		7	44	1	7	ሻ	^	7	ሻ	^	7
Traffic Volume (vph)	284	249	79	173	309	444	87	1132	180	247	700	309
Future Volume (vph)	284	249	79	173	309	444	87	1132	180	247	700	309
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		0%			1%			1%			-2%	
Storage Length (ft)	225		225	300		250	0		225	0		150
Storage Lanes	1		1	2		1	1		1	1		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Ped Bike Factor									0.96			
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	3416	1853	1575	1761	3522	1575	1787	3575	1599
Flt Permitted	0.950			0.950			0.198			0.071		
Satd. Flow (perm)	1770	1863	1583	3416	1853	1575	367	3522	1519	134	3575	1599
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		1360			1121			1058			1059	
Travel Time (s)		26.5			21.8			20.6			20.6	
Confl. Peds. (#/hr)		_0.0						_0.0	5	5	_0.0	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	299	262	83	182	325	467	92	1192	189	260	737	325
Shared Lane Traffic (%)					V-V		<u> </u>					0_0
Lane Group Flow (vph)	299	262	83	182	325	467	92	1192	189	260	737	325
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								. •				
Headway Factor	1.00	1.00	1.00	1.01	1.01	1.01	1.01	1.01	1.01	0.99	0.99	0.99
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	pm+pt	NA		pm+pt	NA	pm+ov
Protected Phases	7	4	5	3	8	1	5	2	3	1	6	7
Permitted Phases			4			8	2		2	6		6
Detector Phase	7	4	5	3	8	1	5	2	3	1	6	7
Switch Phase		•				•		_				·
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	12.0	7.0	7.0	12.0	7.0
Minimum Split (s)	14.0	14.0	15.0	15.0	14.0	15.0	15.0	35.0	15.0	15.0	19.0	14.0
Total Split (s)	38.0	57.0	15.0	20.0	39.0	32.0	15.0	71.0	20.0	32.0	88.0	38.0
Total Split (%)	21.1%	31.7%	8.3%	11.1%	21.7%	17.8%	8.3%	39.4%	11.1%	17.8%	48.9%	21.1%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lead	Lead	Lag	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	C-Max	None	None	C-Max	None
TOGAII WICKE	INOHE	INOHE	INOLIG	INOHE	INUITE	INOTIE	INOITE	O-IVIAX	INOHE	INOHE	O-IVIAX	INOTIE

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Act Effct Green (s)	32.6	51.6	66.6	14.6	33.6	60.6	66.8	66.8	81.4	83.9	83.9	121.4
Actuated g/C Ratio	0.18	0.29	0.37	0.08	0.19	0.34	0.37	0.37	0.45	0.47	0.47	0.67
v/c Ratio	0.93	0.49	0.14	0.66	0.94	0.88	0.43	0.91	0.27	0.84	0.44	0.30
Control Delay	107.6	56.9	38.4	92.2	106.1	54.3	44.5	65.1	17.6	90.5	33.6	12.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	107.6	56.9	38.4	92.2	106.1	54.3	44.5	65.1	17.6	90.5	33.6	12.9
LOS	F	Е	D	F	F	D	D	Е	В	F	С	В
Approach Delay		78.1			78.7			57.7			39.7	
Approach LOS		Е			Е			Е			D	
Queue Length 50th (ft)	352	258	65	109	383	323	73	711	84	250	308	150
Queue Length 95th (ft)	#540	356	110	155	#580	#478	120	#824	122	#404	367	204
Internal Link Dist (ft)		1280			1041			978			979	
Turn Bay Length (ft)	225		225	300		250			225			150
Base Capacity (vph)	324	538	585	284	350	530	214	1307	695	310	1665	1082
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.92	0.49	0.14	0.64	0.93	0.88	0.43	0.91	0.27	0.84	0.44	0.30

Intersection Summary

Area Type: Other

Cycle Length: 180

Actuated Cycle Length: 180

Offset: 88 (49%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 100

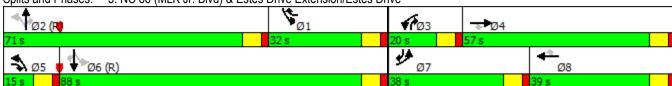
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.94 Intersection Signal Delay: 59.9 Intersection Capacity Utilization 93.6%

Intersection LOS: E
ICU Level of Service F

Analysis Period (min) 15

Queue shown is maximum after two cycles.



^{# 95}th percentile volume exceeds capacity, queue may be longer.

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Recall Mode	None	C-Max	None	None	C-Max	None						
Act Effct Green (s)	29.4	38.5	48.5	13.3	22.4	45.9	64.6	54.6	67.9	83.2	68.2	97.6
Actuated g/C Ratio	0.20	0.26	0.32	0.09	0.15	0.31	0.43	0.36	0.45	0.55	0.45	0.65
v/c Ratio	0.51	0.83	0.19	0.55	0.68	0.59	0.38	0.37	0.19	0.64	0.68	0.24
Control Delay	56.6	68.0	23.3	72.8	72.6	28.1	24.7	38.3	14.9	25.7	36.2	6.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	56.6	68.0	23.3	72.8	72.6	28.1	24.7	38.3	14.9	25.7	36.2	6.9
LOS	Е	Е	С	Е	Е	С	С	D	В	С	D	Α
Approach Delay		58.2			52.8			32.1			29.8	
Approach LOS		Ε			D			С			С	
Queue Length 50th (ft)	153	369	50	77	169	128	38	175	42	194	459	53
Queue Length 95th (ft)	210	475	80	118	241	160	71	244	76	286	564	99
Internal Link Dist (ft)		1280			1041			978			979	
Turn Bay Length (ft)	300		225	300		250			225			150
Base Capacity (vph)	673	558	514	300	404	506	219	1222	663	584	1609	1031
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.51	0.72	0.19	0.53	0.45	0.54	0.37	0.37	0.19	0.61	0.68	0.24

Intersection Summary

Area Type: Other

Cycle Length: 150

Actuated Cycle Length: 150

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

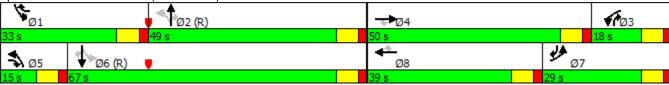
Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.83

Intersection Signal Delay: 40.1 Intersection LOS: D
Intersection Capacity Utilization 83.6% ICU Level of Service E

Analysis Period (min) 15



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Recall Mode	None	C-Max	None	None	C-Max	None						
Act Effct Green (s)	16.8	30.5	47.0	14.6	28.4	53.4	69.8	69.8	84.5	83.4	83.4	105.2
Actuated g/C Ratio	0.10	0.19	0.29	0.09	0.18	0.33	0.44	0.44	0.53	0.52	0.52	0.66
v/c Ratio	0.57	0.62	0.14	0.52	0.74	0.55	0.18	0.33	0.22	0.41	0.28	0.18
Control Delay	74.2	66.6	39.6	75.3	75.9	30.8	30.7	31.9	11.8	31.4	23.6	12.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	74.2	66.6	39.6	75.3	75.9	30.8	30.7	31.9	11.8	31.4	23.6	12.4
LOS	Е	Е	D	Е	Е	С	С	С	В	С	С	В
Approach Delay		66.2			57.0			27.0			23.4	
Approach LOS		Ε			Ε			С			С	
Queue Length 50th (ft)	104	212	49	82	240	179	40	174	47	137	151	71
Queue Length 95th (ft)	144	282	80	121	322	212	83	252	92	240	234	132
Internal Link Dist (ft)		1280			1041			978			979	
Turn Bay Length (ft)	300		225	300		250			225			150
Base Capacity (vph)	441	525	490	355	477	515	379	1479	801	591	1827	1072
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.45	0.41	0.13	0.45	0.50	0.55	0.17	0.33	0.22	0.41	0.28	0.17

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 160

Offset: 104 (65%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

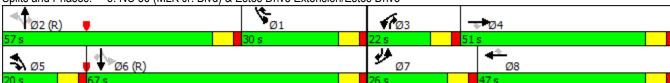
Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.74

Intersection Signal Delay: 39.7 Intersection LOS: D
Intersection Capacity Utilization 70.7% ICU Level of Service C

Analysis Period (min) 15



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Act Effct Green (s)	20.8	42.9	58.4	14.6	36.7	66.7	72.5	72.5	87.1	92.0	92.0	117.8
Actuated g/C Ratio	0.12	0.24	0.32	80.0	0.20	0.37	0.40	0.40	0.48	0.51	0.51	0.65
v/c Ratio	0.85	0.65	0.26	0.66	0.88	0.80	0.42	0.84	0.26	0.72	0.40	0.32
Control Delay	98.0	68.8	45.9	92.2	93.3	45.5	40.4	55.5	15.2	73.5	28.3	14.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	98.0	68.8	45.9	92.2	93.3	45.5	40.4	55.5	15.2	73.5	28.3	14.9
LOS	F	Е	D	F	F	D	D	Е	В	Е	С	В
Approach Delay		77.9			70.3			49.4			33.7	
Approach LOS		Е			Е			D			С	
Queue Length 50th (ft)	206	303	113	109	379	341	79	685	73	217	285	171
Queue Length 95th (ft)	#285	412	175	155	#534	451	127	784	107	329	338	232
Internal Link Dist (ft)		1280			1041			978			979	
Turn Bay Length (ft)	225		225	300		250			225			150
Base Capacity (vph)	400	465	513	284	401	583	248	1418	742	359	1827	1048
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.85	0.62	0.26	0.64	0.83	0.80	0.42	0.84	0.25	0.72	0.40	0.32

Intersection Summary

Area Type: Other

Cycle Length: 180

Actuated Cycle Length: 180

Offset: 88 (49%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

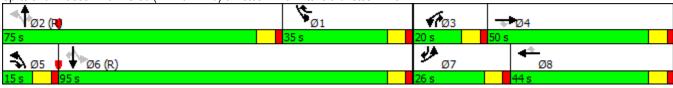
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.88 Intersection Signal Delay: 54.0 Intersection Capacity Utilization 87.4%

Intersection LOS: D
ICU Level of Service E

Analysis Period (min) 15

Queue shown is maximum after two cycles.



^{# 95}th percentile volume exceeds capacity, queue may be longer.

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Lanes, Volumes, Timings 1: Estes Drive Extension & Seawell School Road

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Lane Group	SEL	SER	NEL	NET	SWT	SWR
Actuated g/C Ratio	0.23	0.23	0.59	0.59	0.32	0.64
v/c Ratio	0.53	0.56	0.52	0.67	0.51	0.10
Control Delay	25.1	26.6	9.7	12.2	19.5	3.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	25.1	26.6	9.7	12.2	19.5	3.8
LOS	С	С	Α	В	В	Α
Approach Delay	25.8			11.4	15.5	
Approach LOS	С			В	В	
Queue Length 50th (ft)	60	57	42	132	76	10
Queue Length 95th (ft)	144	140	104	308	157	21
Internal Link Dist (ft)	782			565	745	
Turn Bay Length (ft)	400		250			75
Base Capacity (vph)	653	584	599	1808	1699	1198
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.33	0.35	0.51	0.39	0.17	0.08
Intersection Summary						
Area Type:	Other					
Cycle Length: 100						
Actuated Cycle Length: 56	5					
Natural Cycle: 50						
Control Type: Actuated-Ur	ncoordinated					
Maximum v/c Ratio: 0.67						
Intersection Signal Delay:				ln	tersectior	LOS: B
Intersection Capacity Utiliz	zation 53.5%			IC	U Level o	of Service
Analysis Period (min) 15						

Splits and Phases: 1: Estes Drive Extension & Seawell School Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	1/1	1	7	ሻ	^	7	ሻ	^	7
Traffic Volume (vph)	320	371	90	148	167	254	76	418	117	330	1021	229
Future Volume (vph)	320	371	90	148	167	254	76	418	117	330	1021	229
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		0%			1%			1%			-2%	
Storage Length (ft)	225		225	300		250	0		225	0		150
Storage Lanes	1		1	2		1	1		1	1		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Ped Bike Factor	1.00					0.99			0.96	0.99		
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	3287	1783	1516	1678	3357	1502	1770	3540	1584
Flt Permitted	0.950			0.950			0.154			0.361		
Satd. Flow (perm)	1768	1863	1583	3287	1783	1496	272	3357	1444	668	3540	1584
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		1360			1121			1058			1059	
Travel Time (s)		26.5			21.8			20.6			20.6	
Confl. Peds. (#/hr)	1					1			7	7		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	2%	2%	6%	6%	6%	7%	7%	7%	3%	3%	3%
Adj. Flow (vph)	344	399	97	159	180	273	82	449	126	355	1098	246
Shared Lane Traffic (%)											,,,,,	
Lane Group Flow (vph)	344	399	97	159	180	273	82	449	126	355	1098	246
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.01	1.01	1.01	1.01	1.01	1.01	0.99	0.99	0.99
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov
Protected Phases	7	4	5	3	8	1	5	2	3	1	6	7
Permitted Phases	•	•	4			8	2	_	2	6		6
Detector Phase	7	4	5	3	8	1	5	2	3	1	6	7
Switch Phase	•	•	J			•		_		•		•
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	12.0	7.0	7.0	12.0	7.0
Minimum Split (s)	14.0	14.0	15.0	15.0	14.0	15.0	15.0	35.0	15.0	15.0	19.0	14.0
Total Split (s)	45.0	54.0	15.0	17.0	26.0	34.0	15.0	45.0	17.0	34.0	64.0	45.0
Total Split (%)	30.0%	36.0%	10.0%	11.3%	17.3%	22.7%	10.0%	30.0%	11.3%	22.7%	42.7%	30.0%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lag	Lead	Lead	Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Leau-Lay Optimize?	168	1 68	168	168	168	168	1 65	168	168	168	168	168

	•	-	•	•	•	•	1	†	~	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Recall Mode	None	C-Max	None	None	C-Max	None						
Act Effct Green (s)	35.1	39.5	49.6	15.2	19.7	44.0	61.0	51.0	66.2	80.2	65.2	100.3
Actuated g/C Ratio	0.23	0.26	0.33	0.10	0.13	0.29	0.41	0.34	0.44	0.53	0.43	0.67
v/c Ratio	0.83	0.81	0.19	0.48	0.77	0.62	0.40	0.39	0.20	0.66	0.71	0.23
Control Delay	71.7	64.8	21.9	68.9	84.4	29.6	27.4	41.5	15.8	28.2	39.2	5.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	71.7	64.8	21.9	68.9	84.4	29.6	27.4	41.5	15.8	28.2	39.2	5.8
LOS	Е	Е	С	Е	F	С	С	D	В	С	D	Α
Approach Delay		62.7			55.9			34.8			32.1	
Approach LOS		Е			Ε			С			С	
Queue Length 50th (ft)	320	368	47	75	171	118	40	183	42	205	480	51
Queue Length 95th (ft)	428	456	77	119	#272	147	74	254	82	298	584	74
Internal Link Dist (ft)		1280			1041			978			979	
Turn Bay Length (ft)	225		225	300		250			225			150
Base Capacity (vph)	472	608	525	333	249	489	207	1140	643	570	1539	1111
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.73	0.66	0.18	0.48	0.72	0.56	0.40	0.39	0.20	0.62	0.71	0.22

Intersection Summary

Area Type: Other

Cycle Length: 150

Actuated Cycle Length: 150

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

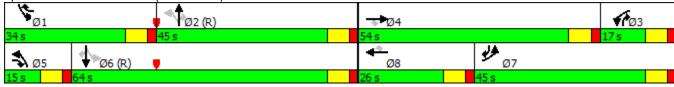
Maximum v/c Ratio: 0.83

Intersection Signal Delay: 43.1 Intersection LOS: D
Intersection Capacity Utilization 84.8% ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lanes, Volumes, Timings 1: Estes Drive Extension & Seawell School Road

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Lane Group	SEL	SER	NEL	NET	SWT	SWR
Actuated g/C Ratio	0.20	0.20	0.63	0.66	0.51	0.79
v/c Ratio	0.20	0.25	0.12	0.31	0.40	0.05
Control Delay	19.5	20.4	4.5	5.4	13.0	3.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	19.5	20.4	4.5	5.4	13.0	3.2
LOS	В	С	Α	Α	В	Α
Approach Delay	20.0			5.3	11.5	
Approach LOS	В			Α	В	
Queue Length 50th (ft)	17	19	7	40	80	5
Queue Length 95th (ft)	53	59	20	85	166	16
Internal Link Dist (ft)	782			565	745	
Turn Bay Length (ft)	400		250			75
Base Capacity (vph)	858	768	693	1826	1844	1450
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.08	0.11	0.11	0.20	0.20	0.05
Intersection Summary						
Area Type:	Other					
Cycle Length: 100						
Actuated Cycle Length: 44	1					
Natural Cycle: 50						
Control Type: Actuated-Ur	ncoordinated					
Maximum v/c Ratio: 0.40						
Intersection Signal Delay:					tersectior	
Intersection Capacity Utiliz	zation 43.2%			IC	U Level o	of Service
Analysis Period (min) 15						

Splits and Phases: 1: Estes Drive Extension & Seawell School Road



Lane Configurations T		۶	-	•	•	←	•	4	†	~	>	ţ	4
Traffic Volume (vph) 192 208 60 153 229 274 62 467 167 232 486 178 178 178 180 180 190	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph) 192 208 60 153 229 274 62 467 167 232 486 178 178 178 180 180 190	Lane Configurations	ሻ	^	7	14.54		7	7	^	7	ሻ	^	7
Future Volume (volph)				60						167			
Ideal Flow (yphphi) 1900		192	208	60	153	229	274	62	467	167	232	486	
Grade (%) Storage Length (ft) Storage Length (ft) Storage Lanes 1							1900	1900					
Storage Length (ft) 225 225 300 250 0 225 0 150													
Storage Lanes	. ,	225		225	300		250	0		225	0		150
Taper Length (ff)	• • • • • • • • • • • • • • • • • • • •	1		1	2		1	1		1	1		1
Lane Util. Factor		25			25			25			25		
Fith		1.00	1.00	1.00	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Fit Protected 0,950 1736 1827 1553 3350 1818 1545 1694 3389 1516 1753 3506 1568 Fit Permitted 0,950 0,950 0,950 0,960 0,961 0,975 0,97	Ped Bike Factor									0.97	0.99		
Fit Protected 0,950 1736 1827 1553 3350 1818 1545 1694 3389 1516 1753 3506 1568 Fit Permitted 0,950 0,950 0,950 0,960 0,961 0,975 0,97				0.850			0.850						0.850
Satd. Flow (prot) 1736 1827 1553 3350 1818 1545 1694 3389 1516 1753 3506 1568 1516 1750 0.475		0.950			0.950			0.950			0.950		
Fit Permitted	Satd. Flow (prot)		1827	1553		1818	1545		3389	1516		3506	1568
Satd. Flow (perm) 1736 1827 1553 3350 1818 1545 558 3389 1469 871 3506 1568 Right Turn on Red Satd. Flow (RTOR) Sa								0.313					
Right Turn on Red Satd. Flow (RTOR) Satd			1827	1553		1818	1545		3389	1469		3506	1568
Satid. Flow (RTOR)													
Link Speed (mph)	· ·												
Link Distance (ft)			35			35			35			35	
Travel Time (s)													
Confil Peds. (#hr)	· /												
Peak Hour Factor	` ,									4	4		
Heavy Vehicles (%)	` ,	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96			0.96	0.96
Adj. Flow (vph) 200 217 63 159 239 285 65 486 174 242 506 185 Shared Lane Traffic (%) Lane Group Flow (vph) 200 217 63 159 239 285 65 486 174 242 506 185 Enter Blocked Intersection No													
Shared Lane Traffic (%) Lane Group Flow (vph) 200 217 63 159 239 285 65 486 174 242 506 185 Enter Blocked Intersection No No No No No No No													
Lane Group Flow (vph) 200 217 63 159 239 285 65 486 174 242 506 185 Enter Blocked Intersection No No No No No No No													
Enter Blocked Intersection No No No No No No No		200	217	63	159	239	285	65	486	174	242	506	185
Left Alignment Left Left Right Left Left Right Left Left Right Left Left Right Left Right Left Left Right Left Right Left Left	,												
Median Width(ft)													
Link Offset(ft) 0 0 0 0 0 0 0 16 14 16 17 17 17 10 10 11 10 <				J			J			J .			3
Crosswalk Width(fft) 16 16 16 16 16 16 16 16 16 Two way Left Turn Lane 100 1.00 1.00 1.01 1.01 1.01 1.01 1.01 1.01 1.01 0.99 15 9 15 9 0.99 0.99 0.99 0.99 15 9 15 9 15 9 15 9 15 9 <													
Two way Left Turn Lane Headway Factor 1.00 1.00 1.00 1.01 1.01 1.01 1.01 1.01 1.01 1.01 0.99 15 9 15 9 15 9 15 9 15 9 15 9 15 9 15 9 15 9 15 9 15 9 15 </td <td></td>													
Headway Factor 1.00 1.00 1.00 1.01 1.01 1.01 1.01 1.01 1.01 1.01 0.99 0.90 0.99 0.90 0.99 0.90 0.99 0.99 0.99 0.99 0.99 0.99 0.99 0.90 0.99 0.99 0.99 0.99 0.99 0.99 0.99 0.99 0.99 0.99 0.90 0.99	. ,												
Turning Speed (mph) 15 9 15 9 15 9 15 9 Turn Type Prot NA pm+ov Prot NA pm+ov pm+pt NA pm+ov pm+ov pm+pt NA pm+ov	•	1.00	1.00	1.00	1.01	1.01	1.01	1.01	1.01	1.01	0.99	0.99	0.99
Turn Type Prot NA pm+ov Prot NA pm+pt NA <	-												
Protected Phases 7 4 5 3 8 1 5 2 3 1 6 7 Permitted Phases 4 8 2 2 6 6 6 Detector Phase 7 4 5 3 8 1 5 2 3 1 6 7 Switch Phase Minimum Initial (s) 7.0 7.0 7.0 7.0 7.0 7.0 7.0 12.0 7.0 7.0 12.0 7.0 Minimum Split (s) 14.0 14.0 15.0 15.0 14.0 15.0 15.0 35.0 15.0 15.0 15.0 19.0 14.0 Total Split (s) 37.0 59.0 17.0 20.0 42.0 35.0 17.0 46.0 20.0 35.0 64.0 37.0 Total Split (%) 23.1% 36.9% 10.6% 12.5% 26.3% 21.9% 10.6% 28.8% 12.5% 21.9% 40.0% 23.1% Yellow Time (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0			NA	pm+ov		NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov
Permitted Phases 4 8 2 2 6 6 Detector Phase 7 4 5 3 8 1 5 2 3 1 6 7 Switch Phase Minimum Initial (s) 7.0 7.0 7.0 7.0 7.0 7.0 7.0 7.0 7.0 7.0 12.0 7.0 7.0 12.0 7.0 7.0 12.0 7.0 7.0 12.0 7.0 7.0 12.0 7.0 7.0 12.0 7.0 7.0 12.0 7.0 7.0 12.0 7.0 7.0 12.0 7.0 12.0 7.0 12.0 7.0 12.0 7.0 12.0 7.0 12.0 12.0 7.0 12.0 12.0 12.0 12.0 12.0 15.0 15.0 15.0 15.0 15.0 15.0 14.0 14.0 14.0 15.0 15.0 15.0 15.0 14.0 14.0 15.0 17.0 46.0				•						•			
Detector Phase 7 4 5 3 8 1 5 2 3 1 6 7 Switch Phase Minimum Initial (s) 7.0 7.0 7.0 7.0 7.0 7.0 7.0 7.0 7.0 7.0 7.0 12.0 7.0 7.0 12.0 7.0 12.0 7.0 12.0 7.0 12.0 7.0 12.0 7.0 12.0 7.0 12.0 7.0 12.0 7.0 12.0 7.0 12.0 7.0 12.0 7.0 12.0 7.0 12.0 7.0 12.0 7.0 12.0 12.0 7.0 12.0 1											6		
Switch Phase Minimum Initial (s) 7.0 7		7	4		3	8			2	3		6	
Minimum Initial (s) 7.0 10.0 7.0 10													
Minimum Split (s) 14.0 14.0 15.0 14.0 15.0 15.0 15.0 15.0 15.0 15.0 15.0 15.0 15.0 15.0 15.0 15.0 15.0 19.0 14.0 Total Split (s) 37.0 59.0 17.0 20.0 42.0 35.0 17.0 46.0 20.0 35.0 64.0 37.0 Total Split (%) 23.1% 36.9% 10.6% 12.5% 21.9% 10.6% 28.8% 12.5% 21.9% 40.0% 23.1% Yellow Time (s) 5.0		7.0	7.0	7.0	7.0	7.0	7.0	7.0	12.0	7.0	7.0	12.0	7.0
Total Split (s) 37.0 59.0 17.0 20.0 42.0 35.0 17.0 46.0 20.0 35.0 64.0 37.0 Total Split (%) 23.1% 36.9% 10.6% 12.5% 26.3% 21.9% 10.6% 28.8% 12.5% 21.9% 40.0% 23.1% Yellow Time (s) 5.0 5.	. ,												
Total Split (%) 23.1% 36.9% 10.6% 12.5% 26.3% 21.9% 10.6% 28.8% 12.5% 21.9% 40.0% 23.1% Yellow Time (s) 5.0													
Yellow Time (s) 5.0 2.0													
All-Red Time (s) 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0	,												
Lost Time Adjust (s) -2.0 -2.0 -2.0 -2.0 -2.0 -2.0 -2.0 -2.0	` '												
	. ,												
	Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag Lead Lag Lead Lag Lead Lead Lead Lead Lead Lead Lead Lead													
Lead-Lag Optimize? Yes			_				_				_		

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Recall Mode	None	C-Max	None	None	C-Max	None						
Act Effct Green (s)	25.4	39.7	56.0	13.9	28.3	58.3	56.4	56.4	70.3	75.0	75.0	105.4
Actuated g/C Ratio	0.16	0.25	0.35	0.09	0.18	0.36	0.35	0.35	0.44	0.47	0.47	0.66
v/c Ratio	0.73	0.48	0.12	0.55	0.74	0.51	0.23	0.41	0.27	0.42	0.31	0.18
Control Delay	78.9	53.3	32.5	77.1	76.2	24.6	41.4	42.6	17.1	37.6	29.1	12.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	78.9	53.3	32.5	77.1	76.2	24.6	41.4	42.6	17.1	37.6	29.1	12.2
LOS	Е	D	С	Е	Е	С	D	D	В	D	С	В
Approach Delay		61.2			54.9			36.4			28.0	
Approach LOS		Ε			D			D			С	
Queue Length 50th (ft)	201	196	45	82	240	146	46	203	59	155	172	71
Queue Length 95th (ft)	282	250	70	123	322	157	96	294	119	265	258	127
Internal Link Dist (ft)		1280			1041			978			979	
Turn Bay Length (ft)	225		225	300		250			225			150
Base Capacity (vph)	347	616	554	314	420	562	284	1194	659	573	1644	1097
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.58	0.35	0.11	0.51	0.57	0.51	0.23	0.41	0.26	0.42	0.31	0.17

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 160

Offset: 104 (65%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.74

Intersection Signal Delay: 42.3 Intersection LOS: D
Intersection Capacity Utilization 75.5% ICU Level of Service D

Analysis Period (min) 15



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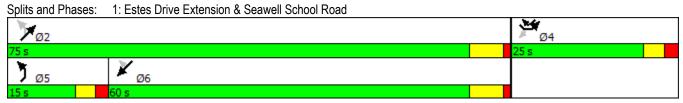
1: Estes Drive Extension & Seawell School Road

	-	À	ን	×	×	*
Lane Group	SEL	SER	NEL	NET	SWT	SWR
v/c Ratio	0.34	0.62	0.47	0.36	0.79	0.15
Control Delay	32.3	40.1	9.4	6.5	24.3	3.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	32.3	40.1	9.4	6.5	24.3	3.1
LOS	С	D	Α	Α	С	Α
Approach Delay	37.2			7.3	20.1	
Approach LOS	D			Α	С	
Queue Length 50th (ft)	47	82	25	74	256	19
Queue Length 95th (ft)	115	185	58	142	428	32
Internal Link Dist (ft)	782			565	745	
Turn Bay Length (ft)	400		250			75
Base Capacity (vph)	510	456	388	1650	1441	1316
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.23	0.42	0.45	0.26	0.48	0.13
Intersection Summary						
Area Type:	Other					
Cycle Length: 100						
Actuated Cycle Length: 7	4.3					
Natural Cycle: 60						
Control Type: Actuated-U	ncoordinated					
Maximum v/c Ratio: 0.79						

Intersection Signal Delay: 18.7

Analysis Period (min) 15

Intersection Capacity Utilization 62.0%



Intersection LOS: B

ICU Level of Service B

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	1	7	44	†	7	ሻ	^	7	ሻ	^	7
Traffic Volume (vph)	322	272	124	173	315	444	98	1132	180	247	700	319
Future Volume (vph)	322	272	124	173	315	444	98	1132	180	247	700	319
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		0%			1%			1%			-2%	
Storage Length (ft)	225		225	300		250	0		225	0		150
Storage Lanes	1		1	2		1	1		1	1		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Ped Bike Factor									0.96			
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	3416	1853	1575	1761	3522	1575	1787	3575	1599
Flt Permitted	0.950			0.950			0.195			0.071		
Satd. Flow (perm)	1770	1863	1583	3416	1853	1575	361	3522	1519	134	3575	1599
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		1360			1121			1058			1059	
Travel Time (s)		26.5			21.8			20.6			20.6	
Confl. Peds. (#/hr)								_0.0	5	5		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	339	286	131	182	332	467	103	1192	189	260	737	336
Shared Lane Traffic (%)												
Lane Group Flow (vph)	339	286	131	182	332	467	103	1192	189	260	737	336
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.01	1.01	1.01	1.01	1.01	1.01	0.99	0.99	0.99
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	pm+pt	NA		pm+pt	NA	pm+ov
Protected Phases	7	4	5	3	8	1	5	2	3	1	6	7
Permitted Phases			4			8	2		2	6		6
Detector Phase	7	4	5	3	8	1	5	2	3	1	6	7
Switch Phase												
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	12.0	7.0	7.0	12.0	7.0
Minimum Split (s)	14.0	14.0	15.0	15.0	14.0	15.0	15.0	35.0	15.0	15.0	19.0	14.0
Total Split (s)	38.0	57.0	15.0	20.0	39.0	32.0	15.0	71.0	20.0	32.0	88.0	38.0
Total Split (%)	21.1%	31.7%	8.3%	11.1%	21.7%	17.8%	8.3%	39.4%	11.1%	17.8%	48.9%	21.1%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lead	Lead	Lag	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	C-Max	None	None	C-Max	None
- Coult Mode	140116	140116	140116	110110	140116	140116	140116	O IVIAX	140116	140116	O IVIAX	140116

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Act Effct Green (s)	33.0	52.4	67.4	14.6	34.0	61.0	66.0	66.0	80.6	83.0	83.0	121.0
Actuated g/C Ratio	0.18	0.29	0.37	0.08	0.19	0.34	0.37	0.37	0.45	0.46	0.46	0.67
v/c Ratio	1.05	0.53	0.22	0.66	0.95	0.88	0.49	0.92	0.28	0.84	0.45	0.31
Control Delay	130.5	57.8	39.8	92.2	107.8	53.4	46.8	66.8	17.8	90.9	34.0	13.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	130.5	57.8	39.8	92.2	107.8	53.4	46.8	66.8	17.8	90.9	34.0	13.2
LOS	F	Е	D	F	F	D	D	Е	В	F	С	В
Approach Delay		87.3			79.0			59.2			39.9	
Approach LOS		F			Е			Е			D	
Queue Length 50th (ft)	~432	286	106	109	393	323	82	711	84	250	308	156
Queue Length 95th (ft)	#647	390	164	155	#597	#478	132	#824	122	#404	367	212
Internal Link Dist (ft)		1280			1041			978			979	
Turn Bay Length (ft)	225		225	300		250			225			150
Base Capacity (vph)	324	542	593	284	350	533	210	1292	688	309	1648	1074
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.05	0.53	0.22	0.64	0.95	0.88	0.49	0.92	0.27	0.84	0.45	0.31

Intersection Summary

Area Type: Other

Cycle Length: 180

Actuated Cycle Length: 180

Offset: 88 (49%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 110

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.05 Intersection Signal Delay: 62.5 Intersection Capacity Utilization 96.1%

Intersection LOS: E
ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



Appendix F – Synchro Unsignalized HCM Analysis Output

Intersection							
Intersection Int Delay, s/veh	0.1						
·							
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	ሻ		₽			7	
Traffic Vol, veh/h	1	621	336	8	5	0	
Future Vol, veh/h	1	621	336	8	5	0	
Conflicting Peds, #/hr		_ 0	0	_ 0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	125	-	-	-	75	0	
Veh in Median Storag		0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	7	7	40	2	
Mvmt Flow	1	690	373	9	6	0	
Major/Minor	Major1	N	Major2		Minor2		
Conflicting Flow All	382	0	- viajoi <u>-</u>	0	1070	378	
Stage 1	-	-	_	-	378	-	
Stage 2		_	_	_	692	_	
Critical Hdwy	4.12	_	_	-	6.8	6.22	
Critical Hdwy Stg 1	7.12	-	_	_	5.8	0.22	
Critical Hdwy Stg 2	-	-	-	-	5.8	-	
Follow-up Hdwy	2.218	-	_	-		3.318	
Pot Cap-1 Maneuver	1176	<u>-</u>	_		208	669	
•	1170	-	-	-	617	- 009	
Stage 1	-	-	-	-	433		
Stage 2	-	-	-	-	433	-	
Platoon blocked, %	1470	-	-	-	200	660	
Mov Cap-1 Maneuver		-	-	-	208	669	
Mov Cap-2 Maneuver		-	-	-	208	-	
Stage 1	-	-	-	-	617	-	
Stage 2	-	-	-	-	433	-	
Approach	EB		WB		SB		
HCM Control Delay, s	0		0		22.8		
HCM LOS					С		
J							
Minor Long (Maior M		EDI	EDT	WDT	WDD	ODL 4. C	י וחר
Minor Lane/Major Mvr	nt	EBL	EBT	WBT		SBLn1 S	
Capacity (veh/h)		1176	-	-	-	208	-
HCM Lane V/C Ratio		0.001	-	-	-	0.027	-
HCM Control Delay (s	5)	8.1	-	-	-	22.8	0
HCM Lane LOS		Α	-	-	-	С	Α
HCM 95th %tile Q(vel	۱)	0	-	-	-	0.1	-

Intersection						
Int Delay, s/veh	1.1					
		NIME	NICT	NED	CVA	CVACT
Movement	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	¥	0.4	\$	0.0	40	4
Traffic Vol, veh/h	25	21	744	88	12	316
Future Vol, veh/h	25	21	744	88	12	316
Conflicting Peds, #/hr	0	0	0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	8	14	2	2	17	7
Mvmt Flow	27	23	818	97	13	347
N.A ' /N.A'	N A'		1			
	Minor1		//ajor1		Major2	
Conflicting Flow All	1240	866	0	0	914	0
Stage 1	866	-	-	-	-	-
Stage 2	374	-	-	-	-	-
Critical Hdwy	6.48	6.34	-	-	4.27	-
Critical Hdwy Stg 1	5.48	-	-	-	-	-
Critical Hdwy Stg 2	5.48	-	-	-	-	-
Follow-up Hdwy	3.572	3.426	-	-	2.353	-
Pot Cap-1 Maneuver	188	336	-	_	687	-
Stage 1	402	-	_	_	-	_
Stage 2	683	_	_	_	_	-
Platoon blocked, %	300		_	_		_
Mov Cap-1 Maneuver	184	336	_		687	
Mov Cap-1 Maneuver	184	-		_	001	_
Stage 1	402	-	-	-	-	-
ŭ			-	-		-
Stage 2	667	-	-	-	-	-
Approach	NW		NE		SW	
HCM Control Delay, s	24.8		0		0.4	
HCM LOS	С					
1101111200						
Minor Lane/Major Mvm	nt	NET	NER	WLn1	SWL	SWT
Capacity (veh/h)		-	-	232	687	-
HCM Lane V/C Ratio		-	-	0.218	0.019	-
HCM Control Delay (s)		-	-		10.3	0
HCM Lane LOS		-	-	C	В	A
)			0.8	0.1	

Intersection						
Int Delay, s/veh	1.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
						SDK
Lane Configurations	\	7	\	↑ ↑	↑ }	120
Traffic Vol, veh/h	3	92	77	513	1055	130
Future Vol, veh/h	3	92	77	513	1055	130
Conflicting Peds, #/hr	10	0	_ 1	_ 0	_ 0	_ 1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	125	0	0	-	-	-
Veh in Median Storag		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	6	6	6	6	3	3
Mvmt Flow	3	99	83	552	1134	140
NA - ' /NA'	N4: O		4 . ' 4		4	
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	1656	638	1275	0	-	0
Stage 1	1205	-	-	-	-	-
Stage 2	451	-	-	-	-	-
Critical Hdwy	6.92	7.02	4.22	-	-	-
Critical Hdwy Stg 1	5.92	-	-	-	-	-
Critical Hdwy Stg 2	5.92	-	-	-	-	-
Follow-up Hdwy	3.56	3.36	2.26	-	-	-
Pot Cap-1 Maneuver	85	410	519	-	-	-
Stage 1	239	-	-	-	-	-
Stage 2	597	_	_	-	-	-
Platoon blocked, %				_	_	-
Mov Cap-1 Maneuver	71	410	519	_	_	_
Mov Cap-2 Maneuver		-	-	_	_	_
Stage 1	239	_	_	_	_	_
Stage 2	501	_		_	_	
Slage 2	301	_		_	_	_
Approach	EB		NB		SB	
HCM Control Delay, s	17.9		1.7		0	
HCM LOS	С					
Minaul ana /Maiau M	-4	NDI	NDT	EDL 4 -	TDL O	CDT
Minor Lane/Major Mvr	nt	NBL		EBLn1 I		SBT
Capacity (veh/h)		519	-		410	-
HCM Lane V/C Ratio		0.16	-	0.045		-
HCM Control Delay (s)	13.2	-	••••	16.6	-
HCM Lane LOS		В	-	F	С	-
HCM 95th %tile Q(veh	1)	0.6	-	0.1	0.9	-
-						

0.1					
	FDT	WDT	WED	CDI	CDD
			WBK		SBR
			_		*
					0
					0
					0
		Free			Stop
-	None	-	None		None
	-	-	-		0
e,# -	0	0	-	0	-
-	0	0	-	0	-
90	90	90	90	90	90
3	3	4	4	2	2
4	447	433	9	4	0
NA=:==4		Ania no		Min = =0	
					10.5
442	0	-	0		438
-	-	-	-		-
-	-	-	-		-
4.13	-	-	-		6.22
-	-	-	-		-
-	-	-	-	5.42	-
	-	-	-	3.518	3.318
1113	-	-	-	312	619
-	-	-	-	651	-
-	-	-	-	638	-
	-	-	-		
1113	-	-	-	311	619
_	-	_	-		-
-	-	_	-		-
_	_	_	_		-
				300	
0.1		0		16.7	
				С	
a t	EDI	CDT	WDT	WDD	CDI =4 CD
IL				WBR	
		-	-	-	311
		-		-	0.014
		-	-	-	16.7
		-	-	-	С
)	0	-	-	-	0
	EBL 4 4 0 Free - 125 9, # - 90 3 4 Major1 442 - 4.13 - 2.227 1113 - 1113	EBL EBT 4 402 4 402 0 0 0 Free Free - None 125 0 90 90 3 3 4 447 Major1 N 442 0 0 4.13 1 1113 1 1113 1 1113 1 EB 0.1 int EBL 1113 0.004 8.2 A	EBL EBT WBT 4 402 390 4 402 390 0 0 0 Free Free Free - None - 125 9,# - 0 0 90 90 90 3 3 4 4 447 433 Major1 Major2 442 0 4.13 2.227 1113 1113 1113	EBL EBT WBT WBR 4 402 390 8 4 402 390 8 0 0 0 0 0 Free Free Free Free - None - None 125 0 0 - 90 90 90 90 90 90 90 3 3 4 4 4 447 433 9 Major1 Major2 442 0 - 0 1113 1113 1113 1113 1113 1113 1113 1113	EBL EBT WBT WBR SBL 4 402 390 8 4 0 0 0 0 0 Free Free Free Free Stop None - None - 75 9,# - 0 0 - 0 90 90 90 90 90 90 90 90 90 90 3 3 4 4 2 442 0 - 0 894 - - - 438 - - - 438 - - - 456 4.13 - - 6.42 2.227 - - 3.518 1113 - - 651 - - - 638 - - - 651 - - -<

Intersection						
Int Delay, s/veh	0.8					
					0)	011
Movement	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	Y		₽			4
Traffic Vol, veh/h	22	17	363	38	12	385
Future Vol, veh/h	22	17	363	38	12	385
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	18	6	2	8	2	3
Mvmt Flow	23	18	382	40	13	405
	20	10	002	10	10	.00
Major/Minor	Minor1		/lajor1		Major2	
Conflicting Flow All	833	402	0	0	422	0
Stage 1	402	-	-	-	-	-
Stage 2	431	-	-	-	-	-
Critical Hdwy	6.58	6.26	-	-	4.12	-
Critical Hdwy Stg 1	5.58	-	-	-	_	-
Critical Hdwy Stg 2	5.58	-	-	-	_	-
Follow-up Hdwy	3.662	3.354	_	_	2.218	_
Pot Cap-1 Maneuver	318	640	_	-	1137	_
Stage 1	642	-	_	_		_
Stage 2	623	_				_
Platoon blocked, %	023		_		_	
Mov Cap-1 Maneuver	313	640	-	-	1137	-
•	313			-	1137	-
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	642	-	-	-	-	-
Stage 2	614	-	-	-	-	-
Approach	NW		NE		SW	
HCM Control Delay, s	14.9		0		0.2	
HCM LOS	14.3 B		U		0.2	
I IOIVI LOG	D					
Minor Lane/Major Mvn	nt	NET	NERN	IWLn1	SWL	SWT
Capacity (veh/h)		-	-	403	1137	-
HCM Lane V/C Ratio		-	-	0.102		-
HCM Control Delay (s)	-	-		8.2	0
HCM Lane LOS		_	_	В	Α	A
HCM 95th %tile Q(veh	1)	-	_	0.3	0	-
	,			3.0	J	

Intersection						
Int Delay, s/veh	1.4					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ነ	7	<u>ሻ</u>	^	↑ }	20
Traffic Vol, veh/h	20	58	74	574	591	30
Future Vol, veh/h	20	58	74	574	591	30
Conflicting Peds, #/hr	1	0	_ 1	_ 0	_ 0	_ 1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-		-		-	None
Storage Length	125	0	0	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	5	5	6	6	3	3
Mvmt Flow	22	63	80	624	642	33
Major/Minor	MinorO		Aniar1		Majora	
	Minor2		Major1		Major2	
Conflicting Flow All	1134	339	676	0	-	0
Stage 1	660	-	-	-	-	-
Stage 2	474	-	-	-	-	-
Critical Hdwy	6.9	7	4.22	-	-	-
Critical Hdwy Stg 1	5.9	-	-	-	-	-
Critical Hdwy Stg 2	5.9	-	-	-	-	-
Follow-up Hdwy	3.55	3.35	2.26	-	-	-
Pot Cap-1 Maneuver	192	648	885	-	-	-
Stage 1	468	-	-	-	-	-
Stage 2	584	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	174	647	885	-	-	-
Mov Cap-2 Maneuver	174	-	-	-	-	-
Stage 1	468	-	-	-	_	-
Stage 2	531	_	_	_	-	_
olago =						
Approach	EB		NB		SB	
HCM Control Delay, s	15.7		1.1		0	
HCM LOS	С					
Minor Lane/Major Mvm	nt	NBL	NRT	EBLn1 l	FRI n2	SBT
	ıı					ODI
Capacity (veh/h)		885	-		647	
HCM Central Delay (a)		0.091		0.125		-
HCM Control Delay (s)		9.5	-	-0.0	11.2	-
HCM Lane LOS	\	A	-	D	В	-
HCM 95th %tile Q(veh)	0.3	-	0.4	0.3	-

Intersection							
Int Delay, s/veh	0.2						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	ኘ	↑	1>	WDIX	ሻ	7	
Traffic Vol, veh/h	2	501	749	7	7	1	
Future Vol, veh/h	2	501	749	7	7	1	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	125	-	_	-	75	0	
Veh in Median Storage		0	0	_	0	-	
Grade, %	-	0	0	_	0	_	
Peak Hour Factor	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	28	2	
Mymt Flow	2	527	788	7	7	1	
					•		
	Major1		Major2		Minor2		
Conflicting Flow All	796	0	-		1324	792	
Stage 1	-	-	-	-	792	-	
Stage 2	-	-	-	-	532	-	
Critical Hdwy	4.12	-	-	-	6.68	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.68	-	
Critical Hdwy Stg 2	-	-	-	-	5.68	-	
Follow-up Hdwy	2.218	-	-	-	3.752		
Pot Cap-1 Maneuver	826	-	-	-	152	389	
Stage 1	-	-	-	-	404	-	
Stage 2	-	-	-	-	540	-	
Platoon blocked, %		-	-	-	4-0		
Mov Cap-1 Maneuver		-	-	-	152	389	
Mov Cap-2 Maneuver		-	-	-	152	-	
Stage 1	-	-	-	-	404	-	
Stage 2	-	-	-	-	539	-	
Approach	EB		WB		SB		
HCM Control Delay, s			0		28		
HCM LOS					D		
TIOW LOO							
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)		826	-	-	-	152	389
HCM Lane V/C Ratio		0.003	-	-	-	0.048	
HCM Control Delay (s)	9.4	-	-	-	29.9	14.3
HCM Lane LOS		Α	-	-	-	D	В
HCM 95th %tile Q(veh	1)	0	-	-	-	0.2	0

Intersection Int Delay, s/veh						
: · · · · · · · · · · · · · · · · · · ·	5.6					
Mayamant		NIME	NET	NED	CIAII	CMT
Movement	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	100	20	17 0	25	^	4
Traffic Vol, veh/h	106	38	476	35	6	659
Future Vol, veh/h	106	38	476	35	6	659
Conflicting Peds, #/hr	0	0	_ 0	_ 0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-		-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storag		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	5	4	2	6	2	2
Mvmt Flow	114	41	512	38	6	709
Major/Miner	Minard		lais 1		Mais	
Major/Minor	Minor1		//ajor1		Major2	
Conflicting Flow All	1253	531	0	0	549	0
Stage 1	531	-	-	-	-	-
Stage 2	722	-	-	-	-	-
Critical Hdwy	6.45	6.24	-	-	4.12	-
Critical Hdwy Stg 1	5.45	-	-	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-	-	-
Follow-up Hdwy	3.545	3.336	-	-	2.218	-
Pot Cap-1 Maneuver	187	544	-	-	1021	-
Stage 1	584	-	-	-	-	-
Stage 2	476	-	-	-	-	-
Platoon blocked, %			-	_		-
Mov Cap-1 Maneuver	185	544	_	_	1021	_
Mov Cap 1 Maneuver		-	_	_	-	_
Stage 1	584	_	_	_	_	_
Stage 2	471	_	-	_	_	_
Olaye Z	711					-
Approach	NW		NE		SW	
	50.7		0		0.1	
HCM Control Delay, s						
HCM Control Delay, s HCM LOS	F					
	F					
HCM LOS		NET	NEDA	I\A/I 4	CIVII	CVAT
HCM LOS Minor Lane/Major Mvr		NET		WLn1	SWL	SWT
Minor Lane/Major Mvr Capacity (veh/h)		-	-	224	1021	-
Minor Lane/Major Mvr Capacity (veh/h) HCM Lane V/C Ratio	nt	NET - -	-	224 0.691	1021 0.006	-
Minor Lane/Major Mvr Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s	nt	- - -	-	224 0.691 50.7	1021 0.006 8.5	- - 0
Minor Lane/Major Mvr Capacity (veh/h) HCM Lane V/C Ratio	nt)	-	-	224 0.691	1021 0.006	-

Intersection							
Int Delay, s/veh	1.6						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	ŀ
Lane Configurations			NDL			SDN	
Traffic Vol, veh/h	ኝ 36	1 7	43	↑↑ 1210	↑ ↑ 758	6	
Future Vol, veh/h	36	44	43	1210	758	6	
Conflicting Peds, #/hr	4	0	2	0	0	2	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	Stop -		-		-	None	
Storage Length	125	0	0	NOHE -	_	NOHE	
Veh in Median Storage,		-	-	0	0	-	
Grade, %	, # 0 0	-		0	0	-	
Peak Hour Factor	94	94	94	94	94	94	
			94 7				
Heavy Vehicles, %	6	6		1207	2	2	
Mvmt Flow	38	47	46	1287	806	6	
Major/Minor N	Minor2	N	Major1	N	Major2		
Conflicting Flow All	1551	408	815	0	-	0	
Stage 1	812	-	-	-	-	-	
Stage 2	739	-	-	-	-	-	
Critical Hdwy	6.92	7.02	4.24	-	-	-	
Critical Hdwy Stg 1	5.92	-	-	-	-	-	
Critical Hdwy Stg 2	5.92	_	_	-	_	-	
Follow-up Hdwy	3.56	3.36	2.27	_	-	-	
Pot Cap-1 Maneuver	100	581	777	-	_	-	
Stage 1	387	-	_	_	-	-	
Stage 2	423	_	-	-	-	-	
Platoon blocked, %				_	_	_	
Mov Cap-1 Maneuver	94	580	777	_	_	_	
Mov Cap-2 Maneuver	94	-		_	_	_	
Stage 1	386	_	_	_	_	_	
Stage 2	397	_	_	_	_	_	
Olage 2	001						
Approach	EB		NB		SB		
HCM Control Delay, s	36.8		0.3		0		
HCM LOS	Е						
Minor Lane/Major Mvmt	t	NBL	NRT	EBLn1 I	FRI n2	SBT	
Capacity (veh/h)		777	-		580	-	
HCM Lane V/C Ratio		0.059		0.407		_	
HCM Control Delay (s)		9.9	_	67.4	11.8	_	
HCM Lane LOS		9.9 A		67.4 F	11.0 B	_	
HCM 95th %tile Q(veh)		0.2	-	1.7	0.3	_	
LIGINI JOHL /OHIC GIVEII)		0.2	_	1.7	0.0	_	

Intersection							
Int Delay, s/veh	0.1						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	<u> </u>	<u></u>	₩ <u>₽</u>	WDIX	JDL	7	
Traffic Vol, veh/h	1	856	354	8	5	0	
Future Vol, veh/h	1	856	354	8	5	0	
Conflicting Peds, #/hr	0	0.00	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	- -		
Storage Length	125	-	_	-	75	0	
Veh in Median Storage		0	0	_	0	-	
Grade, %		0	0	_	0	_	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	7	7	40	2	
Mvmt Flow	1	951	393	9	6	0	
IVIVIIIL I IUW		301	333	3	U	U	
Major/Minor	Major1	N	//ajor2	N	/linor2		
Conflicting Flow All	402	0	-	0	1351	398	
Stage 1	-	-	-	-	398	-	
Stage 2	-	-	-	-	953	-	
Critical Hdwy	4.12	-	-	-	6.8	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.8	-	
Critical Hdwy Stg 2	-	-	-	-	5.8	-	
Follow-up Hdwy	2.218	-	-	-	3.86	3.318	
Pot Cap-1 Maneuver	1157	-	-	-	138	652	
Stage 1	-	-	-	-	603	-	
Stage 2	-	-	-	-	321	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1157	-	-	_	138	652	
Mov Cap-2 Maneuver		-	-	-	138	-	
Stage 1	-	-	-	-	603	-	
Stage 2	-	-	-	-	321	-	
Annragah	ED		WD		CD		
Approach	EB		WB		SB		
HCM Control Delay, s	0		0		32.2		
HCM LOS					D		
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR	SBLn1 S	BLn2
Capacity (veh/h)		1157	-	-	_	138	-
HCM Lane V/C Ratio		0.001	-	_	_	0.04	-
HCM Control Delay (s)	8.1	-	_	-	32.2	0
HCM Lane LOS	,	A	-	_	-	D	A
HCM 95th %tile Q(veh	1)	0	_	-	_	0.1	-
. Tom com 70mo Q(Voi	'/	U				J. 1	

Intersection						
Int Delay, s/veh	1.1					
		A IVA / D	NICT	NED	0)4"	OVACE
Movement	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	¥		ĵ.			4
Traffic Vol, veh/h	26	22	776	92	12	334
Future Vol, veh/h	26	22	776	92	12	334
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	8	14	2	2	17	7
Mvmt Flow	29	24	853	101	13	367
	Minor1		/lajor1		Major2	
Conflicting Flow All	1296	903	0	0	954	0
Stage 1	903	-	-	-	-	-
Stage 2	393	-	-	-	-	-
Critical Hdwy	6.48	6.34	-	-	4.27	-
Critical Hdwy Stg 1	5.48	-	-	-	-	-
Critical Hdwy Stg 2	5.48	-	-	-	-	-
Follow-up Hdwy	3.572	3.426	-	-	2.353	-
Pot Cap-1 Maneuver	174	319	_	_	663	-
Stage 1	386	_	_	_	-	-
Stage 2	669	_	_	_	_	_
Platoon blocked, %	000		_	_		_
Mov Cap-1 Maneuver	170	319	_		663	_
Mov Cap-1 Maneuver	170	313	_	_	000	_
Stage 1	386	-	-	_		_
ŭ	652	-	-	-	-	-
Stage 2	052	-	-	_	-	-
Approach	NW		NE		SW	
HCM Control Delay, s	27		0		0.4	
HCM LOS	D					
Minor Lane/Major Mvm	nt	NET	NERN	VWLn1	SWL	SWT
Capacity (veh/h)		-	-	216	663	-
HCM Lane V/C Ratio		-	-	0.244	0.02	-
HCM Control Delay (s)		-	-	27	10.5	0
HCM Lane LOS		-	-	D	В	Α
HCM 95th %tile Q(veh			_		0.1	-

Intersection						
Int Delay, s/veh	1.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
						אמט
Lane Configurations	\	7	أ	↑↑ 542	↑1	135
Traffic Vol, veh/h Future Vol, veh/h	3	96 96	80 80	542	1123	135
	10	96	1	0		135
Conflicting Peds, #/hr Sign Control				Free	0 Free	Free
RT Channelized	Stop -	Stop None	Free			None
	125	0	-	None -	-	None
Storage Length			0		-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	6	6	6	6	3	3
Mvmt Flow	3	103	86	583	1208	145
Major/Minor I	Minor2	N	Major1	N	Major2	
Conflicting Flow All	1754	677	1354	0		0
Stage 1	1281	-	-	-	_	-
Stage 2	473	_	_	_	_	_
Critical Hdwy	6.92	7.02	4.22	_	_	_
Critical Hdwy Stg 1	5.92	-	-	_	_	_
Critical Hdwy Stg 2	5.92	_	_	_	_	_
Follow-up Hdwy	3.56	3.36	2.26	_	_	_
Pot Cap-1 Maneuver	73	386	484	_	_	_
Stage 1	217	-	-	_	_	_
Stage 2	582	-	_	_	_	_
Platoon blocked, %	002			_	_	_
Mov Cap-1 Maneuver	60	386	484			
Mov Cap-2 Maneuver	60	-	-	_	_	_
Stage 1	217	_	-	_	-	
•	478	-	-	-	_	-
Stage 2	470	_	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	19.2		1.8		0	
HCM LOS	С					
Minor Long/Major Mare		NDI	NDT	EDI 54 F	EDI 20	CDT
Minor Lane/Major Mvm	ı	NBL		EBLn1 E		SBT
Capacity (veh/h)		484	-	00	386	-
HCM Lane V/C Ratio		0.178	-	0.054		-
HCM Control Delay (s)		14	-	68.4	17.7	-
HCM Lane LOS		В	-	F	C	-
HCM 95th %tile Q(veh)		0.6	-	0.2	1.1	-

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
				WDK		SBR
Lane Configurations Traffic Vol, veh/h	ነ	↑ 421	1 → 410	8	ች	ր 0
Future Vol, veh/h	4	421	410	8	4	0
	0	421	410	0	0	0
Conflicting Peds, #/hr		Free	Free			
Sign Control RT Channelized	Free -	None	Free -	Free None	Stop -	Stop None
	125	None -		None -	- 75	
Storage Length			-			0
Veh in Median Storage		0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	3	3	4	4	2	2
Mvmt Flow	4	468	456	9	4	0
Major/Minor	Major1	N	Major2		Minor2	
Conflicting Flow All	464	0	- viajoi <u>-</u>	0	937	460
Stage 1	-	-	_	-	460	400
Stage 2	_		_	_	477	-
Critical Hdwy	4.13	_	_	_	6.42	6.22
Critical Hdwy Stg 1	4.13	-	-	-	5.42	0.22
	-	-	-	-	5.42	-
Critical Hdwy Stg 2	2.227	_	-		3.518	
Follow-up Hdwy		-	-	-		
Pot Cap-1 Maneuver	1092	-	-	-	294	601
Stage 1	-	-	-	-	636	-
Stage 2	-	-	-	-	624	-
Platoon blocked, %	4000	-	-	-	000	004
Mov Cap-1 Maneuver	1092	-	-	-	293	601
Mov Cap-2 Maneuver	-	-	-	-	293	-
Stage 1	-	-	-	-	636	-
Stage 2	-	-	-	-	622	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		17.5	
HCM LOS	0.1		0		17.5	
TIOWI LOG					U	
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR	SBLn1 S
Capacity (veh/h)		1092	-	-	-	293
HCM Lane V/C Ratio		0.004	-	-	-	0.015
HCM Control Delay (s))	8.3	-	-	-	17.5
HCM Lane LOS		Α	-	-	-	С
HCM 95th %tile Q(veh)	0	-	-	-	0

Intersection						
Int Delay, s/veh	0.8					
		NIMD	NET	NED	CIVII	CMT
Movement	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	*	40	\$	40	40	4
Traffic Vol, veh/h	23	18	381	40	12	404
Future Vol, veh/h	23	18	381	40	12	404
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	18	6	2	8	2	3
Mvmt Flow	24	19	401	42	13	425
						120
	Minor1		Major1		Major2	
Conflicting Flow All	873	422	0	0	443	0
Stage 1	422	-	-	-	-	-
Stage 2	451	-	-	-	-	-
Critical Hdwy	6.58	6.26	-	-	4.12	-
Critical Hdwy Stg 1	5.58	-	-	_	_	_
Critical Hdwy Stg 2	5.58	_	-	_	_	_
Follow-up Hdwy	3.662	3 354	_	_	2.218	_
Pot Cap-1 Maneuver	301	623	_	_	1117	_
Stage 1	629	-	_	_	_	_
Stage 2	609	_	_	_	_	_
Platoon blocked, %	003		_	_	_	_
	296	623	-	-	1117	
Mov Cap-1 Maneuver			-	-	1117	-
Mov Cap-2 Maneuver	296	-	-	-	-	-
Stage 1	629	-	-	-	-	-
Stage 2	600	-	-	-	-	-
Approach	NW		NE		SW	
HCM Control Delay, s	15.5		0		0.2	
HCM LOS	13.5 C		U		0.2	
TICIVI LOS	U					
Minor Lane/Major Mvn	nt	NET	NERN	WLn1	SWL	SWT
Capacity (veh/h)		-	-	385	1117	-
HCM Lane V/C Ratio		-	_	0.112		_
HCM Control Delay (s)		_	-		8.3	0
HCM Lane LOS		_	_	C	Α	A
HCM 95th %tile Q(veh	\		_	• •	0	-
HOW SOUL WILL CALLED	J	-	-	0.4	U	_

Intersection						
Int Delay, s/veh	1.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
						SDK
Lane Configurations	\	7	7	^	† }	24
Traffic Vol, veh/h	21	60	77	614	633	31
Future Vol, veh/h	21	60	77	614	633	31
Conflicting Peds, #/hr	1	0	_ 1	_ 0	_ 0	_ 1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-		-	None	-	None
Storage Length	125	0	0	-	-	-
Veh in Median Storage	,#0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	5	5	6	6	3	3
Mvmt Flow	23	65	84	667	688	34
		_				
	Minor2		Major1		Major2	
Conflicting Flow All	1208	362	723	0	-	0
Stage 1	706	-	-	-	-	-
Stage 2	502	-	-	-	-	-
Critical Hdwy	6.9	7	4.22	-	-	-
Critical Hdwy Stg 1	5.9	-	-	-	-	-
Critical Hdwy Stg 2	5.9	-	-	-	-	-
Follow-up Hdwy	3.55	3.35	2.26	-	-	-
Pot Cap-1 Maneuver	171	626	849	_	_	_
Stage 1	443	-	-	_	_	_
Stage 2	565	_	_	_	_	_
Platoon blocked, %	000			_	_	_
Mov Cap-1 Maneuver	154	625	849		_	
	154	025	043	-	-	_
Mov Cap-2 Maneuver			-	-	-	-
Stage 1	443	-	-	-	-	-
Stage 2	509	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	16.8		1.1		0	
HCM LOS	C		1.1		U	
TIOWI LOG	U					
Minor Lane/Major Mvm	t	NBL	NBT	EBLn1	EBLn2	SBT
Capacity (veh/h)		849	-	154	625	-
HCM Lane V/C Ratio		0.099	-	0.148		-
HCM Control Delay (s)		9.7	-	32.4	11.4	-
HCM Lane LOS		A	_	D	В	_
HCM 95th %tile Q(veh)		0.3	_	0.5	0.3	_
How John Johne Q(Ven)		0.0		0.5	0.0	

Interception							
Intersection Int Delay, s/veh	0.2						
iiii Deiay, S/VeII							
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	ች		- î∍		1	- 7	
Traffic Vol, veh/h	2	528	783	7	7	1	
Future Vol, veh/h	2	528	783	7	7	1	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	125	-	-	-	75	0	
Veh in Median Storage	e,# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	28	2	
Mvmt Flow	2	556	824	7	7	1	
Major/Minor	Major1		/lajor2		Minor2		
Conflicting Flow All	832	0	- najoiz	0	1388	828	
Stage 1	032	-			828	020	
•	-		-	-	560		
Stage 2	4.12	-	-	-	6.68	6.22	
Critical Hdwy	4.12			-	5.68		
Critical Hdwy Stg 1	- -	-	-	-		-	
Critical Hdwy Stg 2	2 240	-	-	-	5.68	2 240	
Follow-up Hdwy	2.218	-	-		3.752		
Pot Cap-1 Maneuver	801	-	-	-	138	371	
Stage 1	-	-	-	-	388	-	
Stage 2	-	-	-	-	523	-	
Platoon blocked, %	004	-	-	-	400	0=1	
Mov Cap-1 Maneuver	801	-	-	-	138	371	
Mov Cap-2 Maneuver	-	-	-	-	138	-	
Stage 1	-	-	-	-	388	-	
Stage 2	-	-	-	-	522	-	
Approach	EB		WB		SB		
HCM Control Delay, s	0		0		30.4		
HCM LOS	U		U		30.4 D		
I IOIVI LOS					U		
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)		801	-	-	-	138	371
HCM Lane V/C Ratio		0.003	-	-	-	0.053	
HCM Control Delay (s))	9.5	-	-	-	32.6	14.7
HCM Lane LOS		A	-	-	_	D	В
HCM 95th %tile Q(veh)	0	_	_	-	0.2	0
TOW John John Q(Ven	1	J	_	_		U.Z	J

Interception						
Intersection Int Delay, s/veh	7.1					
•					0)	011
Movement	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	¥		₽			4
Traffic Vol, veh/h	110	40	502	36	6	689
Future Vol, veh/h	110	40	502	36	6	689
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	5	4	2	6	2	2
Mvmt Flow	118	43	540	39	6	741
Majar/Minar	N din a ud		1-:1		Maiano	
	Minor1		//ajor1		Major2	
Conflicting Flow All	1313	559	0	0	578	0
Stage 1	559	-	-	-	-	-
Stage 2	754	-	-	-	-	-
Critical Hdwy	6.45	6.24	-	-	4.12	-
Critical Hdwy Stg 1	5.45	-	-	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-	-	-
Follow-up Hdwy	3.545		-	-	2.218	-
Pot Cap-1 Maneuver	172	525	-	-	996	-
Stage 1	567	-	-	-	-	-
Stage 2	459	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	170	525	-	-	996	-
Mov Cap-2 Maneuver	170	-	-	-	-	-
Stage 1	567	_	_	-	-	-
Stage 2	454	-	-	_	-	_
g 						
Annroach	NW		NE		SW	
Approach						
HCM Control Delay, s	65.3		0		0.1	
HCM LOS	F					
Minor Lane/Major Mvn	nt	NET	NERN	WLn1	SWL	SWT
Capacity (veh/h)		-		207	996	_
HCM Lane V/C Ratio		_		0.779		_
HCM Control Delay (s)		_	_	65.3	8.6	0
HCM Lane LOS		_	_	65.5 F	Α	A
HCM 95th %tile Q(veh)		_	5.4	0	-
HOW SOUT MILE Q(VEH)	_	-	5.4	U	-

Intersection						
Int Delay, s/veh	1.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
						SDK
Lane Configurations	\	7	<u>ነ</u>	^	†	c
Traffic Vol, veh/h	37	46	45	1291	808	6
Future Vol, veh/h	37	46	45	1291	808	6
Conflicting Peds, #/hr	4	0	_ 2	_ 0	_ 0	_ 2
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-		-		-	None
Storage Length	125	0	0	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	6	6	7	2	2	2
Mvmt Flow	39	49	48	1373	860	6
NA = : = = /NA:= = =	1:O		4-:4		M-:0	
	Minor2		Major1		Major2	
Conflicting Flow All	1651	435	868	0	-	0
Stage 1	865	-	-	-	-	-
Stage 2	786	-	-	-	-	-
Critical Hdwy	6.92	7.02	4.24	-	-	-
Critical Hdwy Stg 1	5.92	-	-	-	-	-
Critical Hdwy Stg 2	5.92	-	-	-	-	-
Follow-up Hdwy	3.56	3.36	2.27	-	-	-
Pot Cap-1 Maneuver	86	558	741	-	-	-
Stage 1	363	-	-	-	-	-
Stage 2	399	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	80	557	741	_	-	-
Mov Cap-2 Maneuver	80	-	-	_	_	_
Stage 1	362	_	_	_	_	-
Stage 2	372	_	_	_	_	_
Jugo 2	512					
Approach	EB		NB		SB	
HCM Control Delay, s	45.7		0.3		0	
HCM LOS	Е					
Minor Long/Major Mare		NDI	NDT	EDI 51	EDI 22	CDT
Minor Lane/Major Mvm	l e	NBL		EBLn1		SBT
Capacity (veh/h)		741	-	- 00	557	-
HCM Lane V/C Ratio		0.065	-	0.492		-
HCM Control Delay (s)		10.2	-	87.5	12.1	-
HCM Lane LOS		В	-	F	В	-
HCM 95th %tile Q(veh)		0.2	-	2.1	0.3	-

Intersection												
Int Delay, s/veh	0.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	1>	LDIX	ኘ	ĵ.	TT DIX	IIDL	4	HOIL	ኘ	4	ODIT
Traffic Vol, veh/h	1	862	6	19	355	8	1	0	2	5	0	0
Future Vol, veh/h	1	862	6	19	355	8	1	0	2	5	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	_	_	None	-	-	None	-	-	None
Storage Length	125	-	-	100	-	-	-	-	-	75	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	7	7	2	2	2	40	2	2
Mvmt Flow	1	958	7	21	394	9	1	0	2	6	0	0
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	403	0	0	964	0	0	1404	1409	961	1405	1408	399
Stage 1	403	-	-	30 4 -	-	-	963	963	901	441	441	399
Stage 2	_	_	_		-	_	441	446	_	964	967	
Critical Hdwy	4.12	-	-	4.12	<u>-</u>	-	7.12	6.52	6.22	7.5	6.52	6.22
Critical Hdwy Stg 1	7.12	_	_	T. 1Z	_	_	6.12	5.52	0.22	6.5	5.52	0.22
Critical Hdwy Stg 1	-	_	_	_		_	6.12	5.52	_	6.5	5.52	_
Follow-up Hdwy	2.218	_	<u>-</u>	2.218	<u>-</u>	_	3.518	4.018	3.318	3.86	4.018	3.318
Pot Cap-1 Maneuver	1156	_	_	714	_	_	117	139	311	97	139	651
Stage 1	- 100	_	_	- 17	<u>-</u>	_	307	334	-	528	577	-
Stage 2	_	_	_	_	_	_	595	574	-	262	333	-
Platoon blocked, %		_	_		_	_	300	J1 1			300	
Mov Cap-1 Maneuver	1156	_	_	714	-	_	114	135	311	94	135	651
Mov Cap-2 Maneuver	-	-	_	-	_	-	114	135	-	94	135	-
Stage 1	-	-	-	-	-	-	307	334	-	528	560	-
Stage 2	_	_	_	_	_	_	578	557	_	260	333	_
g							J. J	J			300	
Approach	EB			WB			NB			SB		
				0.5								
HCM Control Delay, s HCM LOS	0			0.5			23.6			45.7		
HOM FOS							С			E		
Minor Lane/Major Mvm	nt l	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2		
Capacity (veh/h)		197	1156	-	-	714	-	-	94	-		
HCM Lane V/C Ratio		0.017		-	-	0.03	-	-	0.059	-		
HCM Control Delay (s)		23.6	8.1	-	-	10.2	-	-	45.7	0		
HCM Lane LOS		С	A	-	-	В	-	-	Е	Α		
HCM 95th %tile Q(veh)	0.1	0	-	-	0.1	-	-	0.2	-		

-							
Intersection							
Int Delay, s/veh	0.9						
Movement	NBL	NBR	NET	NER	SWL	SWT	
				NEK			
Lane Configurations	<u> </u>	12	1	C	<u>ነ</u>	270	
Traffic Vol., veh/h	1	12 12	863	6	95	379	
Future Vol, veh/h	1		863	6	95	379	
Conflicting Peds, #/hr	0	0	0	_ 0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-		-	None	-	None	
Storage Length	50	0	-	-	100	-	
Veh in Median Storage		-	0	-	-	0	
Grade, %	0	-	0	-	-	0	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	1	13	959	7	106	421	
Major/Minor I	Minor1		Major1	_	Major2		
Conflicting Flow All	1594	962	0	0	966	0	
Stage 1	962	-		_	-	-	
Stage 2	632	_	_	_	_	_	
Critical Hdwy	6.42	6.22	-	_	4.12	_	
Critical Hdwy Stg 1	5.42	-	_	_		_	
Critical Hdwy Stg 2	5.42	_	_	_	_	_	
Follow-up Hdwy	3.518		_	_	2.218	_	
Pot Cap-1 Maneuver	118	310	_		713	_	
Stage 1	371	-	_	_	. 10	_	
Stage 2	530	_		_			
Platoon blocked, %	550			_	_		
Mov Cap-1 Maneuver	100	310	_	_	713	_	
Mov Cap-1 Maneuver	231	310	-	_	113	-	
•	371	-	-		-		
Stage 1		-	-	-	-	-	
Stage 2	451	-	-	-	-	-	
Approach	NB		NE		SW		
HCM Control Delay, s	17.4		0		2.2		
HCM LOS	С						
Minor Lane/Major Mvm	t	NET	NER	NBLn11	VRI n2	SWL	SWT
Capacity (veh/h)				231	310	713	-
HCM Lane V/C Ratio		_		0.005			_
HCM Control Delay (s)		-	_		17.1	10.9	-
HCM Lang LOC		-	-	20.7	17.1	10.9	-

С

С

0.1

В

0.5

HCM Lane LOS

HCM 95th %tile Q(veh)

Movement	Intersection						
Name		1.3					
Cane Configurations			NIME	NET	NED	CIAII	CVA/T
Traffic Vol, veh/h Future Vol, veh/future Vol, veh			NWK		NER		
Future Vol, veh/h Conflicting Peds, #/hr O O O O O O O O O O O O O O O O O O O			00		0.5		
Conflicting Peds, #/hr 0 None Free Round None O O Grade, % 0 - 0 - 0 0 99 0 Heavy Vehicles, % 8 14 2 2 17 7 Mwy Major/Minor							
Sign Control Stop RT Channelized Stop None Free RT Channelized Free RT Channelized - None - None - None - None Description All Description All Description All Description All							
RT Channelized							
Storage Length 0							
Veh in Median Storage, # 0 - 0 - - 0 Grade, % 0 - 0 - - 0 Peak Hour Factor 91							None
Grade, % 0 - 0 - - 0 Peak Hour Factor 91 7 7 7 7 7 8 14 2 2 17 7 7 7 7 7 7 8 44 2 2 17 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 8 4 4 4 4 4 4 4 4 4 4 4 4 4 4 2 2 1 4 4 4 4 4 4 4 4 4 2 2 1 4 4 4 4 4 2 7 <td< td=""><td></td><td></td><td>-</td><td></td><td>-</td><td></td><td>-</td></td<>			-		-		-
Peak Hour Factor 91 91 91 91 91 91 91 91 91 91 91 91 91 91 91 91 91 91 91 7 7 Mwmt Flow 56 24 865 104 13 464 Major/Minor Minor Major Major <td>•</td> <td></td> <td>-</td> <td></td> <td>-</td> <td>-</td> <td></td>	•		-		-	-	
Heavy Vehicles, % 8							
Mymt Flow 56 24 865 104 13 464 Major/Minor Minor1 Major1 Major2 Conflicting Flow All 1407 917 0 0 969 0 Stage 1 917 - - - - - Stage 2 490 - - - - - Critical Hdwy 548 - - - - - Critical Hdwy Stg 1 5.48 - - - - - Critical Hdwy Stg 2 5.48 - - - - - Critical Hdwy Stg 2 5.48 - - - - - - Critical Hdwy Stg 2 5.48 - <t< td=""><td>Peak Hour Factor</td><td>91</td><td></td><td></td><td>91</td><td>91</td><td></td></t<>	Peak Hour Factor	91			91	91	
Major/Minor Minor1 Major1 Major2 Conflicting Flow All 1407 917 0 0 969 0 Stage 1 917 - - - - - Stage 2 490 - - - - - Critical Hdwy 6.48 6.34 - 4.27 - Critical Hdwy Stg 1 5.48 - - - - - Critical Hdwy Stg 2 5.48 - - - - - - Follow-up Hdwy 3.572 3.426 - 2.353 -	Heavy Vehicles, %	8	14	2		17	7
Conflicting Flow All	Mvmt Flow	56	24	865	104	13	464
Conflicting Flow All							
Conflicting Flow All	Major/Minor	Minor		laier1		Major	
Stage 1 917 -							
Stage 2 490 -				0	0	969	0
Critical Hdwy 6.48 6.34 - - 4.27 - Critical Hdwy Stg 1 5.48 - - - - - Critical Hdwy Stg 2 5.48 - - - - - - Follow-up Hdwy 3.572 3.426 - - 2.353 - Pot Cap-1 Maneuver 149 313 - 654 - Stage 1 380 - - - - Stage 2 604 - - - - - Mov Cap-1 Maneuver 146 313 - 654 - - Mov Cap-2 Maneuver 273 -				-	-		-
Critical Hdwy Stg 1 5.48 - - - - Critical Hdwy Stg 2 5.48 - - - - Follow-up Hdwy 3.572 3.426 - - 2.353 - Pot Cap-1 Maneuver 149 313 - 654 - Stage 1 380 - - - - Stage 2 604 - - - - - Platoon blocked, % -				-	-		-
Critical Hdwy Stg 2 5.48 -	•		6.34	-	-	4.27	-
Follow-up Hdwy 3.572 3.426 2.353 - Pot Cap-1 Maneuver 149 313 - 654 - Stage 1 380 Stage 2 604 Platoon blocked, % Mov Cap-1 Maneuver 146 313 - 654 - Mov Cap-2 Maneuver 273 Stage 1 380 Stage 1 380 Stage 2 592 Approach NW NE SW HCM Control Delay, s 22.6 HCM LOS C Minor Lane/Major Mvmt NET NERNWLn1 SWL SWT Capacity (veh/h) - 284 654 - HCM Lane V/C Ratio - 0.282 0.02 - HCM Control Delay (s) - 22.6 10.6 - HCM Control Delay (s) - 22.6 10.6 - HCM Control Delay (s) C B -	Critical Hdwy Stg 1		-	-	-	-	-
Pot Cap-1 Maneuver	Critical Hdwy Stg 2			-	-		-
Stage 1 380 -	Follow-up Hdwy			-	-		-
Stage 2 604 -	Pot Cap-1 Maneuver	149	313	-	-	654	-
Stage 2 604 -	Stage 1	380	-	-	-	-	-
Platoon blocked, %		604	-	-	-	-	-
Mov Cap-1 Maneuver 146 313 - - 654 - Mov Cap-2 Maneuver 273 - - - - - Stage 1 380 -	Platoon blocked, %			-	-		-
Mov Cap-2 Maneuver 273 -		146	313	-	-	654	-
Stage 1 380 -	•			_	_		_
Stage 2 592 -				_	-	_	-
Approach NW NE SW HCM Control Delay, s 22.6 0 0.3 HCM LOS C Minor Lane/Major Mvmt NET NERNWLn1 SWL SWT Capacity (veh/h) - - 284 654 - HCM Lane V/C Ratio - - 0.282 0.02 - HCM Control Delay (s) - - 22.6 10.6 - HCM Lane LOS - C B -	•			_	_		_
HCM Control Delay, s 22.6 0 0.3	Olage Z	332					
HCM Control Delay, s 22.6 0 0.3							
Minor Lane/Major Mvmt NET NERNWLn1 SWL SWT Capacity (veh/h) - - 284 654 - HCM Lane V/C Ratio - - 0.282 0.02 - HCM Control Delay (s) - - 22.6 10.6 - HCM Lane LOS - C B -	Approach	NW		NE		SW	
Minor Lane/Major Mvmt NET NERNWLn1 SWL SWT Capacity (veh/h) - - 284 654 - HCM Lane V/C Ratio - - 0.282 0.02 - HCM Control Delay (s) - - 22.6 10.6 - HCM Lane LOS - C B -	HCM Control Delay, s	22.6		0		0.3	
Minor Lane/Major Mvmt NET NERNWLn1 SWL SWT Capacity (veh/h) - - 284 654 - HCM Lane V/C Ratio - - 0.282 0.02 - HCM Control Delay (s) - - 22.6 10.6 - HCM Lane LOS - C B -	HCM LOS						
Capacity (veh/h) - - 284 654 - HCM Lane V/C Ratio - - 0.282 0.02 - HCM Control Delay (s) - - 22.6 10.6 - HCM Lane LOS - C B -							
Capacity (veh/h) - - 284 654 - HCM Lane V/C Ratio - - 0.282 0.02 - HCM Control Delay (s) - - 22.6 10.6 - HCM Lane LOS - C B -	Minor Long/Major M.	m t	NET	NEDA	1\A/I 1	CIAII	CMT
HCM Lane V/C Ratio - - 0.282 0.02 - HCM Control Delay (s) - - 22.6 10.6 - HCM Lane LOS - C B -		III	INE I				21/1
HCM Control Delay (s) 22.6 10.6 - HCM Lane LOS C B -	. , ,		-				-
HCM Lane LOS C B -		,	-				-
		S)	-	-			-
HCM 95th %tile Q(veh) 1.1 0.1 -			-	-			-
	HCM 95th %tile Q(vel	h)	-	-	1.1	0.1	-

2: West Site Driveway/UNC Park-and-Ride Dr & Estes Drive Extension

Int Delay, s/veh	Intersection												
Movement EBL EBR WBL WBT WBR NBL NBT NBR SBL SBR SBR Lane Configurations Traffic Vol, veh/h 4 425 4 13 414 8 4 0 13 4 0 0 0 Conflicting Peds, #hr 0 0 0 0 0 0 0 0 0		0.5											
Lane Configurations	-												
Traffic Vol, veh/h				EBR			WBR	NBL		NBR			SBR
Future Vol, veh/h Conflicting Peds, #hhr O O O O O O O O O O O O O O O O O O													
Conflicting Peds, #/hr													
Sign Control Free RTCANNOR Free RTCANNOR Free RTCANNOR Free RTCANNOR Free RTCANNOR Stop None RTCANNOR St	· ·												
RT Channelized	•												
Storage Length		Free	Free		Free	Free		Stop	Stop		Stop	Stop	
Veh in Median Storage, # - 0			-	None		-	None	-	-	None		-	None
Grade, % - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - 0 - 0 - 0 9 90			-	-	100	-	-	-	-	-	75	-	-
Peak Hour Factor	•	,# -		-	-		-	-		-	-		-
Heavy Vehicles, % 3 3 2 2 4 4 2 2 2 2 2 2													
Mynt Flow 4 472 4 14 460 9 4 0 14 4 0 0 Major/Minor Major1 Major2 Minor1 Minor2 Minor2 Conflicting Flow All 469 0 0 477 0 0 976 981 474 984 979 464 Stage 1 - - - - - 483 483 - 493 493 - Critical Hdwy 4.13 - 4.12 - 7.12 6.52 6.22 7.12 6.52 6.22 7.12 6.52 6.22 7.12 6.52 6.22 7.12 6.52 6.22 7.12 6.52 6.22 7.12 6.52 6.22 7.12 6.52 6.22 7.12 6.52 6.22 7.12 6.52 6.22 7.12 6.52 6.22 7.12 6.52 6.22 7.12 6.52 6.22 7.12 6.52 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>													
Major/Minor Major1													
Conflicting Flow All	Mvmt Flow	4	472	4	14	460	9	4	0	14	4	0	0
Conflicting Flow All 469 0 0 477 0 0 976 981 474 984 979 464													
Conflicting Flow All	Major/Minor N	Major1			Major2			Minor1			Minor2		
Stage 1		_	0		_	0			981			979	464
Stage 2 - - - - 493 498 - 491 486 - Critical Hdwy 4.13 - - 4.12 - - 7.12 6.52 6.22 7.12 6.52 6.22 Critical Hdwy Stg 1 - - - - 6.12 5.52 - 6.12 5.52 - Critical Hdwy Stg 2 - - - - 6.12 5.52 - 6.12 5.52 - Follow-up Hdwy 2.227 - 2.218 - 3.518 4.018 3.318 3.518 4.018 3.318 Pol Cap-1 Maneuver 1087 - 1085 - 230 249 590 228 250 598 Stage 2 - - - - - 558 544 - 559 551 - Mov Cap-1 Maneuver 1087 - 1085 - 227 245 590 <td>•</td> <td>-</td> <td></td> <td>-</td> <td>-</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	•	-		-	-								
Critical Hdwy 4.13 - 4.12 - - 7.12 6.52 6.22 7.12 6.52 6.22 Critical Hdwy Stg 1 - - - - - 6.12 5.52 - 6.12 5.52 - Critical Hdwy Stg 2 - - - - 6.12 5.52 - 6.12 5.52 - Follow-up Hdwy 2.227 - - 2.218 - - 3.518 4.018 3.318 3.518 4.018 3.318 Pot Cap-1 Maneuver 1087 - 1085 - - 230 249 590 228 250 598 Stage 1 - - - - - 555 553 - 558 547 - Stage 2 - - - - - 227 245 590 220 246 598 Mov Cap-2 Maneuver - - - - - 227 245 590 220 246 - 384	•	_		_	_								
Critical Hdwy Stg 1 - - - - - 6.12 5.52 - 5.52 - 6.12 5.52 - 5.52 - 6.12 5.52 - 5.52 - 5.58 544 - 5.58 547 - - 5.58 547 -		4.13		_	4.12								6.22
Critical Hdwy Stg 2 - - - - 6.12 5.52 - 6.12 5.52 - Follow-up Hdwy 2.227 - - 2.218 - - 3.518 4.018 3.318 3.518 4.018 3.318 Pot Cap-1 Maneuver 1087 - 1085 - - 230 249 590 228 250 598 Stage 1 - - - - - 555 553 - 558 547 - Stage 2 - - - - - - 558 544 - 559 551 - Mov Cap-1 Maneuver 1087 - 1085 - - 227 245 590 220 246 598 Mov Cap-2 Maneuver - - - - 227 245 - 220 246 - - Stage 2 - - - -	•	-	_	-	-	_				-			
Follow-up Hdwy 2.227 2.218 3.518 4.018 3.318 3.518 4.018 3.318 Pot Cap-1 Maneuver 1087 1085 230 249 590 228 250 598 Stage 1	, ,	_	_	_	_		_			_			_
Pot Cap-1 Maneuver 1087	, ,		_	_	2.218		_						3.318
Stage 1 - - - - 565 553 - 558 547 - Stage 2 - - - - - 558 544 - 559 551 - Platoon blocked, % -<				_									
Stage 2 - - - 558 544 - 559 551 - Platoon blocked, % - <	•	-	_	_	-	_							
Platoon blocked, %		_	_	_	_								
Mov Cap-1 Maneuver 1087 - - 1085 - - 227 245 590 220 246 598 Mov Cap-2 Maneuver - - - - - - 227 245 - 220 246 - Stage 1 - - - - - 563 551 - 556 540 - Stage 2 - - - - - 551 537 - 543 549 - Approach EB WB NB SB HCM Control Delay, s 0.1 0.2 13.8 21.7 HCM Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 SBLn2 Capacity (veh/h) 429 1087 - - 1085 - - 220 - HCM Lane V/C Ratio 0.044 0.004 - - 0.013 - - <td>•</td> <td></td> <td>_</td> <td>_</td> <td></td> <td></td> <td></td> <td>500</td> <td>J-1-7</td> <td></td> <td>505</td> <td>301</td> <td></td>	•		_	_				500	J-1-7		505	301	
Mov Cap-2 Maneuver - - - - 227 245 - 220 246 - Stage 1 - - - - - 563 551 - 556 540 - Stage 2 - - - - - 551 537 - 543 549 - Approach EB WB NB NB SB HCM Control Delay, s 0.1 0.2 13.8 21.7 HCM Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 SBLn2 Capacity (veh/h) 429 1087 - - 1085 - - 220 - HCM Lane V/C Ratio 0.044 0.004 - - 0.013 - - 0.02 - HCM Lane LOS B A - A - C A		1087		_	1085			227	245	590	220	246	598
Stage 1 - - - - 563 551 - 556 540 - Stage 2 - - - - - 551 537 - 543 549 - Approach EB WB NB NB SB HCM Control Delay, s 0.1 0.2 13.8 21.7 HCM LOS B C Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 SBLn2 Capacity (veh/h) 429 1087 - 1085 - 220 - HCM Lane V/C Ratio 0.044 0.004 - 0.013 - 0.013 - 0.02 - HCM Control Delay (s) 13.8 8.3 - 8.4 - 21.7 0 HCM Lane LOS B A - A - C A - C A - C A - C - C	•			_	-								
Stage 2 - - - - 551 537 - 543 549 - Approach EB WB NB SB HCM Control Delay, s 0.1 0.2 13.8 21.7 HCM LOS B C Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 SBLn2 Capacity (veh/h) 429 1087 - - 1085 - - 220 - HCM Lane V/C Ratio 0.044 0.004 - - 0.013 - - 0.02 - HCM Control Delay (s) 13.8 8.3 - - 8.4 - - 21.7 0 HCM Lane LOS B A - A - C A			_										
Approach EB WB NB SB HCM Control Delay, s 0.1 0.2 13.8 21.7 HCM LOS B C Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 SBLn2 Capacity (veh/h) 429 1087 - - 1085 - - 220 - HCM Lane V/C Ratio 0.044 0.004 - - 0.013 - - 0.02 - HCM Control Delay (s) 13.8 8.3 - - 8.4 - - 21.7 0 HCM Lane LOS B A - - A - C A	•		_	_	_	_	_						
HCM Control Delay, s	Olugo Z							001	501		J-10	070	
HCM Control Delay, s	Annragah	ED			WD			NID			CD		
Minor Lane/Major Mvmt NBLn1 EBL EBR WBL WBT WBR SBLn1 SBLn2 Capacity (veh/h) 429 1087 - - 1085 - - 220 - HCM Lane V/C Ratio 0.044 0.004 - - 0.013 - - 0.02 - HCM Control Delay (s) 13.8 8.3 - - 8.4 - - 21.7 0 HCM Lane LOS B A - - A - C A													
Minor Lane/Major Mvmt NBLn1 EBL EBR WBL WBT WBR SBLn1 SBLn2 Capacity (veh/h) 429 1087 - - 1085 - - 220 - HCM Lane V/C Ratio 0.044 0.004 - - 0.013 - - 0.02 - HCM Control Delay (s) 13.8 8.3 - - 8.4 - - 21.7 0 HCM Lane LOS B A - - A - C A		0.1			0.2								
Capacity (veh/h) 429 1087 1085 220 - HCM Lane V/C Ratio 0.044 0.004 0.013 0.02 - HCM Control Delay (s) 13.8 8.3 8.4 21.7 0 HCM Lane LOS B A - A - C A	HCM LOS							В			С		
Capacity (veh/h) 429 1087 1085 220 - HCM Lane V/C Ratio 0.044 0.004 0.013 0.02 - HCM Control Delay (s) 13.8 8.3 8.4 21.7 0 HCM Lane LOS B A - A - C A													
HCM Lane V/C Ratio 0.044 0.004 0.013 0.02 - HCM Control Delay (s) 13.8 8.3 8.4 21.7 0 HCM Lane LOS B A - A - C A	Minor Lane/Major Mvm	t I	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2		
HCM Lane V/C Ratio 0.044 0.004 - - 0.013 - - 0.02 - HCM Control Delay (s) 13.8 8.3 - - 8.4 - - 21.7 0 HCM Lane LOS B A - A - C A	Capacity (veh/h)		429	1087	-	-	1085	-	-	220	-		
HCM Control Delay (s) 13.8 8.3 8.4 21.7 0 HCM Lane LOS B A A C A			0.044	0.004	-	-	0.013	-	-	0.02	-		
HCM Lane LOS B A A C A	HCM Control Delay (s)		13.8	8.3	-	-		-	-	21.7	0		
			В		-	-	Α	-	-	С	Α		
	HCM 95th %tile Q(veh)		0.1	0	-	-	0	-	-	0.1	-		

HCM 2010 TWSC

Intersection						
Int Delay, s/veh	1.4					
Movement	NBL	NBR	NET	NER	SWL	SWT
				NEK		
Lane Configurations	<u>ን</u>	7	₽		\	140
Traffic Vol, veh/h	4	63	434	4	65	440
Future Vol, veh/h	4	63	434	4	65	440
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	100	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	_	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	4	70	482	4	72	489
IVIVIIIL FIUW	4	70	402	4	12	409
Major/Minor	Minor1	N	/lajor1		Major2	
Conflicting Flow All	1117	484	0	0	487	0
Stage 1	484	-			707	
•			-	-	-	-
Stage 2	633	-	-	-	1.10	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	229	583	-	-	1076	-
Stage 1	620	-	-	-	-	-
Stage 2	529	-	-	-	-	-
Platoon blocked, %	323		_	_		_
Mov Cap-1 Maneuver	214	583	_	_	1076	_
	347	505	_	_	1070	-
Mov Cap-2 Maneuver				-	-	
Stage 1	620	-	-	-	-	-
Stage 2	494	-	-	-	-	-
Approach	NB		NE		SW	
HCM Control Delay, s	12.2		0		1.1	
HCM LOS	В					
Minor Lane/Major Mvr	ot	NET	NEDI	NBLn11	VIBI no	SWL
	TI.					
Capacity (veh/h)		-	-	347	583	1076
HCM Lane V/C Ratio		-	-	0.013		0.067
HCM Control Delay (s)	-	-	15.5	12	8.6
HCM Lane LOS		-	-	С	В	Α
HCM 95th %tile Q(veh	1)	-	-	0	0.4	0.2

Intersection						
Int Delay, s/veh	0.9					
		A IV A I'D	NICT	NED	0\4#	OVA /T
Movement	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	À		ĵ.			
Traffic Vol, veh/h	40	18	440	57	12	465
Future Vol, veh/h	40	18	440	57	12	465
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	18	6	2	8	2	3
Mvmt Flow	42	19	463	60	13	489
	Minor1		/lajor1		Major2	
Conflicting Flow All	1008	493	0	0	523	0
Stage 1	493	-	-	-	-	-
Stage 2	515	-	-	-	-	-
Critical Hdwy	6.58	6.26	-	-	4.12	-
Critical Hdwy Stg 1	5.58	-	-	-	-	-
Critical Hdwy Stg 2	5.58	-	-	-	-	-
Follow-up Hdwy	3.662	3.354	-	-	2.218	-
Pot Cap-1 Maneuver	249	568	-	_	1043	-
Stage 1	582	-	_	_	-	_
Stage 2	568	_	_	_	_	-
Platoon blocked, %	300		_	_		_
Mov Cap-1 Maneuver	246	568	_		1043	_
Mov Cap-1 Maneuver	373	-	_		1070	
	582	-	-	-	-	-
Stage 1		-	-	-	-	-
Stage 2	561	-	-	-	-	-
Approach	NW		NE		SW	
HCM Control Delay, s	15.1		0		0.2	
HCM LOS	С					
Minor Lane/Major Mvm	nt	NET	NER	WLn1	SWL	SWT
Capacity (veh/h)		-	-	417	1043	-
HCM Lane V/C Ratio		-	-	0.146	0.012	-
HCM Control Delay (s)		-	-	15.1	8.5	-
HCM Lane LOS		-	-	С	Α	-
HCM 95th %tile Q(veh)	-	-	0.5	0	-

Intersection												
Int Delay, s/veh	0.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ች	ĵ.		ሻ	ĵ.			4		*	ĵ.	
Traffic Vol, veh/h	2	530	2	6	791	7	8	0	23	7	0	1
Future Vol, veh/h	2	530	2	6	791	7	8	0	23	7	0	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	_	None	_	-	None	_	-	None
Storage Length	125	-	_	0	_	-	-	-	-	75	-	_
Veh in Median Storage		0	-	-	0	_	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	90	90	95	95	90	90	90	95	90	95
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	28	2	2
Mvmt Flow	2	558	2	7	833	7	9	0	26	7	0	1
Major/Minor	Major1		ı	Major2			Minor1			Minor2		
Conflicting Flow All	840	0	0	560	0	0	1413	1416	559	1426	1414	836
Stage 1	040	-	<u> </u>	500	-	-	563	563	559	850	850	- 030
Stage 2		_	_		_	_	850	853	_	576	564	_
Critical Hdwy	4.12	-	<u>-</u>	4.12	_	-	7.12	6.52	6.22	7.38	6.52	6.22
Critical Hdwy Stg 1	7.12	_	_	7.12	_		6.12	5.52	0.22	6.38	5.52	0.22
Critical Hdwy Stg 2	_	-		-	-	-	6.12	5.52	-		5.52	-
Follow-up Hdwy	2.218	_	_	2.218	_		3.518	4.018	3.318	3.752	4.018	3.318
Pot Cap-1 Maneuver	795	_	_	1011	_	_	115	137	529	99	138	367
Stage 1		_	_	-	_	_	511	509	JZ3 -	321	377	- 301
Stage 2		_	_	_	_	_	355	376	_	460	508	_
Platoon blocked, %		_	_		_	_	000	010		-100	500	
Mov Cap-1 Maneuver	795	_	_	1011	_	_	114	136	529	94	137	367
Mov Cap-1 Maneuver	133	_	_	-	_	_	114	136	-	94	137	- 301
Stage 1		_	_	_	_	_	510	508			374	_
Stage 2	_	_	_	_	_	_	352	373	_	437	507	_
Olago Z							002	010		-101	307	
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.1			20.1			42.5		
HCM LOS	U			0.1			20.1			42.5 E		
I IOIVI LOG							U					
Minor Lane/Major Mvm	nt I	NBLn1	EBL	EBT	EBR	WBL	WBT	WRR	SBLn1	SBI n2		
Capacity (veh/h)		273	795			1011	-	WDI(94	367		
HCM Lane V/C Ratio		0.126		-		0.007		-	0.078			
HCM Control Delay (s)		20.1	9.5		-	8.6	-	-	46.5	14.8		
HCM Lane LOS		20.1 C	9.5 A	-		0.0 A	-		40.5 E	14.0 B		
HCM 95th %tile Q(veh)	١	0.4	0	-	-	0	-	-	0.2	0		
TIOW 35th 76the Q(Ven)	0.4	U	-	_	U	-	-	0.2	U		

Int Delay, s/veh	1.4							
Movement	NBL	NBR	NET	NER	SWL	SWT		
Lane Configurations	ኘ	7	1		ሻ	<u> </u>		
Traffic Vol, veh/h	8	113	558	2	29	806		
Future Vol, veh/h	8	113	558	2	29	806		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-	None		
Storage Length	50	0	_	-	200	-		
Veh in Median Storage		_	0	-		0		
Grade, %	0	_	0	-	_	0		
Peak Hour Factor	90	90	90	90	90	90		
Heavy Vehicles, %	2	2	2	2	2	2		
Mvmt Flow	9	126	620	2	32	896		
Major/Minor	Minor1	N	Major1		Major			
			Major1		Major2			
Conflicting Flow All	1581	621	0	0	622	0		
Stage 1	621	-	-	-	-	-		
Stage 2	960 6.42	6.22	-	-	4.12	-		
Critical Hdwy	5.42	0.22	-	-	4.12	-		
Critical Hdwy Stg 1	5.42		-	-	-	-		
Critical Hdwy Stg 2 Follow-up Hdwy	3.518	3.318	-	-	2.218	-		
Pot Cap-1 Maneuver	120	487	-		959			
Stage 1	536	407	-	-	909	-		
Stage 2	372	-	_	-	-	-		
Platoon blocked, %	312	-	-	_	-	_		
Mov Cap-1 Maneuver	116	487	-	_	959	-		
Mov Cap-1 Maneuver	247	407	_	_	303	_		
Stage 1	536		_		-	-		
Stage 2	360	-	_	_	_	_		
Stage 2	300	-	-	-	-	-		
Approach	NB		NE		SW			
HCM Control Delay, s	15.2		0		0.3			
HCM LOS	С							
Minor Lane/Major Mvm	nt	NET	NER	NBLn11	NBLn2	SWL	SWT	
Capacity (veh/h)		-		247	487	959	-	
HCM Lane V/C Ratio		_	_		0.258		_	
HOM COLL 15 1			_	0.000	0.200	0.004		

20.1

С

0.1

14.9

В

8.9

0.1

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HCM Control Delay (s)

HCM 95th %tile Q(veh)

HCM Lane LOS

Intersection						
Int Delay, s/veh	3.2					
		NIVATO	NIET	NED	0)4//	OVACE
Movement	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	Y		\$		<u>ች</u>	↑
Traffic Vol, veh/h	118	40	607	66	6	716
Future Vol, veh/h	118	40	607	66	6	716
Conflicting Peds, #/hr	0	0	0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	200	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	5	4	2	6	2	2
Mvmt Flow	127	43	653	71	6	770
Majar/Minar	Minard		Asiau1		Anin nO	
	Minor1		Major1		Major2	
Conflicting Flow All	1471	688	0	0	724	0
Stage 1	688	-	-	-	-	-
Stage 2	783	-	-	-	-	-
Critical Hdwy	6.45	6.24	-	-	4.12	-
Critical Hdwy Stg 1	5.45	-	-	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-	-	-
Follow-up Hdwy	3.545	3.336	-	-	2.218	-
Pot Cap-1 Maneuver	138	443	-	-	879	-
Stage 1	493	-	-	-	-	-
Stage 2	445	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	137	443	-	-	879	-
Mov Cap-2 Maneuver	275	-	-	-	-	-
Stage 1	493	-	-	-	-	-
Stage 2	442	-	-	-	-	-
ŭ						
Δ	N IVA		NIE		0)44	
Approach	NW		NE		SW	
HCM Control Delay, s	30.9		0		0.1	
HCM LOS	D					
Minor Lane/Major Mvm	nt	NET	NFRN	IWLn1	SWL	SWT
Capacity (veh/h)		-		304	879	-
HCM Lane V/C Ratio		_		0.559		_
HCM Control Delay (s)			_	30.9	9.1	_
HCM Lane LOS		-	_	30.9 D	9.1 A	-
HCM 95th %tile Q(veh	\	_	<u>-</u>	3.2	0	_
HOW JULY WILL WINE	1	-	-	J.Z	U	-

Intersection Int Delay, s/veh O.4 Movement EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR SBT SBR SBT SBR SBT SBR SBT SBR SBT SBR SBT S
Movement EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR
Lane Configurations
Traffic Vol, veh/h
Traffic Vol, veh/h
Conflicting Peds, #/hr O O O O O O O O O
Sign Control Free
RT Channelized - None - 755 - - O - - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 0 90
RT Channelized - None - O - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 0 90
Veh in Median Storage, # 0 - 0 0 - 0 0 - 0 0 - 0 0 90
Veh in Median Storage, # 0 - 0 0 - 0 0 - 0 0 - 0 0 90
Grade, % - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - 0 0 - 0 0 90 <t< td=""></t<>
Peak Hour Factor 90 90 90 90 90 90 90 9
Mynt Flow 1 958 7 21 394 9 1 0 2 6 0 0 Major/Minor Major1 Major2 Minor1 Minor2 Conflicting Flow All 403 0 0 964 0 0 1404 1409 961 1405 1408 399 Stage 1 - - - - - - 963 963 - 441 441 - Stage 2 - - - - 963 963 - 441 441 - 964 967 - - - - 441 446 - 964 967 -
Mymt Flow 1 958 7 21 394 9 1 0 2 6 0 0 Major/Minor Major1 Major2 Minor1 Minor2 Conflicting Flow All 403 0 0 964 0 0 1404 1409 961 1405 1408 399 Stage 1 - - - - 963 963 - 441 441 - 964 967 - Critical Hdwy 4.12 - - 4.12 - 7.12 6.52 6.22 7.5 6.52 6.22 Critical Hdwy Stg 1 - - - - 6.12 5.52 - 6.5 5.52 - Critical Hdwy Stg 2 - - - - 6.12 5.52 - 6.5 5.52 - Follow-up Hdwy 2.218 - - 2.218 - - 3.518 4.018
Conflicting Flow All 403 0 0 964 0 0 1404 1409 961 1405 1408 399 Stage 1 - - - - - 963 963 - 441 441 - Stage 2 - - - - - 441 446 - 964 967 - Critical Hdwy 4.12 - - 4.12 - - 7.12 6.52 6.22 7.5 6.52 6.22 Critical Hdwy Stg 1 - - - - - 6.12 5.52 - 6.5 5.52 - Critical Hdwy Stg 2 - - - - 6.12 5.52 - 6.5 5.52 - Critical Hdwy Stg 2 - - - - 6.12 5.52 - 6.5 5.52 - Critical Hdwy Stg 2 - - - - 6.12 5.52 - 6.5 5.52 - 6.5 5.52 -
Conflicting Flow All 403 0 0 964 0 0 1404 1409 961 1405 1408 399 Stage 1 963 963 - 441 441 - Stage 2 441 446 - 964 967 - Critical Hdwy 4.12 - 4.12 7.12 6.52 6.22 7.5 6.52 6.22 Critical Hdwy Stg 1 6.12 5.52 - 6.5 5.52 - Critical Hdwy Stg 2 6.12 5.52 - 6.5 5.52 - Follow-up Hdwy 2.218 - 2.218 - 3.518 4.018 3.318 3.86 4.018 3.318 Pot Cap-1 Maneuver 1156 - 714 - 117 139 311 97 139 651 Stage 1 595 574 - 262 333 - Platoon blocked, % Mov Cap-1 Maneuver 1156 - 714 - 114 134 311 93 134 651 Mov Cap-2 Maneuver 114 134 311 93 134 - Stage 1 572 552 - 260 333 - Stage 2 572 552 - 260 333 Stage 2 572 552 - 260 333 Stage 2 572 552 - 260 333
Stage 1 - - - - 963 963 - 441 441 - Stage 2 - - - - - 441 446 - 964 967 - Critical Hdwy 4.12 - - 4.12 - - 7.12 6.52 6.22 7.5 6.52 6.22 Critical Hdwy Stg 1 - - - - 6.12 5.52 - 6.5 5.52 - Critical Hdwy Stg 2 - - - - 6.12 5.52 - 6.5 5.52 - Critical Hdwy Stg 1 - - - - 6.12 5.52 - 6.5 5.52 - Critical Hdwy Stg 2 - - - 6.12 5.52 - 6.5 5.52 - 6.5 5.52 - 6.5 5.52 - 6.5 5.52 - 6.5 5.52 - 6.5 5.52 - 6.5 5.52 - 6.5 5.52
Stage 1 - - - - 963 963 - 441 441 - Stage 2 - - - - - 441 446 - 964 967 - Critical Hdwy 4.12 - - 4.12 - - 7.12 6.52 6.22 7.5 6.52 6.22 Critical Hdwy Stg 1 - - - - 6.12 5.52 - 6.5 5.52 - Critical Hdwy Stg 2 - - - - 6.12 5.52 - 6.5 5.52 - Critical Hdwy Stg 1 - - - - 6.12 5.52 - 6.5 5.52 - Critical Hdwy Stg 2 - - - 6.12 5.52 - 6.5 5.52 - 6.5 5.52 - - 6.5 5.52 - - 6.5 5.52 - - 6.5 5.52 - - 6.5 5.52 - - 6.5
Stage 2 - - - - 441 446 - 964 967 - Critical Hdwy 4.12 - - 4.12 - - 7.12 6.52 6.22 7.5 6.52 6.22 Critical Hdwy Stg 1 - - - - - 6.12 5.52 - 6.5 5.52 - Critical Hdwy Stg 2 - - - - 6.12 5.52 - 6.5 5.52 - Follow-up Hdwy 2.218 - 2.218 - 3.518 4.018 3.318 3.86 4.018 3.318 Pot Cap-1 Maneuver 1156 - 714 - 117 139 311 97 139 651 Stage 1 - - - - - - 595 574 - 262 333 - Platoon blocked, % - - - - - 114 134 311 93 134 651 Mov Cap-1 Maneuver 1156<
Critical Hdwy 4.12 - 4.12 - 7.12 6.52 6.22 7.5 6.52 6.22 Critical Hdwy Stg 1 - - - - 6.12 5.52 - 6.5 5.52 - Critical Hdwy Stg 2 - - - - 6.12 5.52 - 6.5 5.52 - Follow-up Hdwy 2.218 - - 2.218 - 3.518 4.018 3.318 3.86 4.018 3.318 Pot Cap-1 Maneuver 1156 - 714 - 117 139 311 97 139 651 Stage 1 - - - - - 595 574 - 262 333 - Platoon blocked, % -
Critical Hdwy Stg 1 - - - - 6.12 5.52 - 6.5 5.52 - Critical Hdwy Stg 2 - - - - 6.12 5.52 - 6.5 5.52 - Follow-up Hdwy 2.218 - - 2.218 - - 3.518 4.018 3.318 3.86 4.018 3.318 Pot Cap-1 Maneuver 1156 - 714 - 117 139 311 97 139 651 Stage 1 - - - - - 595 574 - 262 333 - Platoon blocked, % - </td
Critical Hdwy Stg 2 - - - - 6.12 5.52 - 6.5 5.52 - Follow-up Hdwy 2.218 - - 2.218 - - 3.518 4.018 3.318 3.86 4.018 3.318 Pot Cap-1 Maneuver 1156 - 714 - 117 139 311 97 139 651 Stage 1 - - - - - - 528 577 - Stage 2 - - - - - 595 574 - 262 333 - Platoon blocked, % -
Follow-up Hdwy 2.218 2.218 3.518 4.018 3.318 3.86 4.018 3.318 Pot Cap-1 Maneuver 1156 - 714 - 117 139 311 97 139 651 Stage 1 307 334 - 528 577 - Stage 2 595 574 - 262 333 - Platoon blocked, % Mov Cap-1 Maneuver 1156 - 714 - 114 134 311 93 134 651 Mov Cap-2 Maneuver 114 134 311 93 134 - Stage 1 307 334 - 528 555 - Stage 2 572 552 - 260 333 -
Pot Cap-1 Maneuver 1156 - - 714 - - 117 139 311 97 139 651 Stage 1 - - - - - 307 334 - 528 577 - Stage 2 - - - - - 595 574 - 262 333 - Platoon blocked, % -
Stage 2 - - - - - 595 574 - 262 333 - Platoon blocked, % - <t< td=""></t<>
Platoon blocked, % - - - - Mov Cap-1 Maneuver 1156 - 714 - 114 134 311 93 134 651 Mov Cap-2 Maneuver - - - - - 114 134 - 93 134 - Stage 1 - - - - - 307 334 - 528 555 - Stage 2 - - - - - - 572 552 - 260 333 - Approach EB WB NB SB HCM Control Delay, s 0 0.5 23.6 46.2
Mov Cap-1 Maneuver 1156 - - 714 - - 114 134 311 93 134 651 Mov Cap-2 Maneuver - - - - - 114 134 - 93 134 - Stage 1 - - - - - 307 334 - 528 555 - Stage 2 - - - - - - 572 552 - 260 333 - Approach EB WB NB SB HCM Control Delay, s 0 0.5 23.6 46.2
Mov Cap-2 Maneuver - - - - - 114 134 - 93 134 - Stage 1 - - - - - 307 334 - 528 555 - Stage 2 - - - - - 572 552 - 260 333 - Approach EB WB NB SB HCM Control Delay, s 0 0.5 23.6 46.2
Stage 1 - - - - - 307 334 - 528 555 - Stage 2 - - - - - - 572 552 - 260 333 - Approach EB WB NB SB HCM Control Delay, s 0 0.5 23.6 46.2
Stage 2 - - - - 572 552 - 260 333 - Approach EB WB NB SB HCM Control Delay, s 0 0.5 23.6 46.2
Approach EB WB NB SB HCM Control Delay, s 0 0.5 23.6 46.2
HCM Control Delay, s 0 0.5 23.6 46.2
HCM Control Delay, s 0 0.5 23.6 46.2
HCM LOS C E
Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 SBLn2
Capacity (veh/h) 197 1156 714 93 -
HCM Lane V/C Ratio 0.017 0.001 0.03 0.06 -
HCM Control Delay (s) 23.6 8.1 10.2 0 - 46.2 0
HCM Lane LOS C A B A - E A
HCM 95th %tile Q(veh) 0.1 0 0.1 0.2 -

Intersection							
Int Delay, s/veh	1						
Movement	NBL	NBR	NET	NER	SWL	SWT	
Lane Configurations	ሻ	7	f)			4	
Traffic Vol, veh/h	1	12	863	6	95	379	
Future Vol, veh/h	1	12	863	6	95	379	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-			None	
Storage Length	50	0	-	_	-	-	
Veh in Median Storage		-	0	-	-	0	
Grade, %	0	_	0	_	_	0	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	1	13	959	7	106	421	
N A - 1 /N A1	M				M. '. O		
	Minor1		Major1		Major2		
Conflicting Flow All	1594	962	0	0	966	0	
Stage 1	962	-	-	-	-	-	
Stage 2	632	-	-	-	-	-	
Critical Hdwy	6.42	6.22	-	-	4.12	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518		-	-	2.218	-	
Pot Cap-1 Maneuver	118	310	-	-	713	-	
Stage 1	371	-	-	-	-	-	
Stage 2	530	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	95	310	-	-	713	-	
Mov Cap-2 Maneuver	95	-	-	-	-	-	
Stage 1	371	-	-	-	-	-	
Stage 2	427	-	-	-	-	-	
Approach	NB		NE		SW		
HCM Control Delay, s	19.1		0		2.2		
HCM LOS	13.1 C		U		۷.۷		
TIOWI LOO	J						
Minor Long/Major Mar	at .	NET	NED	NIDI 511	MDI ~2	CIVII	
Minor Lane/Major Mvm	IL	NET	NEK	NBLn1 I			
Capacity (veh/h)		-	-	95	310	713	
HCM Lane V/C Ratio		-	-	0.012			
HCM Control Delay (s))	-	-	43.3	17.1	10.9	

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HCM Lane LOS

HCM 95th %tile Q(veh)

С

0.1

В

0.5

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Intersection						
Int Delay, s/veh	2.3					
					01:"	01:
Movement	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	Y		ĵ»			ન
Traffic Vol, veh/h	51	22	787	95	12	422
Future Vol, veh/h	51	22	787	95	12	422
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	8	14	2	2	17	7
Mymt Flow	56	24	865	104	13	464
WWW.CT IOW	00	- 1	000	101	10	101
Major/Minor	Minor1	N	/lajor1	N	Major2	
Conflicting Flow All	1407	917	0	0	969	0
Stage 1	917	-	-	-	-	-
Stage 2	490	-	-	-	-	-
Critical Hdwy	6.48	6.34	-	_	4.27	_
Critical Hdwy Stg 1	5.48	-	_	_	-	_
Critical Hdwy Stg 2	5.48	_	_	_	_	_
Follow-up Hdwy	3.572		_	_	2.353	_
Pot Cap-1 Maneuver	149	313	_		654	_
Stage 1	380	-		_	- 004	_
Stage 2	604	_	_		_	
	004	-	-	-	-	
Platoon blocked, %	4.45	242	-	-	CE 4	-
Mov Cap-1 Maneuver		313	-	-	654	-
Mov Cap-2 Maneuver	145	-	-	-	-	-
Stage 1	380	-	-	-	-	-
Stage 2	588	-	-	-	-	-
Approach	NW		NE		SW	
HCM Control Delay, s	42.6		0		0.3	
HCM LOS	42.0 E		U		0.5	
I IOIVI LOG						
Minor Lane/Major Mvr	nt	NET	NERN	WLn1	SWL	SWT
Capacity (veh/h)			_	173	654	-
HCM Lane V/C Ratio		-	-	0.464	0.02	-
HCM Control Delay (s)	_	-	42.6	10.6	0
HCM Lane LOS	,	_	_	E	В	A
HCM 95th %tile Q(veh	1)	_	_	2.2	0.1	-
HOW JOHN JOHNE Q(VEI	'/	_	_	۷.۷	U. I	_

Intersection							
Int Delay, s/veh	1.8						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	J
Lane Configurations	7	T T	NDL	† †	↑	אופט	
Traffic Vol, veh/h	3	99	105	TT 580	1128	135	
Future Vol, veh/h	3	99	105	580	1128	135	
Conflicting Peds, #/hr	10	0	103	0	0	133	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	Stop -	None	-	None	-	None	
Storage Length	125	0	0	INOITE	_	INUITE	
Veh in Median Storage		-	-	0	0	-	
Grade, %	0	-	-	0	0	_	
Peak Hour Factor	93	93	93	93	93	93	
Heavy Vehicles, %	6	6	6	6	3	3	
Mvmt Flow	3	106	113	624	1213	145	
Major/Minor I	Minor2	N	Major1	ľ	Major2		
Conflicting Flow All	1834	680	1359	0		0	
Stage 1	1286	-	-	_	-	-	
Stage 2	548	_	_	_	_	_	
Critical Hdwy	6.92	7.02	4.22	_	_	_	
Critical Hdwy Stg 1	5.92	- 1.02	- 1.22	_	_	_	
Critical Hdwy Stg 2	5.92	_	_	_	_	_	
Follow-up Hdwy	3.56	3.36	2.26	_	_	_	
Pot Cap-1 Maneuver	65	384	481	_	_	_	
Stage 1	216	-	-	_	_	_	
Stage 2	532	_	_	_	_	_	
Platoon blocked, %	JJZ	_	_	_	_	_	
	50	384	481	-	-	_	
Mov Cap-1 Maneuver	50	J04 -					
Mov Cap-2 Maneuver		-	-	-	-	-	
Stage 1	216	-	-	-	-	-	
Stage 2	407	-	-	-	-	-	
Approach	EB		NB		SB		
HCM Control Delay, s	19.8		2.3		0		
HCM LOS	С						
Minor Long/Major Mym	1	NDI	NDT	EDL 51 I	רמ וחד	CDT	
Minor Lane/Major Mvm	I	NBL	INRI	EBLn1 I		SBT	
Capacity (veh/h)		481	-	50	384	-	
HCM Lane V/C Ratio		0.235	-	0.065		-	
HCM Control Delay (s)		14.8	-		17.9	-	
HCM Lane LOS		В	-	F	С	-	
HCM 95th %tile Q(veh)		0.9	-	0.2	1.1	-	

Intersection												
Int Delay, s/veh	0.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ች	f)			4			4		ሻ	f)	
Traffic Vol, veh/h	4	425	4	13	414	8	4	0	13	4	0	0
Future Vol, veh/h	4	425	4	13	414	8	4	0	13	4	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	_	-	None	-	_	None	_	_	None	_	-	None
Storage Length	125	-	-	-	_	_	-	_	_	75	-	_
Veh in Median Storage		0	-	-	0	_	-	0	-	_	0	-
Grade, %	, -	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	3	3	2	2	4	4	2	2	2	2	2	2
Mvmt Flow	4	472	4	14	460	9	4	0	14	4	0	0
Major/Minor I	Major1		ı	Major2		- 1	Minor1			Minor2		
Conflicting Flow All	469	0	0	477	0	0	976	981	474	984	979	464
Stage 1	409	-	U	4//	-	-	483	483	4/4	493	493	404
Stage 2	-	-	-	-	-	-	493	498	-	493	493	-
Critical Hdwy	4.13	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	7.13	_		7.12	_	_	6.12	5.52	0.22	6.12	5.52	0.22
Critical Hdwy Stg 2	_	-	_	-	-	-	6.12	5.52	-		5.52	_
Follow-up Hdwy	2.227	_	_	2.218	_	_	3.518		3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1087	_	_	1085	_	_	230	249	590	228	250	598
Stage 1	-	_	_	-	_	_	565	553	-	558	547	-
Stage 2	_	_	_	_	_	_	558	544		559	551	_
Platoon blocked, %		_	_		_	_	000	UT7		000	001	
Mov Cap-1 Maneuver	1087	_	_	1085	_	_	226	244	590	219	245	598
Mov Cap-1 Maneuver	-	_	_	-	_	_	226	244	-	219	245	-
Stage 1	_	_	_	_	_	_	563	551	_		538	_
Stage 2	_	_	_	_	_	_	549	535	<u>-</u>	543	549	_
Slago Z							3-13	500		3-10	3-13	
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			0.2			13.8			21.8		
HCM LOS	0.1			0.2			13.0 B			Z1.0		
TIOWI LOO							U			J		
Minor Lane/Major Mvm	ıt l	NBLn1	EBL	EBT	EBR	WBL	WBT	WRR	SBLn1	SBI n2		
Capacity (veh/h)		428	1087			1085	-		219			
HCM Lane V/C Ratio		0.044		_		0.013	_	_	0.02	_		
HCM Control Delay (s)		13.8	8.3	_	_	8.4	0	_	21.8	0		
HCM Lane LOS		13.0 B	Α	_	_	Α	A	_	Z1.0	A		
HCM 95th %tile Q(veh)	1	0.1	0	_	-	0	-	_	0.1	-		
TOWN COURT FOUND COLVERN		0.1	0			J			0.1			

HCM 2010 TWSC

Intersection							
Int Delay, s/veh	1.4						
		NDD	NICT	NED	014/	014/7	
Movement	NBL	NBR	NET	NER	SWL	SWT	
Lane Configurations	<u></u>	7	ĵ,			્રની	
Traffic Vol, veh/h	4	63	434	4	65	440	
Future Vol, veh/h	4	63	434	4	65	440	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	0	-	-	-	-	
Veh in Median Storage	e,# 0	-	0	-	-	0	
Grade, %	0	-	0	-	-	0	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	4	70	482	4	72	489	
NA = : = =/NA:= :	NA: A		A-!. A		M-: 0		Į
	Minor1		Major1		Major2		
Conflicting Flow All	1117	484	0	0	487	0	
Stage 1	484	-	-	-	-	-	
Stage 2	633	-	-	-	-	-	
Critical Hdwy	6.42	6.22	-	-	4.12	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	-	-	2.218	-	
Pot Cap-1 Maneuver	229	583	-	-	1076	-	
Stage 1	620	-	-	-	-	-	
Stage 2	529	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	208	583	-	-	1076	-	
Mov Cap-2 Maneuver	208	-	-	_	_	-	
Stage 1	620	-	-	_	-	-	
Stage 2	480	_	-	_	-	-	
J							
A	ND		NIE		CVA		
Approach	NB		NE		SW		
HCM Control Delay, s	12.6		0		1.1		
HCM LOS	В						
Minor Lane/Major Mvr	nt	NET	NERI	NBLn1 I	NBLn2	SWL	
Capacity (veh/h)				208	583	1076	
HCM Lane V/C Ratio		_	_	0.021		0.067	
HCM Control Delay (s)			22.7	12	8.6	
HCM Lane LOS		_		C	В	Α	
HCM 95th %tile Q(veh	1)	_	_	0.1	0.4	0.2	
TION 3301 /000E Q(VEI	1)	-	-	0.1	0.4	U.Z	

Intersection						
Int Delay, s/veh	1.2					
		NIME	NET	NED	CVVII	CMT
Movement	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	Y	40	^		40	4
Traffic Vol, veh/h	40	18	440	57	12	465
Future Vol, veh/h	40	18	440	57	12	465
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	18	6	2	8	2	3
Mvmt Flow	42	19	463	60	13	489
in the contract of the contrac			100	00		100
Major/Minor	Minor1	N	/lajor1	l	Major2	
Conflicting Flow All	1008	493	0	0	523	0
Stage 1	493	-	-	-	-	-
Stage 2	515	-	-	-	-	-
Critical Hdwy	6.58	6.26	_	_	4.12	_
Critical Hdwy Stg 1	5.58	-	_	_	-	_
Critical Hdwy Stg 2	5.58	_	_	_	_	_
Follow-up Hdwy	3.662		_	<u>_</u>	2.218	_
Pot Cap-1 Maneuver	249	568		_	1043	_
•	582	-	_	_	1043	_
Stage 1	568		-	_	-	
Stage 2	200	-	-	-	-	-
Platoon blocked, %	0.1=		-	-	1010	-
Mov Cap-1 Maneuver		568	-	-	1043	-
Mov Cap-2 Maneuver	245	-	-	-	-	-
Stage 1	582	-	-	-	-	-
Stage 2	558	-	-	-	-	-
Approach	NW		NE		SW	
HCM Control Delay, s	20.2		0		0.2	
			U		0.2	
HCM LOS	С					
Minor Lane/Major Mvn	nt	NET	NERN	IWLn1	SWL	SWT
Capacity (veh/h)		_	_	298	1043	_
HCM Lane V/C Ratio		_	_	0.205		-
HCM Control Delay (s)	_	_	20.2	8.5	0
HCM Lane LOS)	-		20.2 C	0.5 A	A
	.\	-	-			
HCM 95th %tile Q(veh	1)	-	-	8.0	0	-

Int Delay, s/veh
Movement EBL EBR NBL NBT SBT SBR Lane Configurations Image: Configuration of the process of the pr
Lane Configurations Image: Configuration of the confi
Traffic Vol, veh/h 21 77 94 640 658 31 Future Vol, veh/h 21 77 94 640 658 31 Conflicting Peds, #/hr 1 0 1 0 0 1 Sign Control Stop Stop Free Free Free Free Free RT Channelized - None - - 0 0 - - - 0 0 - - - 0 0 - - - 0 0 - 92 92 92 92 92 92<
Future Vol, veh/h 21 77 94 640 658 31 Conflicting Peds, #/hr 1 0 1 0 0 1 Sign Control Stop Stop Stop Free Free Free Free RT Channelized - None - None - None Storage Length 125 0 0 - - - Veh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 92 92 92 92 92 92 Heavy Vehicles, % 5 5 6 6 3 3 Mvmt Flow 23 84 102 696 715 34 Major/Minor Minor2 Major1 Major2 Conflicting Flow All 1286 375 750 0 - 0 Stage 1
Conflicting Peds, #/hr 1 0 1 0 0 1 Sign Control Stop Stop Free
Sign Control Stop Stop Free Room None 125 0 0 - - 0 0 - - - 0 0 - - - 0 0 - - - 0 0 - - - - 92
RT Channelized - None - None - None Storage Length 125 0 0 Veh in Median Storage, # 0 0 0 0 0 Grade, % 0 0 0 0 0 Peak Hour Factor 92
Storage Length 125 0 0 - - - Veh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 92 92 92 92 92 92 Heavy Vehicles, % 5 5 6 6 3 3 Mvmt Flow 23 84 102 696 715 34 Major/Minor Minor2 Major1 Major2 Conflicting Flow All 1286 375 750 0 - 0 Stage 1 733 - - - - - - Stage 2 553 - - - - - - Critical Hdwy 6.9 7 4.22 - - - Critical Hdwy Stg 1 5.9 - - - - - Critical Hdwy S
Weh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 92 92 92 92 92 92 Heavy Vehicles, % 5 5 6 6 3 3 Mvmt Flow 23 84 102 696 715 34 Major/Minor Minor2 Major1 Major2 Conflicting Flow All 1286 375 750 0 - 0 Stage 1 733 - - - - - - Stage 2 553 - - - - - - Critical Hdwy 6.9 7 4.22 - - - - Critical Hdwy Stg 1 5.9 - - - - - - Critical Hdwy Stg 2 5.9 - - - - -
Grade, % 0 - - 0 0 - Peak Hour Factor 92
Peak Hour Factor 92 93 93 93 93 94 92 93 93 93
Major/Minor Minor2 Major1 Major2 Conflicting Flow All 1286 375 750 0 - 0 Stage 1 733 - - - - - - Stage 2 553 - - - - - - Critical Hdwy 6.9 7 4.22 - - - - Critical Hdwy Stg 1 5.9 - - - - - - - - Critical Hdwy Stg 2 5.9 - <td< td=""></td<>
Mvmt Flow 23 84 102 696 715 34 Major/Minor Minor2 Major1 Major2 Conflicting Flow All 1286 375 750 0 - 0 Stage 1 733 - - - - - Stage 2 553 - - - - - Critical Hdwy 6.9 7 4.22 - - - Critical Hdwy Stg 1 5.9 - - - - - Critical Hdwy Stg 2 5.9 - - - - -
Major/Minor Minor2 Major1 Major2 Conflicting Flow All 1286 375 750 0 - 0 Stage 1 733 -
Conflicting Flow All 1286 375 750 0 - 0 Stage 1 733 - - - - - Stage 2 553 - - - - - Critical Hdwy 6.9 7 4.22 - - - Critical Hdwy Stg 1 5.9 - - - - - Critical Hdwy Stg 2 5.9 - - - - -
Conflicting Flow All 1286 375 750 0 - 0 Stage 1 733 - - - - - Stage 2 553 - - - - - Critical Hdwy 6.9 7 4.22 - - - Critical Hdwy Stg 1 5.9 - - - - - Critical Hdwy Stg 2 5.9 - - - - -
Conflicting Flow All 1286 375 750 0 - 0 Stage 1 733 - - - - - Stage 2 553 - - - - - Critical Hdwy 6.9 7 4.22 - - - Critical Hdwy Stg 1 5.9 - - - - - Critical Hdwy Stg 2 5.9 - - - - -
Stage 1 733 -
Stage 2 553 - - - - Critical Hdwy 6.9 7 4.22 - - Critical Hdwy Stg 1 5.9 - - - - Critical Hdwy Stg 2 5.9 - - - -
Critical Hdwy 6.9 7 4.22 - - - Critical Hdwy Stg 1 5.9 - - - - Critical Hdwy Stg 2 5.9 - - - -
Critical Hdwy Stg 1 5.9 Critical Hdwy Stg 2 5.9
Critical Hdwy Stg 2 5.9
, ,
Follow-up Hdwy 3.55 3.35 2.26
Pot Cap-1 Maneuver 152 614 829
Stage 1 428
Stage 2 532
Platoon blocked, %
Mov Cap-1 Maneuver 133 613 829
Mov Cap-2 Maneuver 133
Stage 1 428
Stage 2 466
- 100 - 100
Approach EB NB SB
HCM Control Delay, s 17.3 1.3 0
HCM LOS C
Minor Land/Major Mumt NDL NDT EDL 51 EDL 52 CDT
Minor Lane/Major Mvmt NBL NBT EBLn1 EBLn2 SBT
Capacity (veh/h) 829 - 133 613 -
HCM Lane V/C Ratio 0.123 - 0.172 0.137 -
HCM Control Delay (s) 10 - 37.6 11.8 -
HCM Lane LOS A - E B -
HCM 95th %tile Q(veh) 0.4 - 0.6 0.5 -

Intersection												
Int Delay, s/veh	0.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ች	f)			4			4		ሻ	f)	
Traffic Vol, veh/h	2	530	2	6	791	7	8	0	23	7	0	1
Future Vol, veh/h	2	530	2	6	791	7	8	0	23	7	0	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	125	_	-	_	_	-	_	_	-	75	_	-
Veh in Median Storage		0	-	-	0	-	-	0	_	-	0	-
Grade, %	-	0	-	-	0	-	-	0	_	_	0	_
Peak Hour Factor	95	95	90	90	95	95	90	90	90	95	90	95
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	28	2	2
Mvmt Flow	2	558	2	7	833	7	9	0	26	7	0	1
Major/Minor	Major1			Major			Minor1			Minor2		
	Major1	^		Major2	^			1440			1111	000
Conflicting Flow All	840	0	0	560	0	0	1413	1416	559	1426	1414	836
Stage 1	-	-	-	-	-	-	563	563	-	850	850	-
Stage 2	4.40	-	-	4.40	-	-	850	853	- 6.00	576	564	6.00
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.38	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.38	5.52	-
Critical Hdwy Stg 2	2 240	-	-	2 240	-	-	6.12	5.52	2 240	6.38	5.52	2 240
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.752	4.018	3.318
Pot Cap-1 Maneuver	795	-	-	1011	-	-	115	137	529	99	138	367
Stage 1	-	-	-	-	-	-	511	509	-	321	377	-
Stage 2	-	-	-	-	-	-	355	376	-	460	508	-
Platoon blocked, %	705	-	-	1011	-	-	112	125	529	02	126	367
Mov Cap-1 Maneuver	795	-	-	1011	-	-	113	135		93	136	307
Mov Cap-2 Maneuver	-	-	-	-	-	-	113 510	135 508	-	93 320	136 372	-
Stage 1	-	-	-	-	-	-	349	371	-	437	507	-
Stage 2	-	-	-	_	_	-	349	3/1	-	437	507	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.1			20.2			43		
HCM LOS							С			Е		
Minor Lane/Major Mvm	nt I	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2		
Capacity (veh/h)		271	795			1011	_		93	367		
HCM Lane V/C Ratio		0.127		_		0.007	_	_	0.079			
HCM Control Delay (s)		20.2	9.5	_	_	8.6	0	_	47	14.8		
HCM Lane LOS		C	Α.	_	_	Α	A	_	E	В		
HCM 95th %tile Q(veh)	0.4	0	_	_	0	-	_	0.3	0		
	,	J. r	- 0			J			0.0	- 3		

3: Estes Drive Extension & East Site Driveway

Intersection							
Int Delay, s/veh	1.5						
Movement	NBL	NBR	NET	NER	SWL	SWT	J
Lane Configurations	NDL	TION.		NEIN	OVVE	- - 3 √1	
Traffic Vol, veh/h	8	113	558	2	29	806	
Future Vol, veh/h	8	113	558	2	29	806	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	- -	None	-	None	-	None	
Storage Length	50	0	_	INOHE	_	INUITE	
Veh in Median Storage		-	0	_	_	0	
Grade, %	s, # 0 0	_	0	_	_	0	
Peak Hour Factor	90	90	90	90	90	90	
			2			2	
Heavy Vehicles, %	2	2		2	2		
Mvmt Flow	9	126	620	2	32	896	
Major/Minor	Minor1	N	Major1	_	Major2		
Conflicting Flow All	1581	621	0	0	622	0	
Stage 1	621	_	_	-	_	_	
Stage 2	960	_	_	_	_	_	
Critical Hdwy	6.42	6.22	_	_	4.12	_	
Critical Hdwy Stg 1	5.42	-	_	_		_	
Critical Hdwy Stg 2	5.42	_	_	_	_	_	
Follow-up Hdwy	3.518	3.318	_	_	2.218	_	
Pot Cap-1 Maneuver	120	487	_	_	959	_	
Stage 1	536	-	_	_	-	_	
Stage 2	372	_	_	_	_	_	
Platoon blocked, %	312	_	_	-	-	_	
	112	487	_	-	959	-	
Mov Cap-1 Maneuver	112	407		-		-	
Mov Cap-2 Maneuver			-	-	-	-	
Stage 1	536	-	-	-	-	-	
Stage 2	347	-	-	-	-	-	
Approach	NB		NE		SW		
HCM Control Delay, s	16.6		0		0.3		
HCM LOS	С						
Minor Long/Major My	-1	NET	MEDI	VIDI 4 I	מים וחו	CVVII	
Minor Lane/Major Mvm	Ι	NET	NEKI	NBLn11		SWL	
Capacity (veh/h)		-	-	112	487	959	
HCM Lane V/C Ratio		-	-	0.079			
HCM Control Delay (s)		-	-	39.9	14.9	8.9	
HCM Lane LOS		-	-	Е	В	Α	
HCM 95th %tile Q(veh)	-	-	0.3	1	0.1	

Intersection						
Int Delay, s/veh	13.7					
		A 11 6 / 5	NET	NES	0)4"	014.7
Movement	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	Y		₽			4
Traffic Vol, veh/h	118	40	607	66	6	716
Future Vol, veh/h	118	40	607	66	6	716
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	5	4	2	6	2	2
Mvmt Flow	127	43	653	71	6	770
Majar/Miner	N //: /		Ania na		Mais =0	
	Minor1		Major1		Major2	
Conflicting Flow All	1471	688	0	0	724	0
Stage 1	688	-	-	-	-	-
Stage 2	783	-	-	-	-	-
Critical Hdwy	6.45	6.24	-	-	4.12	-
Critical Hdwy Stg 1	5.45	-	-	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-	-	-
Follow-up Hdwy		3.336	-	-	2.218	-
Pot Cap-1 Maneuver	138	443	-	-	879	-
Stage 1	493	-	-	-	-	-
Stage 2	445	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	136	443	-	-	879	-
Mov Cap-2 Maneuver	136	-	_	_	-	-
Stage 1	493	-	_	_	_	_
Stage 2	440	_	_	_	_	_
2.5.30 2						
Α	h 11 A /				0147	
Approach	NW		NE		SW	
HCM Control Delay, s	134.3		0		0.1	
HCM LOS	F					
Minor Lane/Major Mvn	nt	NET	NFRN	IWLn1	SWL	SWT
Capacity (veh/h)			-	165	879	
HCM Lane V/C Ratio		-	_		0.007	_
HCM Control Delay (s)				134.3	9.1	0
HCM Lane LOS		-		134.3 F	9.1 A	A
HCM 95th %tile Q(veh	\	-	-	8.3	0	- -
HOW SOUT WILL W(VEI))	-	-	0.5	U	-

Intersection						
Int Delay, s/veh	2.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	CDL	EDK	NDL	<u>ND1</u>	↑	אמט
Traffic Vol, veh/h	37	76	53	1302	853	6
Future Vol, veh/h	37	76	53	1302	853	6
Conflicting Peds, #/hr	4	0	2	0	000	2
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -		-		-	None
Storage Length	125	0	0	-	_	INOHE
Veh in Median Storage		-	-	0	0	_
Grade, %	, # 0	-	-	0	0	
Peak Hour Factor	94	94	94	94	94	94
	94	94	7	2	2	2
Heavy Vehicles, %						
Mvmt Flow	39	81	56	1385	907	6
Major/Minor N	Minor2	N	//ajor1	N	Major2	
Conflicting Flow All	1722	459	916	0	-	0
Stage 1	913	-	-	-	-	-
Stage 2	809	-	_	_	_	_
Critical Hdwy	6.92	7.02	4.24	_	_	-
Critical Hdwy Stg 1	5.92	-	-	_	-	_
Critical Hdwy Stg 2	5.92	_	_	_	_	_
Follow-up Hdwy	3.56	3.36	2.27	_	_	_
Pot Cap-1 Maneuver	77	538	710	_	_	_
Stage 1	342	-	-	_	_	_
Stage 2	388	_	_	_	_	_
Platoon blocked, %	000			_	_	_
Mov Cap-1 Maneuver	71	537	710	_	_	_
Mov Cap-1 Maneuver	71	-	710	_	_	_
Stage 1	341	_				
Stage 2	357	-	-	-	-	-
Staye 2	331	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	43.4		0.4		0	
HCM LOS	Е					
Minor Long/Major Mym	1	NDI	NDT	EDL 54 F	רמ ום־	CDT
Minor Lane/Major Mvm	ι	NBL		EBLn1 E		SBT
Capacity (veh/h)		710	-		537	-
HCM Lane V/C Ratio		0.079		0.554		-
HCM Control Delay (s)		10.5	-	106.1	12.9	-
HCM Lane LOS		В	-	F	В	-
HCM 95th %tile Q(veh)		0.3	-	2.3	0.5	-

Appendix G – Signal Warrant Analysis

Warrants Summary Page 1 of 2

				Wa	rrant	s S	Summ	ary						
Information														
Analyst Agency/Co Date Performed Project ID East/West Street File Name	H 12 C C E	2/21/2 H Mui ampu stes D	017 nicipa s Orive E	Carolin I Servic Extensiont.xhy	es	! !	Intersect Jurisdict Units Time Pe North/Se Major S	tion eriod Ar outh St		d 2 <i>A</i>	Estes D Chapel J.S. Cu 2021 W Airport I East-We	Hill, N stoma ith Site Or	C ry	
Project Description CH M					2116									
General	iaino	1 <i>par</i> 0	CTVICC	,s Ourn					Roa	dway N	etwork	<u> </u>		
Major Street Speed	35		٦ΙР	opulatio	n < 10	0 0	00		_	o Major				
(mph) 33 Nearest Signal (ft) 0 Coordinated Signal System Weekend Count														
		╟╌												0
Crashes (per year)	0		Adequate Trials of Alternatives 5-yr Growth Factor											
Geometry and Traffic EB WB NB SB LT TH RT LT RT LT TH RT LT RT LT RT LT RT LT RT RT LT RT RT <td>RT</td>														RT
Number of lanes, N 0 1 0 0 1 0 0 0 0 0 0														0
Lane usage TR LT LR I Vehicle Volume Averages 0 152 18 2 133 0 17 0 6 0 0														
(vph) 0 132 16 2 133 0 17 0 6 0 0 0 Pads (pad/h) / Gaps													0	
Peds (ped/h) / Gaps 0 / 0 0 / 0 0 / 0 0 / 0														
Delay (s/veh) / (veh-hr) 0 / 0 0.0 / 0 16.4 / 0 / 0														
Warrant 1: Eight-Hour Vehicular Volume														
1 A. Minimum Vehicular Volumes (Both major approachesand higher minor approach)or														
1 A. Minimum Vehicular Volumes (Both major approachesand higher minor approach)or1 B. Interruption of Continuous Traffic (Both major approachesand higher minor approach)or														
1 (80%) Vehicularand-	- Inte	errupti	on Vo	lumes	Βoth ι	maj	jor appr	oaches	and	highe	r minor	appro	ach)	
Warrant 2: Four-Hour V	ehic	ular \	/olun	1e										
2 A. Four-Hour Vehicular	r Volu	umes	(Both	major a	approa	ach	esand	d high	er min	or appr	oach)			
Warrant 3: Peak Hour														✓
3 A. Peak-Hour Condition	ns (N	/linor o	delay ·	and	minor	vol	lumea	and to	tal vol	ume)	or			~
3 B. Peak- Hour Vehicula	ar Vo	lumes	(Botl	h major	appro	ac	hesar	nd hig	her mi	nor app	roach)			✓
Warrant 4: Pedestrian	/olu	me												
4 A. Four Hour Volumes	or-	-												
4 B. One-Hour Volumes														
Warrant 5: School Cros	ssing	7												
5. Student Volumesand	d													
5. Gaps Same Period														
Warrant 6: Coordinated	l Sig	nal S	ysten	า										
6. Degree of Platooning	(Pred	domin	ant dii	rection	or botl	h di	irections	s)						
Warrant 7: Crash Expe	rienc	e												
7 A. Adequate trials of al	terna	atives,	obse	rvance	and e	nfo	rcemen	t failed	and-	-				
7 B. Reported crashes su	usce	ptible	to cor	rection	by sig	nal	l (12-mc	onth pe	riod)	and				

Warrants Summary Page 2 of 2

7 C. (80%) Volumes for Warrants 1A, 1Bor 4 are satisfied	
Warrant 8: Roadway Network	
8 A. Weekday Volume (Peak hour totaland projected warrants 1, 2 or 3)or	
8 B. Weekend Volume (Five hours total)	
Warrant 9: Grade Crossing	
9 A. Grade Crossing within 140 ftand	
9 B. Peak-Hour Vehicular Volumes	

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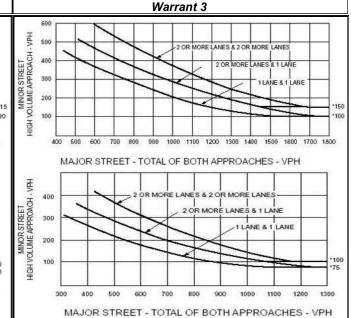
Warrants Volume Page 1 of 1

Warrants Volume Information Analyst CRS Intersection Estes Dr Ext & Airport Dr Agency/Co HNTB North Carolina, PC Jurisdiction Chapel Hill, NC Date Performed 12/21/2017 Units U.S. Customary Time Period Analyzed North/South Street Major Street Project ID CH Municipal Services Campus 2021 With Site Scenario Airport Dr East/West Street Estes Drive Extension East-West File Name Estes & Airport.xhy Project Description CH Municipal Services Campus Warrant 1

	nes for moving ch approach	Vehick (to)	es per hou tal of both	ur on majo approaci	or street nes)	Vehicles per hour on higher-volume minor-street approach (one direction only					
Major Street	Minor Street	100%	80%	70%	56%	100%	80%	70%	56%		
318	1	500	400	350	280	150	120	106	84		
2 or more	1	600	480	420	336	150	120	106	84		
2 or more	2 or more	600	480	420	336	200	160	140	112		
1	2 or more	500	400	350	280	200	160	140	112		

Number of lar traffic on ea	Vehick (to)	es perhoo tal of both	ur on majo approaci	or street nes)	Vehicles per hour on higher-volume minor-street approach (one direction only					
Major Street	Minor Street	100%	80%	70%	56%	100%	80%	70%	56%	
1	1	750	600	525	420	75	60	53	42	
2 or more	1	900	720	630	504	75	60	53	42	
2 or more	2 or more	900	720	630	504	100	80	70	56	
1	2 or more	750	600	525	420	100	80	70	56	

Warrant 2 MINOR STREET HIGH VOLUME APPROACH - VPH MINOR STREET HIGH VOLUME APPROACH - VPH 500 400 2 OR MORE LANES & 1 LANE 400 300 200 200 100 100 1100 600 700 800 900 1000 MAJOR STREET - TOTAL OF BOTH APPROACHES - VPH MINOR STREET HIGH VOLUME APPROACH - VPH MINOR STREET HIGH VOLUME APPROACH -- VPH OR MORE LANES 400 300 300 200 200 100 1000 MAJOR STREET - TOTAL OF BOTH APPROACHES - VPH



				Volu	ume Sumn	nary				
Majo	r Street Lanes	1	Minor S	treet Lanes 1	Sį	oeed	35	Populati	i on 1	0000+
Hours	Major Volume	Minor Volume	Total Volume	1A (100%)	1A (80%)	1B (100%)	1B (80%)	2 (100%)	3A (100%)	3B (100%)
07-08	1316	73	1389	No	No	No	Yes	No	No	No
08-09	0	0	0	No	No	No	No	No	No	No
09-10	0	0	0	No	No	No	No	No	No	No
10-11	0	0	0	No	No	No	No	No	No	No
11-12	0	0	0	No	No	No	No	No	No	No
12-13	974	58	1032	No	No	No	No	No	No	No
13-14	0	0	0	No	No	No	No	No	No	No
14-15	0	0	0	No	No	No	No	No	No	No
15-16	0	0	0	No	No	No	No	No	No	No
16-17	0	0	0	No	No	No	No	No	No	No
17-18	1395	158	1553	Yes	Yes	Yes	Yes	Yes	Yes	Yes
18-19	0	0	0	No	No	No	No	No	No	No
Totals	3685	289	3974	1	1	1	2	1	1	1

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Warrants Summary Page 1 of 2

				Warr	ants	Summ	ary						
Information													
Analyst Agency/Co Date Performed Project ID East/West Street File Name	H 1: C C A	2/21/2 H Mur ampu: irport	017 nicipal s Drive	Carolina, Services		Intersed Jurisdic Units Time Po North/S Major S	tion eriod Ar outh Sti		d	NC 86 6 Chapel U.S. Cu 2021 W NC 86 6 North-S	Hill, N ustoma /ith Site (MLK)	IC ary e Scen	
Project Description CH Municipal Services Campus													
General													
Major Street Speed	35												
(mph) Nearest Signal (ft)	0		Coordinated Signal System Weekend Count										
Crashes (per year)	0		Adequate Trials of Alternatives 5-yr Growth Factor										0
		<u> </u>	EB WB NB SB										
Geometry and Traffic LT TH RT LT TH RT LT TH RT LT TH												RT	
Number of lanes, N		1	0	1	0	0	0	1	2	0	0	2	0
Lane usage Vehicle Volume Average).C	L		R				L	T			TR	<u> </u>
(vph)	75	5	0	21	0	0	0	21	210	0	0	219	14
Peds (ped/h) / Gaps (gaps/h)		-	0/0			0/0		-	0/0			0/0	
Delay (s/veh) / (veh-hr) 6.7 / 0.2 0 / 0 0.3 / 0 / 0													
Warrant 1: Eight-Hour Vehicular Volume													
1 A. Minimum Vehicular Volumes (Both major approachesand higher minor approach)or													
1 B. Interruption of Conti	inuou	ıs Traf	fic (Bo	oth major	appro	aches -	-and h	igher	minor a	pproac	h)or		
1 (80%) Vehicularand-	Inte	errupti	on Vo	lumes (B	oth ma	ajor appı	roaches	and	l highe	er mino	r appro	oach)	
Warrant 2: Four-Hour \	/ehic	ular \	/olum	ie									
2 A. Four-Hour Vehicula	r Vol	umes	(Both	major ap	proacl	hesan	d high	er mir	or app	roach)			
Warrant 3: Peak Hour													
3 A. Peak-Hour Conditio	ns (N	/linor c	lelay -	and m	inor vo	olume	and to	tal vol	ume)-	-or			
3 B. Peak- Hour Vehicul	ar Vo	lumes	(Both	n major a	pproa	chesa	nd hig	her m	inor app	oroach)			
Warrant 4: Pedestrian	Volu	me											
4 A. Four Hour Volumes	or-	-											
4 B. One-Hour Volumes													
Warrant 5: School Cros	ssing	7											
5. Student Volumesan	d												
5. Gaps Same Period													
Warrant 6: Coordinated	d Sig	nal S	/stem)									
6. Degree of Platooning (Predominant direction or both directions)													
Warrant 7: Crash Experience													
7 A. Adequate trials of alternatives, observance and enforcement failedand													
7 B. Reported crashes s	usce	ptible	to cor	rection by	/ signa	al (12-m	onth per	riod) -	and				

Warrants Summary Page 2 of 2

7 C. (80%) Volumes for Warrants 1A, 1Bor 4 are satisfied	
Warrant 8: Roadway Network	
8 A. Weekday Volume (Peak hour totaland projected warrants 1, 2 or 3)or	
8 B. Weekend Volume (Five hours total)	
Warrant 9: Grade Crossing	
9 A. Grade Crossing within 140 ftand	
9 B. Peak-Hour Vehicular Volumes	

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Page 1 of 1 Warrants Volume

Warrants Volume Information Analyst NC 86 & Airport Dr Chapel Hill, NC Intersection Agency/Co HNTB North Carolina, PC Jurisdiction U.S. Customary 2021 With Site Scenario NC 86 (MLK Jr Blvd) North-South Date Performed 12/21/2017 Units Time Period Analyzed North/South Street Major Street Project ID CH Municipal Services Campus East/West Street Airport Drive NC 86 & Airport.xhy File Name Project Description CH Municipal Services Campus Warrant 1

Condition A-Minimum Vehicular Volume Vehicles per hour on higher-volume minor-street approach (one direction only) Vehicles per hour on major street (total of both approaches) 56% 70% 56% 100% 80% 70% 84 350 280 150 120 106 420 336 150 120 105 84

160

160

140

112

112

200

200

Number of lar traffic on ea	nes for moving ch approach	Vehick (to)	es perhoo tal of both	ur on majo approaci	or street nes)	Vehicles per hour on higher-volume minor-street approach (one direction only)							
Major Street	Minor Street	100%	80%	70%	56%	100%	80%	70%	56%				
1	1	750	600	525	420	75	60	53	42				
2 or more	1	900	720	630	504	75	60	53	42				
2 or more	2 or more	900	720	630	504	100	80	70	56				
1	2 or more	750	600	525	420	100	80	70	56				

							Wa	rran	t 2							
£	500		T	7	_ 2	OR N	MORE L	ANES	8 2 OF	R MOF	E LAN	I IES	Т			1
-	400		\downarrow	+	/		1,2	OR MO	RE L	ANES	& 1 LA	NE	+	\dashv		+
ACA.	300		\	\geq	_	2	*	\downarrow	1	_1 LA	NE &	LAN	4			+
IE A	200				_		\Rightarrow	\downarrow	$\stackrel{\sim}{\rightarrow}$	_			_	_		-
500	100							4	4	_		-	1			
HIGH VOLUME APPROACH - VPH											\$5.65.E					
i i	30	10	400	500	60	00	700	800	900	10	00	1100	1200	130	00 1	40
т.	M 400	IAJO	R S	TRE	ET -	TOT	TAL C)F BC)TH	APP	ROA	CHE	S-	VPH		7
4- VP					-20	OR MO	ORE LA	NES &	2 OR	MORE	LANE	s				
HIGH VOLUME APPROACH - VPH	300		1		*	_	20	OR MO	RELA	NES 8	1 LAI	NE		+	_	1
APP	200		\	_	4	<	1	_	/	1 LAN	E & 1	LANE		_		-
LUME					1	_		\geq		-						
31 70	100		1		+				-		=	\Rightarrow	1	=		1:2
Ħ																J'
	20	00	300	1	400		500	60	10	700)	800		900	10	000
								OF B								

Number of lanes for moving traffic on each approach

Minor Street

2 or more

2 or more

100% 80%

500 400

600

500

480

400 350 280

420 336

Major Street

2 or more

2 or more

1

				Warra	ant 3				
122	600		ĪĪ	TT	T				
HdA	500	1		-21	OR MORE LA	ANES & 2 OF	MORE LANE	S	===
ET	400			X	+	2 OR MORE	LANES & 1 L	ANE	
STRE	300				\times	-	1 LANE &	1 LANE	_
MINOR STREET HIGH VOLUME APPROACH - VPH	200			1	\Rightarrow	\Rightarrow			_
MOV.	100			++		_			150
호		600 600	700 800	900 1000	1100 120	0 1300 1	400 1500	1600 1700	
T CCH - VPH			PEET - TO	R MORE LA	ANES & 2	OR MORE	LANES-		
TREE	300			X	2 OR MOR	1 1	& 1 LANE	NE	-
VOR S ME AF	200	_	1		\rightarrow		INC OF LA	NC.	
MINOR STREET HIGH VOLUME APPROACH - VPH	100	12			*			4	*100
	300	400	500 600	700	800	900 10	00 1100	1200	1300
	M	AJOR S	TREET	- TOTAL	OF BO	THAP	PROACH	HES - V	PH

				Volu	ıme Sumn	ary				
Majo	r Street Lanes	2+	Minor St	reet Lanes 2+	Sį	oeed	35	Populati	ion 1	0000+
Hours	Major Volume	Minor Volume	Total Volume	1A (100%)	1A (80%)	1B (100%)	1B (80%)	2 (100%)	3A (100%)	3B (100%)
07-08	1948	102	2050	No	No	Yes	Yes	No	No	No
08-09	0	0	0	No	No	No	No	No	No	No
09-10	0	0	0	No	No	No	No	No	No	No
10-11	0	0	0	No	No	No	No	No	No	No
11-12	0	0	0	No	No	No	No	No	No	No
12-13	1423	98	1521	No	No	No	Yes	No	No	No
13-14	0	0	0	No	No	No	No	No	No	No
14-15	0	0	0	No	No	No	No	No	No	No
15-16	0	0	0	No	No	No	No	No	No	No
16-17	0	0	0	No	No	No	No	No	No	No
17-18	2214	113	2327	No	No	Yes	Yes	No	No	No
18-19	0	0	0	No	No	No	No	No	No	No
Totals	5585	313	5898	0	0	2	3	0	0	0

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Appendix H - Crash Data

Study Criteria Summary

County:ORANGECity:All and RuralDate:11/1/2012to 10/31/2017Study:ESTESDREXTSTRIP

Location: SR 1780 (Estes Dr Ext) from SR 1843 (Seawell School Rd) to NC 86 (Martin Luther King Jr. Blvd)

Report Details

				-	110	port	. DC	Lan	<u> </u>											
Acc				1				'	Total		Inju	ıries		Co	ondi	tion	Ro	ad	Trfc	Ctl
No	Crash ID	Milepost	Date	Acc	iden	t Type	Э	Da	amage	F	Α	В	С	R	L	w	Ch	Ci	Dv	Op
1	103707110	0.950	03/13/2013 08:15	RAN OF RIGHT					1500	0	0	0		1	1	1	1	0	3	
Unit	1:1 	Alchi/Dr	gs : 0	Speed:	35 	MPH	Dir:	N 		Veh	Mnvr	/Ped	Actn:		8		Obj Si	trk:	42	
2	104012507	0.950	03/01/2014 06:35	RAN OF RIGHT	F RO	AD -		\$	3000	1	0	0	0	1	3	2	7	0	13	1
Unit	1: 20	Alchi/Dr	gs: 1	Speed:	55 	MPH	Dir:	S\ 	w 	Veh	Mnvr	/Ped	Actn:	_	4		Obj Si	trk:	64	_
3	103632486	0.964	12/06/2012 17:20	ANIMAL				\$	2000	0	0	0	0	1	5	2	3	0	0	
Unit	1:1	Alchi/Dr	gs: 0	Speed:	35	MPH	Dir:	E		Veh	Mnvr	/Ped	Actn:	_	4		Obj Si	trk:	17	
4	104364723	1.070	04/28/2015 19:15	ANIMAL				\$	500	0	0	0	0	1	1	1	7	0	13	2
Unit	1 :1	Alchi/Dr	gs : 0	Speed:	35	MPH	Dir:	N		Veh	Mnvr	/Ped	Actn:	_	4		Obj Si	trk:	17	
5	103714748	1.250	03/24/2013 18:31	RAN OF	F RO	— — - AD - LE	 ≣FT	\$	8000	0	0	0	0	2	4	2	5	0		
Unit	1:1	Alchi/Dr	gs: 1	Speed:	35	MPH	Dir:	Е		Veh	Mnvr	/Ped	Actn:		4	C	Obj St	trk:	58	
6	103954240	1.461	12/15/2013 17:38	ANIMAL				\$	1000	0	0	0	0	1	4	1	1	0	0	- -
Unit	1:1	Alchi/Dr	gs: 0	Speed:	30	MPH	Dir:	_ S		Veh	Mnvr	/Ped	Actn:	_	4		Obj Si	trk:	17	
7	103998877	1.467	01/21/2014 16:57	RAN OF RIGHT	F RO	AD -		\$	8000	0	0	0	0	1	2	2	3	0	0	2
Unit	1:2	Alchi/Dr	gs: 0	Speed:	45	MPH	Dir:	E		Veh	Mnvr	/Ped	Actn:	_	4		Obj Si	trk:	34	_
8	103628509	1.476	12/10/2012 07:03	REAR E STOP	ND, S	SLOW (DR	\$	4400	0	0	0	1	2	1	2	1	1		
Unit	1 : 1	Alchl/Dr	gs: 0	Speed:	45	MPH	Dir:	W	,	Veh	Mnvr	/Ped	Actn:		4	c	Obj Si	trk:		
Unit	2 : 1	Alchi/Dr	gs : 0	Speed:	0	MPH	Dir:	W	1	Veh	Mnvr	/Ped	Actn:		11	C	Obj St	trk:		
9	104599113	1.500	12/14/2015 20:37	RAN OF RIGHT	F RO	– – - AD -		\$	4000	0	0	1	0	1	- 4	1	3	0	13	1
Unit	1:1	Alchi/Dr	gs : 0	Speed:	50	MPH	Dir:	W	1	Veh	Mnvr	/Ped	Actn:		4	C	Obj Si	trk:	33	
10	104685929	1.677	03/17/2016 07:54	REAR E STOP	– – ND, S	LOW C	 DR	\$	2800	0	0	0	0	1	 1	1	3	0	3	1
Unit	1:2	Alchi/Dr	gs : 0	Speed:	30	MPH	Dir:	Е		Veh	Mnvr	/Ped	Actn:		1	C	Obj St	trk:		

12/11/2017 -1-

A 1									port Tetal		Iniu	rice		_	ondi	tion	Ro	ad	Trf	: Ctl
Acc No	Crash ID	Milepost	Date	۸۵	cidon	t Type		ı	Total amage	F	A	ries B	С	R	_	w	\vdash	Ci		Op
Unit	2: 1	Alchl/Drg		Speed:		MPH			amage	•			Actn:		4		bj St		DV	Ор
												— –	- - -	_			, s, s,			
11	104977435	1.692	12/19/2016 20:43	RAN OF	F ROA	AD - LE	FT	\$	10000	0	0	1	0	1	4	1	5	0	0	
Unit	1:2 	Alchl/Drg	gs: 0 	Speed:	40	MPH 	Dir:	s 		Veh	Mnvr — —	/Ped 	Actn:	_	4 – –)bj St — —	rk: 	17	
12	104924595	1.724	10/24/2016 14:53	REAR E	ND, SI	LOW C	R	\$	1000	0	0	0	0	1	1	1	1	0	3	1
Unit	1 : 1	Alchl/Drg	gs: 1	Speed:	0	MPH	Dir:	Е		Veh l	Mnvr	/Ped	Actn:		11	C	bj St	rk:		
Unit	2 : 4	Alchi/Drg	gs: 0	Speed:	0	MPH	Dir:	Е		Veh I	Mnvr	/Ped	Actn:		11	C	bj St	rk:		
13	103647661	1.730	 12/20/2012 22:43	OTHER				\$	10000	0	0	0	3	2	 4	3	1	7	3	2
Unit	1 : 1	Alchl/Drg	gs: 0	Speed:	10	MPH	Dir:	W	,	Veh l	Mnvr	/Ped	Actn:		4	c	bj St	rk:		
Unit	2 : 1	Alchl/Drg	_	Speed:	35	MPH	Dir:	S		Veh	Mnvr	/Ped	Actn:		4	c	bj St	rk:		
14	103654207	1.730	 12/21/2012 17:20	REAR E	. – – END, SI	 LOW C	- – -	- \$	2000	0	0	0	0	2	 1	3	1	0	3	1
Unit	1:7	Alchl/Drg	gs: 0	Speed:	5	MPH	Dir:	Е		Veh l	Mnvr	/Ped	Actn:		4	c	bj St	rk:		
Unit	2 : 1	Alchl/Drg	gs: 0	Speed:	0	MPH	Dir:	E		Veh	Mnvr	/Ped	Actn:	_	11		bj St	rk:		
15	103659727	1.730	01/01/2013 17:26	LEFT TO	,	OADW	/AYS	\$	2500	0	0	0	1	2	2	3	1	0	3	1
Unit	1:4	Alchl/Drg	gs: 0	Speed:	35	MPH	Dir:	N	E	Veh I	Mnvr	/Ped	Actn:		8	C	bj St	rk:		
Unit	2 : 1	Alchl/Drg	gs: 0	Speed:	0	MPH	Dir:	S		Veh I	Mnvr	/Ped	Actn:		1	C	bj St	rk:		
Unit	3:4	Alchl/Drg	gs: 0	Speed:	15	MPH	Dir:	SI	E	Veh I	Mnvr	/Ped	Actn:		8	C	bj St	rk:		
16	103678294	1.730	02/08/2013 12:12	LEFT TO	-	AME		\$	10000	0	0	0	0	1	1	2	1	0	3	1
Unit	1 : 1	Alchl/Drg	gs: 0	Speed:	20	MPH	Dir:	Е		Veh	Mnvr	/Ped	Actn:		4	C	bj St	rk:		
Unit	2 : 1	Alchl/Drg	gs: 0	Speed:	20	MPH	Dir:	W	•	Veh	Mnvr	/Ped	Actn:		8	C	bj St	rk:		
17	103761905	1.730	 05/20/2013 15:56	LEFT TO	-	AME		\$	8500	0	0	1	0	2	 1	3	3	0	3	1
Unit	1 : 1	Alchl/Drg	gs: 0	Speed:	30	MPH	Dir:	S		Veh	Mnvr	/Ped	Actn:		4	C	bj St	rk:		
Unit	2 : 1	Alchl/Drg	gs: 0	Speed:	15	MPH	Dir:	S		Veh	Mnvr	/Ped	Actn:		8	C	bj St	rk:		
18	103787091	1.730	06/16/2013 18:23	LEFT TO		– – – SAME		\$	6000	0	0	0	0	1	1	1	1	0	3	1
Unit	1:2	Alchl/Drg	gs: 0	Speed:	35	MPH	Dir:	S		Veh	Mnvr	/Ped	Actn:		4	C	bj St	rk:		
Unit	2 : 17	Alchl/Drg	gs : 0	Speed:	15	MPH	Dir:	Ε		Veh	Mnvr	/Ped	Actn:		8	C	bj St	rk:		
19	103892317	1.730	10/21/2013 19:13	SIDESV DIRECT		PPOS	ITE	\$	3500	0	0	0	0	1	- - 2	1	1	0	3	1
Unit	1 : 1	Alchl/Drg	gs: 0	Speed:	10	MPH	Dir:	W	,	Veh	Mnvr	/Ped	Actn:		8	c	bj St	rk:		
Unit	2 : 1	Alchl/Drg	gs: 0	Speed:	25	MPH	Dir:	Ε		Veh l	Mnvr	/Ped	Actn:		4	C	bj St	rk:		

12/11/2017 -2-

Acc							Allai			Total		Iniu	ries		С	ondi	tion	Ro	ad	Trf	: Ctl
No	Crash ID	Milepost	D	ate	Acc	ciden	t Type	Э		amage	F		В	С	R	L	W	Ch	_		Ор
20	103961747	1.730	12/09/ 18	/2013 :08	SIDESW	,	SAME		\$	1800	0	0	0	0	2	4	3	1	0	3	2
Unit	1 : 1	Alchi/Dr	gs:	0	Speed:	20	MPH	Dir:	Ν		Veh	Mnvr	/Ped	Actn:		7	C	bj St	rk:		
Unit	2 : 4	Alchi/Dr	gs:	0	Speed:	20	MPH	Dir:	N		Veh	Mnvr	/Ped	Actn:		8	C	bj St	rk:		
21	104005855	1.730	02/01/	 /2014 :32	LEFT TU DIFFER		ROADV	VAYS	\$	7000	0	0	0	0	1	1	2	1	0	3	1
Unit	1 : 1	Alchi/Drg	gs:	0	Speed:	35	MPH	Dir:	W	1	Veh	Mnvr	/Ped	Actn:		4	c	bj St	rk:		
Unit	2 : 1	Alchi/Dr	gs:	0	Speed:	15	MPH	Dir:	Е		Veh	Mnvr	/Ped	Actn:	!	8	c	bj St	rk:		
22	104002496	1.730	02/14/ 14	_ /2014 :52	RIGHT 1 DIFFER		•	VAYS	\$	1550	0	0	0	0	1	1	1	1	0	3	1
Unit	1:1	Alchl/Dr	gs:	0	Speed:	30	MPH	Dir:	S		Veh	Mnvr	/Ped	Actn:		4	C	bj St	rk:		
Unit	2 : 1	Alchi/Dr	gs:	0	Speed:	10	MPH	Dir:	E		Veh	Mnvr	/Ped	Actn:	:	7	C	bj St	rk:		
23	104068211	1.730	05/16/ 15	_ /2014 :42	ANGLE				\$	2800	0	0	0	0	1	1	1	1	0	3	1
Unit	1 : 1	Alchi/Dr	gs:	0	Speed:	5	MPH	Dir:	SI	=	Veh	Mnvr	/Ped	Actn:		8	c	bj St	rk:		
Unit	2 : 2	Alchi/Dr	gs:	0	Speed:	15	MPH	Dir:	N		Veh	Mnvr	/Ped	Actn:	!	4	c	bj St	rk:		
24	104582812	1.730	11/22 01	– – – /2015 :42	ANGLE				\$	12000	0	0	0	0	1	4	1	1	0	3	1
Unit	1:4	Alchi/Dr	gs:	0	Speed:	40	MPH	Dir:	s		Veh	Mnvr	/Ped	Actn:	:	4	c	bj St	rk:		
Unit	2 : 5	Alchi/Dr	gs:	0	Speed:	10	MPH	Dir:	Е		Veh	Mnvr	/Ped	Actn:		8	C	bj St	rk:		
25	105064389	1.730	04/03/	_ /2017 :46	LEFT TU ROADW	,	SAME		\$	6700	0	0	0	0	2	4	2	3	0	3	1
Unit	1:4	Alchi/Dr	gs:	0	Speed:	30	MPH	Dir:	W	,	Veh	Mnvr	/Ped	Actn:		4	c	bj St	rk:		
Unit	2 : 1	Alchi/Dr	gs:	0	Speed:	30	MPH	Dir:	Е		Veh	Mnvr	/Ped	Actn:		8	C	bj St	rk:		
Unit	3 : 4	Alchi/Dr	gs:	0	Speed:	35	MPH	Dir:	S		Veh	Mnvr	/Ped	Actn:		1	c	bj St	rk:		
			-												-						

Acc No - Accident Number

Legend for Report Details:

Injuries: F - Fatal, A - Class A, B - Class B, C - Class C Condition: R - Road Surface, L - Ambient Light, W - Weather

Rd Ch - Road Character

Rd Ci - Roadway Contributing Circumstances

Trfc Ctl - Traffic Control: Dv - Device, Op - Operating

Alchl/Drgs - Alcohol Drugs Suspected

Veh Mnvr/Ped Actn - Vehicle Maneuver/Pedestrian Action

Obj Strk - Object Struck

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Summary Statistics

High Level Crash Summary

Crash Type	Number of Crashes	Percent of Total
Total Crashes	25	100.00
Fatal Crashes	1	4.00
Non-Fatal Injury Crashes	6	24.00
Total Injury Crashes	7	28.00
Property Damage Only Crashes	18	72.00
Night Crashes	9	36.00
Wet Crashes	8	32.00
Alcohol/Drugs Involvement Crashes	3	12.00

Crash Severity Summary

Crash Type	Number of Crashes	Percent of Total
Total Crashes	25	100.00
Fatal Crashes	1	4.00
Class A Crashes	0	0.00
Class B Crashes	3	12.00
Class C Crashes	3	12.00
Property Damage Only Crashes	18	72.00

Vehicle Exposure Statistics

Annual ADT = 12400

Total Length = 0.78 (Miles) 1.255 (Kilometers)

Total Vehicle Exposure = 17.66 (MVMT) 28.42 (MVKMT)

Crash Rate	Crashes Per 100 Million Vehicle Miles	Crashes Per 100 Million Vehicle Kilometers
Total Crash Rate	141.55	87.96
Fatal Crash Rate	5.66	3.52
Non Fatal Crash Rate	33.97	21.11
Night Crash Rate	50.96	31.66
Wet Crash Rate	45.30	28.15
EPDO Rate	822.15	510.86

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Miscellaneous Statistics

Severity Index =	5.81
EPDO Crash Index =	145.20
Estimated Property Damage Total = \$	120550.00

Accident Type Summary

	Number of	Percent
Accident Type	Crashes	of Total
ANGLE	2	8.00
ANIMAL	3	12.00
LEFT TURN, DIFFERENT ROADWAYS	2	8.00
LEFT TURN, SAME ROADWAY	4	16.00
OTHER COLLISION WITH VEHICLE	1	4.00
RAN OFF ROAD - LEFT	2	8.00
RAN OFF ROAD - RIGHT	4	16.00
REAR END, SLOW OR STOP	4	16.00
RIGHT TURN, DIFFERENT ROADWAYS	1	4.00
SIDESWIPE, OPPOSITE DIRECTION	1	4.00
SIDESWIPE, SAME DIRECTION	1	4.00

Injury Summary

Injury Type	Number of Injuries	Percent of Total
Fatal Injuries	1	11.11
Class A Injuries	0	0.00
Class B Injuries	3	33.33
Class C Injuries	5	55.56
Total Non-Fatal Injuries	8	88.89
Total Injuries	9	100.00

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Monthly Summary

Month	Number of Crashes	Percent of Total
Jan	2	8.00
Feb	3	12.00
Mar	4	16.00
Apr	2	8.00
May	2	8.00
Jun	1	4.00
Jul	0	0.00
Aug	0	0.00
Sep	0	0.00
Oct	2	8.00
Nov	1	4.00
Dec	8	32.00

Daily Summary

Day	Number of Crashes	Percent of Total
Mon	8	32.00
Tue	3	12.00
Wed	1	4.00
Thu	3	12.00
Fri	4	16.00
Sat	2	8.00
Sun	4	16.00

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Hourly Summary

	Number of	Percent
Hour	Crashes	of Total
0000-0059	0	0.00
0100-0159	1	4.00
0200-0259	0	0.00
0300-0359	0	0.00
0400-0459	0	0.00
0500-0559	0	0.00
0600-0659	1	4.00
0700-0759	2	8.00
0800-0859	1	4.00
0900-0959	0	0.00
1000-1059	0	0.00
1100-1159	0	0.00
1200-1259	2	8.00
1300-1359	0	0.00
1400-1459	2	8.00
1500-1559	2	8.00
1600-1659	1	4.00
1700-1759	4	16.00
1800-1859	3	12.00
1900-1959	2	8.00
2000-2059	3	12.00
2100-2159	0	0.00
2200-2259	1	4.00
2300-2359	0	0.00

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Light and Road Conditions Summary

Condition	Dry	Wet	Other	Total
Day	9	3	0	12
Dark	5	4	0	9
Other	3	1	0	4
Total	17	8	0	25

Object Struck Summary

Object Type	Times Struck	Percent of Total
ANIMAL	4	44.44
DITCH	1	11.11
GUARDRAIL FACE ON SHOULDER	1	11.11
OTHER FIXED OBJECT	1	11.11
TREE	1	11.11
UTILITY POLE	1	11.11

Vehicle Type Summary

Vehicle Type	Number Involved	Percent of Total
MOTORCYCLE	1	2.33
PASSENGER CAR	27	62.79
PICKUP	5	11.63
SCHOOL BUS	1	2.33
SPORT UTILITY	7	16.28
TAXICAB	1	2.33
VAN	1	2.33

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Yearly Totals Summary

Accident Totals

Year	Total Accidents	Fatal Accidents	Injury Accidents	Property Damage Only Accidents
2012	4	0	2	2
2013	9	0	2	7
2014	5	1	0	4
2015	3	0	1	2
2016	3	0	1	2
2017	1	0	0	1
Total	25	1	6	18

Injury Totals

Year	Fatal Injuries	Class A, B, or C Injuries
2012	0	4
2013	0	2
2014	1	0
2015	0	1
2016	0	1
2017	0	0
Total	1	8

Miscellaneous Totals

Year	F	Property Damage	EPDO Index
2012	\$	18400	18.80
2013	\$	42800	23.80
2014	\$	22350	80.80
2015	\$	16500	10.40
2016	\$	13800	10.40
2017	\$	6700	1.00
Total	\$	120550	145.20

Type of Accident Totals

	Run Off Road &							
Year	Left Turn	Right Turn	Rear End	Fixed Object	Angle	Side Swipe	Other	
2012	0	0	2	0	0	0	2	
2013	4	0	0	2	0	2	1	
2014	1	1	0	2	1	0	0	

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	Run Off Road &							
Year	Left Turn	Right Turn	Rear End	Fixed Object	Angle	Side Swipe	Other	
2015	0	0	0	1	1	0	1	
2016	0	0	2	1	0	0	0	
2017	1	0	0	0	0	0	0	
Total	6	1	4	6	2	2	4	

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Strip Diagram

Features	Milepost	Crash IDs
SR 1843 SEWELL SCHOOL		103707110 104012507
	0.96	103632486
	0.97	
	0.98	
	0.99	
	1.00	
	1.01	
	1.02	
	1.03	
	1.04	
	1.05	
	1.06	
	1.07	104364723
	1.08	
	1.09	
	1.10	
RR LOT ENTRANCE	1.11	
	1.12	
	1.13	
	1.14	
	1.15	
	1.16	
	1.17	
	1.18	
	1.19	
	1.20	
	1.21	
	1.22	
	1.23	
	1.24	100014040
		103714748
	1.26	
	1.27	
	1.28	
	1.29	
	1.30	
	1.31	
	1.32	
	1.33 1.34	
	1.34	
	1.36	
	1.37	
	1.38	
	1.30	

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Features	Milepost	Crash IDs
	1.39	
	1.40	
	1.41	
	1.42	
	1.43	
	1.44	
	1.45	
	1.46	103954240
AIRPORT	1.47	103998877
	1.48	103628509
	1.49	
	1.50	104599113
	1.51	
	1.52	
	1.53	
	1.54	
	1.55	
	1.56	
	1.57	
	1.58	
	1.59	
FACILITIES DR	1.60	
	1.61	
	1.62	
	1.63	
	1.64	
	1.65	
	1.66	
	1.67	
	1.68	104685929
	1.69	104977435
	1.70	
	1.71	
	1.72	104924595
NC 86 SR 1750 MARTIN LUTHER KING	1.73	103647661 103654207 103659727 103678294
		103761905 103787091 103892317 103961747
		104005855 104002496 104068211 104582812
		105064389

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Study Criteria

Study Name	Log No.	PH No.	TIP No.	K/A Cf.	B/C Cf.	ADT	ADT Route
ESTESDREXTSTRIP				76.8	8.4	12400	40001780

Request Date Courier Service Phone No. Ext. Fax No.

Count	ty		Municipality					
Name	Code	Div.	Name	Code	Y-Line Ft.	Begin Date	End Date	Years
ORANGE	68	7	All and Rural		0	11/1/2012	10/31/2017	5.00

Location Text Requestor

SR 1780 (Estes Dr Ext) from SR 1843 (Seawell School Rd) to NC 86 (Martin Luther King Jr. Blvd)

Included Accidents	Old MP	New MP	Туре
103998877	1.531	1.467	R
104977435	1.294	1.692	R

Excluded Accidents

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Fiche Roads				
Name	Code			
SR 1780	40001780			
ESTES DR EXT	50009903			
SR 1750	40001750			
HILLCREST	50014034			

Strip Road

Name	Code	Begin MP	End MP	Miles	Kilometers
SR 1780	40001780	0.950	1.730	0.780	1.255

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